CITY OF POWELL

PLANNING AND ZONING COMMISSION (P&Z) FINAL DEVELOPMENT PLAN APPLICATION



ALL ITEMS ON THIS APPLICATION MUST BE COMPLETED.

Application Fee: \$700.00 + \$100.00 per acre Per Fee Ordinance 2018-48

Applicant: Home Steitz LLC									
Address/City/State/Zip: <u>c/o John Wicks</u> ; 267 N. Liberty St., Powell, OH 43065									
	mail Address: john.wicks@rpddllc.com								
Phone No: 614-323-7800		Fax No:							
Property Owner: TLK Developme	ent, LLC, c/o Kim Kelsik								
Address/City/State/Zip: 5751 Koll									
Email Address: kelsik@hotmail.co									
Phone No: 614-260-0649		Fax No:							
Architect/Designer for Applicant:	G2 Planning and Design,	c/o Gary Smith							
Address/City/State/Zip: 720 E. Br	oad St., Suite 200, Colum	bus, OH 43215							
Email Address: gsmith@g2plann	ing.com								
Phone No: 614-390-6149		Fax No:							
Property Address: 0 Home Road		76							
		ulture Proposed Use:	Planned Commercial						
Lot Number/Subdivision: N/A Existing Use: Agriculture Proposed Use: Planned Commercial Reason for Administrative Review (attach necessary documents): Final Development Plan Review/Approval									
SEE SUPPORTING DOCUMENTS ATTACHED TO	THIS APPLICATION								
,									

Checklist:

- Preliminary Plan requirements set forth in Section 1143.11(c) and Final Plan requirements set forth in Section 1143.11(i).
- Provide any other information that may be useful to the Planning and Zoning Commission or City Staff in the space below or attach additional pages.
- 5 copies of all drawings, text, any other items, and application.
- 1 digital copy (CD, USB, Email) of the complete application packet.
- Attach the required fee \$700.00 + \$100.00 per acre. \$1,855.50
- Post a public notice sign at least (10) days prior to a public hearing or public meeting, pursuant to ordinance 1107.035 Public notice sign details found <u>here</u>.

(See Over)

APPROVAL SHALL EXPIRE AND MAY BE REVOKED IF CONSTRUCTION DOES NOT BEGIN WITHIN TWO (2) YEARS FROM THE DATE OF ISSUANCE OF APPROVAL.

I agree to grant the City Staff, the Commission, Board or Council considering this application access to the property that is the subject of this application for the purposes of reviewing this application and posting public notice for this application.

ure of Applicant:	un	Date: 4/23/19	
Office Use		Office Use	
		Type/Date:	_
		Base Fee: \$700.00	_
		Per Acre: \$100.00X () =	
		Total:	
Received		Prepared by:	
		Reviewed by:	
		PAYOR:	
		RECIEPT #	

City of Powell · 47 Hall Street · Powell, Ohio 43065 · (614) 885-5380 · (614) 885-5339 fax·www.cityofpowell.us

MIDDLEBURY CROSSING FINAL DEVELOPMENT PLAN CITY OF POWELL, OHIO APRIL 2019

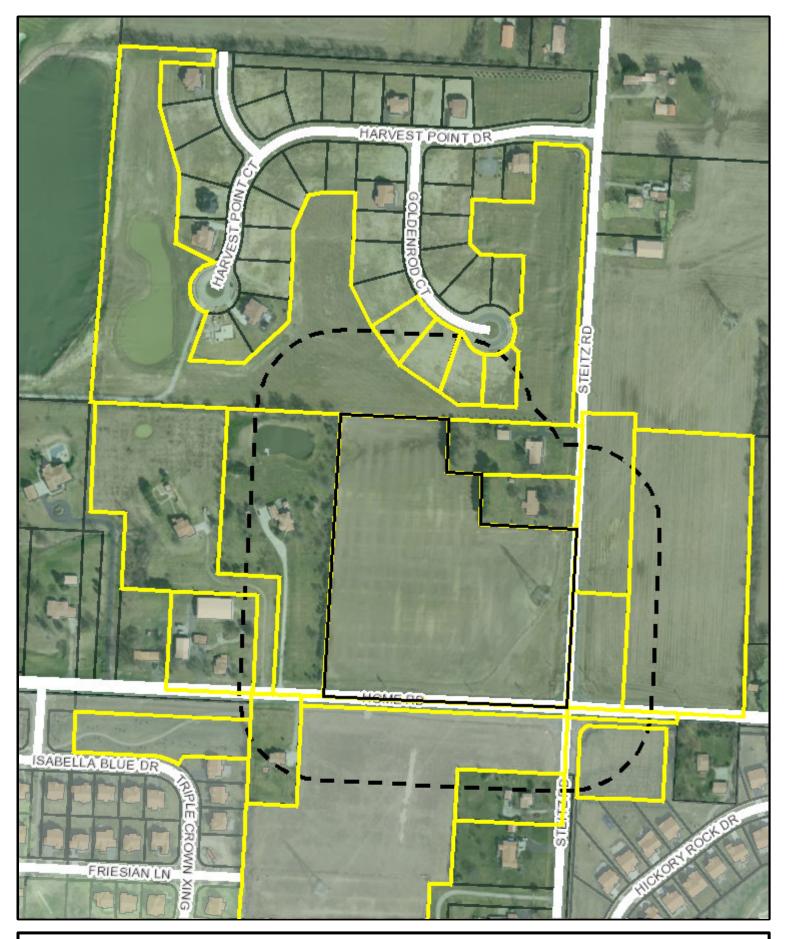


For: Home Steitz LLC 267 N. Liberty Street Powell, Ohio 43065 614-323-7800

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- 8. Architectural Elevations, Color Scheme and Details
- 9. Site Lighting Details
- 10. Traffic Impact Study
- 11. Owner Agency Letter

Parcel Number	Owner Name	Address	Market Value	Conveyance Date	te Year Built	Bedrooms	Baths	Finished Sq Ft	Acreage
31923001002000	CONRAD JEANNE BRAD	7344 STEITZ RD; POWELL; 43065	194800	6/27/2002	1978	3	2	1450	1
31922027010000	CROWN RIDGE INVESTMENTS LLC	7340 GOLDENROD CT; POWELL; 43065	89100	3/10/2016					0.37
31922027012000	CROWN RIDGE INVESTMENTS LLC	7312 GOLDENROD CT; POWELL; 43065	81000	3/10/2016					0.482
31922027011000	CROWN RIDGE INVESTMENTS LLC	7326 GOLDENROD CT; POWELL; 43065	89100	3/10/2016					0.434
31923002120000	DOODAN JOSEPH E	7514 STEITZ RD; POWELL; 43065	144900	10/27/1997	1901	3	1	1508	1.103
31924010006000	GOLF VILLAGE PROPERTY OWNERS ASSOCIATION		7700	12/17/2013					1.276
31924001041005	HOMEWOOD CORPORATION		0	4/11/2013					0.228
31923002001002	HORSEPOWER FARMS LLC		0	1/31/2019					0.549
31923002001000	HORSEPOWER FARMS LLC	4301 HOME RD; POWELL; 43065	230000	1/31/2019					9.458
31923002001001	JENKINS TRACIE A	4319 HOME RD; POWELL; 43065	264300	4/21/2008	1995	3	2	2258	1.136
31922027013000	KAUFMAN JOHN J & KELLY	7300 GOLDENROD CT; POWELL; 43065	81000	9/28/2018					0.43
31924001039000	PEARL DAVID R @3		26600	8/15/2018					1.28
31924001040000	PEARL DAVID R @3		45200	8/15/2018					2
31924001036000	PEARL DAVID R @3		128600	8/15/2018					7.09
31923001004000	PETZ JONATHAN A & STACEY A	4320 HOME RD; POWELL; 43065	450000	11/15/2013	1992	4	2	3126	5
31923001006000	POWELL AUTO CLUB LLC	4346 HOME RD; POWELL; 43065	211000	11/8/2016	1901	3	1	2291	1.72
31923001001000	PRICE JACK A EULA GAY	7294 STEITZ RD; POWELL; 43065	181900	7/30/2001	1962	3	1	1371	1.5
31923025002000	RESERVE AT SCIOTO GLENN LLC THE		8800	8/28/2014					1.463
31923001005000	ROSI PAOLO GIOVANNI I & CYNTHIA A K	4330 HOME RD; POWELL; 43065	423900	9/28/2018	1994	4	3	3523	5.01
31922027001000	SCIOTO RESERVE MASTER ASSOCIATION INC	0; POWELL; 43065	87000	10/27/2017					14.501
31923001003000	TLK DEVELOPMENT LLC		146200	6/21/1999					11.74





2. Adjacent Property Owners Map

Information contained within this map may be used to generally locate, identify and inventory land parcels within Delaware County.

Delaware County cannot warrant or guarantee the information contained herein, including, but not limited to its accuracy or completeness. The map parcel lines shown are approximate and this information cannot be constructed or used as a "legal description" of a parcel.

Flood Plain information is obtained from FEMA and is administered by the Delaware County Building Department (740-833-2201).

Please report any errors or omissions to the Delaware County Auditor's office at delcogis@co.delaware.oh.us.

Prepared by: Delaware County Auditor's GIS Office



ware County Auditor George Kaitsa

MIDDLEBURY CROSSING City of Powell, Ohio Site Narrative

April 2019

The subject property located at the northwest corner of Home Road and Steitz Road in Liberty Township, Delaware County, Ohio, is approximately 11.74 gross acres in size and is currently used as agricultural/farm ground. The parcel has approximately 710 feet of frontage along Home Road to the south and 508 feet of frontage along Steitz Road to the east. A high-tension power line and associated 150-foot wide easement traverse the southeast corner of the tract, occupying approximately 1.84 acres. An existing Del-Co Water raw water line was recently installed within this easement area as well.

Utilities

Public sewer is available on the north side of the property. Del-Co Water public water lines exist along both Home Road and Steitz Road. Electric, natural gas, telephone and cable services are available to the site.

Land Use

While much of the surrounding land in this area has developed over the past 25 years, this property as well as the surrounding properties to the north and east has remained undeveloped because sanitary sewer service was not available. In 2015 sewer was finally extended east of Scioto Reserve to service the Harvest Point project as well as this property. Because this site is situated at the signalized intersection of two major roadways, and because of the high-tension powerline and tower on the parcel, a single-family residential use is not the recommended use for the property. Accordingly, a mixed – use planned commercial project with institutional and multi-family residential components is being proposed for this site.

Proposed Project

This planned mixed-use project is proposed to provide neighborhood services to benefit the local residents of Powell and Liberty Township. The anchor of the project will be a new 25,000+/- square foot Delaware County District Library branch in Subarea A. Subarea B contains up to 14,400 square feet of retail and/or office space along the Home Road frontage to serve the community. Subarea C is a rental residential community of 30 townhomes and 32 garden apartments to provide quality housing options for those residents who choose to rent instead of own. Landscaped mounds are proposed around the perimeter of the property for screening from the adjacent residential properties. The retail and residential components of this project will comingle well with and will provide support for the library branch.

DEVELOPMENT TEXT

for

MIDDLEBURY CROSSING

April 2019

(1) Name, address, and phone number of applicant;

Applicant/Developer
John C. Wicks
Home Steitz, LLC
267 N. Liberty Street
Powell, OH 43065
614-323-7800
john.wicks@rpddllc.com

(2) Name, address, and phone number of registered surveyor, registered engineer and/or urban planner assisting in the preparation of the preliminary development plan;

Planning Professional
Gary B. Smith RLA/CLARB
G2 Planning and Design
720 E. Broad Street, Ste. 200
Columbus, OH 43215
614-390-6149

Engineering Professional

Tom Warner, P.E. Advanced Civil Design 422 Beecher Road Gahanna, OH 43230 614-428-7750

Surveying Professional

Douglas R. Hock, P.S. Advanced Civil Design 422 Beecher Road Gahanna, OH 43230 614-428-7750

Architectural Professional (Retail and Garden Apartments)

Pete Scott, AIA/CDT Meyers & Associates 232 N. Third Street, Ste. 300 Columbus, OH 43215 614-221-9433 <u>Architectural Professional (Townhomes)</u>

Joe Sullivan, AIA
Sullivan Bruck Architects, Inc.
8 South Grant Avenue
Columbus, OH 43215
614-464-9800

- (3) A list containing the names and mailing addresses of all owners of property contiguous to, directly across the street from and within 250 feet of the property in question. see Adjacent Property Owners
- (4) Legal description of the property; see Annexation/Boundary Survey and Legal Description
- (5) A description of present use(s) on and of the land; The property is presently being farmed with no other uses.
- (6) Draft of a proposed Ordinance, prepared with the advice and counsel of the Director Law, establishing this specific Development Plan as an additional effective zoning control over the land in question, consistent with the continuing authorities of the current Planned District zoning in these areas provided for elsewhere in this Zoning Ordinance. Will be submitted by the City of Powell
- (7) A vicinity map at a scale approved by the Zoning Inspector showing all property lines, existing streets and alleys, approved future streets and land uses on adjacent Planned District areas, transportation and land use elements of the Municipality's adopted Comprehensive Plan, current zoning classifications and boundaries, and current land uses on the site of the proposed Planned District development and in the surrounding areas to the physical extent deemed necessary by the Zoning Inspector, but no less than 250 feet beyond the limits of the proposed Planned District Development Plan. See Development Plan
- (8) A final development plan at a scale approved by the Zoning Administrator illustrating: See Development Plan
 - A. The property line definition and dimensions of the perimeter of the site;
 - B. Right-of-ways and paving widths of all existing, currently platted, and previously approved Planned District streets and alleys adjacent to, on, or abutting the site;
 - C. The area of the site and its subareas in acres;
 - D. The topography of the site and abutting areas at no more than five foot contour intervals;
 - E. Existing surface drainageways and surface sheet flow patterns;
 - F. Flood plain areas, ravine-bottom areas, and areas of ground slope in excess of six percent;
 - G. Existing vegetation on the site with specific tree spots for all trees six inches in diameter or greater, measured 24 inches from the ground.
 - H. Existing easements on the site with notations as to their type, extent, and nature;
 - I. The location and dimensions of existing utilities on and adjacent to the site, including the nearest sanitary sewer, with manhole invert elevations;
 - J. Calculation of the maximum residential units permitted on the site under the terms of this Zoning Ordinance, including delineation of the subdistricts of the site upon which these calculations have been made;
 - K. A final plan for the first, or next, phase of site development illustrating:
 - 1. New street centerlines, right-of-ways, and street classification types;
 - 2. Names of existing and proposed streets;

- 3. Generalized lot and block layouts, indicating and illustrating property lines, minimum lot areas, minimum building setbacks and yards, location and extent of major off-street parking areas, etc.;
- 4. Subareas of the site to be developed, by land use type, housing types, and housing densities, including subarea statistics;
- All proposed structures shall be located showing square footage, tenant or user types, and expected entranceways and service or loading areas;
- 6. Common open areas, public lands, and natural scenic easements, including the area of each;
- 7. Proposed landscape treatment of the site;
- Proposed utility patterns and provisions, including sanitary sewer, individual waste disposal systems, storm sewer, trash collection systems, outdoor lighting, and water supply, including relevant easements and engineering feasibility studies or other evidences of reasonableness;
- 9. Provisions for accommodating surface drainage runoff;
- 10. Proposed architectural design criteria;
- Proposed pedestrian/jogging/bicycle pathways and equestrian paths, including locations, dimensions, landscape and construction, including relationships of such pathways to existing and proposed future pathways on surrounding property;
- 12. Overall site development statistics comparing this plan for development with requirements of this Zoning Ordinance and with the comprehensive plan and indicating that all requirements of this Zoning Ordinance and the comprehensive plan have been met in this preliminary plan and will be met in final development.
- L. Projected development schedule by subareas of the entire planned development site, and for the first, or next, phase of development, including land uses, public areas, natural and scenic reserves, streets, buildings, utilities, and other facilities, indicating the relationship of the proposed development to existing and probable uses of surrounding areas during the development timetable; Below is the anticipated schedule for the planning, design and construction of Subareas B and C of the proposed development. Building construction will begin as close to the completion of infrastructure construction as possible.

FDP/Zoning Approval
Final Engineering Approval
Begin Site Construction
Complete Site Construction
Begin Building Construction

Spring 2019 Summer 2019 Summer/Fall 2019 Fall/Winter 2019/2020 Spring 2020

Subarea A will be submitted as a separate FDP by a different Applicant.

M. An overall traffic scheme, illustrating points of access, parking areas, including the number of parking spaces and indicating visitor, employee and service traffic flow, illustrating calculated peak hour traffic use for residents and employees as well as deliveries and other transport and the effect of this traffic on the community traffic ways. see *Traffic Impact Study*

- N. If to be developed in phases, the entire site development shall be described in outline and diagrammatic plan form, and in a complementing detailed text in a manner calculated to assure City officials that Planned Development requirements and other requirements of this Zoning Ordinance shall be met in the detailed development of the phases to follow, and that the entire Planned Development area will meet all of the requirements of this Zoning Ordinance, such diagrams and descriptive texts being accepted with, and becoming a part of the extended zoning plan for the entire site; See Development Plan
- (9) Evidences of the ability of the applicant to carry forth its plan by control of the land and the engineering feasibility of the plan, and that the applicant has sufficient control over the land and financing to initiate the proposed development plan phase within two (2) years. See Agency Letter for evidence of control over the property.

 Applicant shall provide financing evidence to City of Powell prior to the approval of Final Engineering.
- (10) Evidence of the applicant's ability to post a bond if the plan is approved. Applicant shall provide financing evidence to City of Powell prior to the approval of Final Engineering.
- (11) Verification by the owner of the property that all the information in the application is true and correct to the best of his knowledge. The Landowner and Applicant have reviewed the included information in the FDP submittal and believe it to be true and accurate to the best of their knowledge.
- (12) A statement of the character and nature of the development including the cost range or rent levels for housing in residential development and the general types of business or industrial and commercial developments. Developer's intended use of the Property is a neighborhood commercial development including but not limited to the following uses: Retail Sales; Convenience Business; Commercial Establishments; Civic Facilities including public library; Office Uses; General Business; Office Type Business; Office Research Centers; Services Business; Personal Services; Child Daycare; Agriculture; Restaurants; Medical or Dental Office Facilities; Hospitals and Clinics; Multifamily Residential housing.

The following uses shall be prohibited: Nuisance or hazardous materials of any kind whatsoever; Automotive Fuel, Repair or Service Stations; Automotive Sales, New or Used as a primary business; Nightclubs; Adult Entertainment or Bookstores; Storage facilities.

Residential rents are projected to be between \$1,050 and \$1,600 per month

Retail rents are projected to be between \$22 and \$28 per square foot.

All rents are subject to market conditions and construction costs.

(13) A statement of the general impact the development will have on the infrastructure, municipality and schools including projected demographics, a traffic impact study and a fiscal impact analysis may be required by the Planning and Zoning Commission. The Traffic Impact Study (TIS) outlines the public road improvements required for this this project. The impact to schools from multifamily residential homes at 0.15 students per unit is significantly less than the impact from single family residential homes at approximately 1.0 students per unit. This development would likely generate 9 to 10 students based on this formula.

(14) A fee as established by ordinance. A check in the amount of \$1,855.50 has been submitted with this application

Below are the Divergence Requests for deviations from zoning district standards which were approved with the Preliminary Development Plan:

The proposed Middlebury Crossing is a neighborhood mixed-use development which contains three different uses within one zoning application. As such, the project does not fully comply with the Planned Commercial (PC) development standards. The following Divergences to the PC standards are being requested in order to provide the different uses proposed with Middlebury Crossing.

- Rear Yard Setback. The PC code requires a minimum 30' rear yard setback. This project proposes a 25' perimeter setback for all residential buildings. The 25' setback is necessary due to the geometric restrictions of the site. Dense landscaping within the setback is proposed within the setback for maximum screening to adjacent properties.
- 2. <u>Building Separation</u>. The PC code requires a minimum of 50' between structures. The Planned Residential (PR) code requires a minimum of 30' between structures. This project proposes 20' between the townhome buildings in 2 locations due to the geometric restrictions of the site. The 20' provided is greater than the minimum 10' distance required by the Ohio Fire Code and is a customary building separation within multi-family developments. The retail buildings are proposed to be constructed as one structure with a continuous roof with breezeway, so no building separation divergence is requested.
- 3. Sign Setbacks. Per the zoning code, Joint Identification Signs (JIS) are required to be located 25' behind the right-of-way, and Freestanding Signs (FS) are required to be located 15' behind the right-of-way. This project proposes that the "Primary" JIS located at the intersection of Home and Steitz Roads to be 18' from the right-of-way, and the remaining signs on Home Road to be located 5' from the right-of-way. Considering Home Road's 100' right-of-way designation, combined with the fact that the centerline of the pavement is located south of the centerline of the right-of-way, the distance between the edge of pavement and the signs would be much greater than typical, making the signs difficult to see. The reduced sign setbacks will locate the signs approximately 40 feet from the edge of pavement which falls well outside the safety clear-zone requirements and places them in a more conventional location with respect to the road.
- 4. Parking in Front of Retail Buildings. The intent of the PC district is to place buildings close to the front setback providing parking in the rear. Due to the 150' overhead powerline easement, placing the retail buildings closer to Home Road is impractical and would create internal traffic circulation problems. With the site layout as proposed, the approved access locations function very well, providing excellent internal traffic circulation and cross-access between the different uses within the project.

Annexation Description 11.555 Acres North side of Home Road (C.R. 124) West side of Steitz Road (C.R. 125)

-1-

Situated in the State of Ohio, County of Delaware, Township of Liberty, Farm Lots 15 and 16, Quarter 2, Township 3, Range 19, being a 11.555 acre tract, said 11.555 acre tract being part of Parcel No. 2 as conveyed to TLK Development, LLC of record in Official Record 666, Page 198 and part of a 10.006 acre tract conveyed to Howard R. Vance of record in Official Record 1491, Page 2826 and more particularly described as follows:

Beginning at a northeasterly corner of said Parcel No. 2, being the northwesterly corner of a tract of land conveyed to Jack A Price and Eula Gay Price of record in Official Record 243, Page 481, being in the northerly line of Farm Lot 16 and the southerly line of Farm Lot 30;

Thence S 03° 26' 31" W, along an easterly line of said Parcel No. 2, 160.00 feet to a northeasterly corner thereof;

Thence S 87° 01' 02" E, along a northerly line of said Parcel No. 2, 109.50 feet to a northeasterly corner thereof;

Thence S 03° 22' 58" W, along an easterly line of said Parcel No. 2, 150.05 feet to a northeasterly corner thereof;

Thence S 87° 00' 57" E, along a northerly line of said Parcel No. 2, 245.30 feet to a point, said point being in the westerly right-of-way line of Steitz Road as shown on (Del. C.R. 124-2.77) state Right-of-Way plans;

Thence across said Parcel No. 2 and along said westerly right-of-way line, the following four (4) courses;

S 03° 27' 51" W, 81.77 feet;

S 07° 16' 41" W, 150.34 feet;

S 03° 27' 51" W, 199.99 feet;

S 27° 31' 34" W, 60.83 feet to a point in the northerly right-of-way line of said Home Road as shown on (Del. C.R. 124-2.77);

Thence S 03° 04' 35" W, across said Parcel No. 2, across said 10.006 acre tract and across said Home Road, 75.00 feet to a City of Powell Annexation Line (O.R. 826, Pg. 1463, Ord. No. 2007-58)

Thence N 86° 55' 25" W, across said 10.006 acre tract, with the northerly line of said City of Powell Annexation line, 715.53 feet to a point in the westerly line of said 10.006 acre tract;

Thence N 04° 02' 05" E, along the westerly line of said 10.006 acre tract, 30.00 feet to a northwesterly corner thereof, to the centerline of said Home Road and a common line of said Farm Lot 14 and said Farm Lot 15;

Thence S 86° 55' 25" E, along the center line of said Home Road and the common line of said Farm Lot 14 and said Farm Lot 15, 59.27 feet to a point;

Thence N 03° 26' 18" E, across said 10.006 acre tract, across said Home Road and along the westerly line of said Parcel No. 2, 841.02 feet to a northwesterly corner thereof, being in the northerly line of said Farm Lot 16 and the southerly line of Farm Lot 30;

Annexation Description 11.555 Acres North side of Home Road (C.R. 124) West side of Steitz Road (C.R. 125)

-2-

Thence S 87° 00' 57" E, along a northerly line of said Parcel No. 2 and along the common line of said Farm Lot 16 and Farm Lot 30, 335.57 feet to the True Point of Beginning, and containing 11.555 acres, more or less.

Subject, however, to all legal highways, easements, and restrictions. The above description was prepared by Advanced Civil Design, Inc. on April 23, 2018 and is based on existing records from the Delaware County Auditor's and Recorder's Office. A drawing of the above description is attached hereto and made a part thereof.

This description is to be used for annexation purposes only and not to be used in the transfer of land.

The total length of the annexation perimeter is 3214.17 feet, of which 715.53 feet are contiguous with existing City of Powell Corporation lines, being 22% contiguous.

This annexation does not create any islands of township property.

All references used in this description can be found at the Recorder's Office, Delaware County, Ohio.

ADVANCED CIVIL DESIGN, INC.

Douglas R. Hock, P.S. 7661

Date:

Z:\17-0005-579\survey\11.555 ac annexation desc (rev 04-23-18).doc

County Engineer Delaware County, Ohio

I hereby certify the within to be a true copy of the document that is on file in the Map Department.

CHRIS E. BAUSERMAN, P.E., P.S.

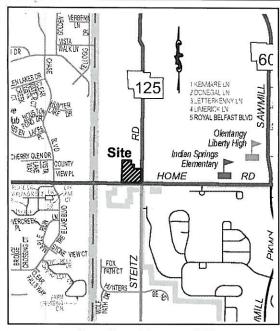
County Engineer

, Date <u>5/29/18</u>

_

R. HOCK S-7661

SONALS



Location Map - NTS

Map of Territory to be Annexed to the City of Powell

11.555 ACRE ANNEXATION TO THE CITY OF POWELL FROM TOWNSHIP OF LIBERTY EXPEDITED TYPE II ANNEXATION UNDER ORC §709.021 AND §709.023

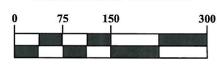
Township of Liberty, Delaware County, Ohio Farm Lots 15 & 16, Section 2, Township 3, Range 19, United States Military District

LEGEND

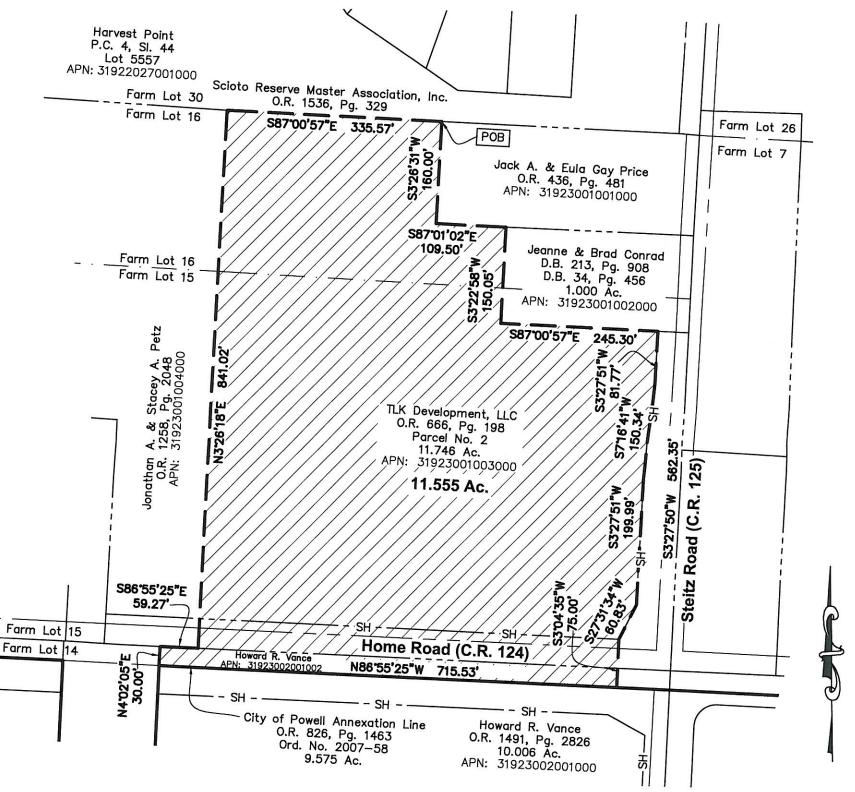
Area to be Annexed

Existing Corp Line Proposed Corp Line

GRAPHIC SCALE



1 inch = 150 feet



Notes:

1.) The number of owners in the territory sought to be annexed is one.

2.) TLK Development LLC, C/O Kim E. Kelsik is located at 5751 Kolb Road, Tucson, Arizona, 85750 and is the petitioner for said proposed annexation.

3.) No islands of unincorporated areas are created by this annexation as defined in ORC

709.023(E)(5).

422 Beecher Road

4.) 715.53' of lineal feet of Home Road (C.R. 124) will be annexed into the City of Powell.

Bearings are based on the Ohio State Plane Coordinate System, North Zone, NAD83 (NSRS2007). Said bearings were derived from GPS observation and determine a portion of the existing centerline of Home Road as having a bearing of N86°55'25"W.

This drawing is based on the existing records from the Delaware County Auditor's office and Delaware County Recorder's office and an actual field survey by Advanced Civil Design, Inc. in April of 2018. A boundary survey for this property has been approved and is on file in the Delaware County Map Department's office.

U

D 10x

STATEOFON

DOUGLAS

HOCK

Length of Contiguity: 715.53 feet Total Length of Perimeter: 3214.17 feet Percentage of Contiguity: 22%

Douglas R. Hock, P.S. 7661 Advanced Civil Design, Inc

Gahanna, OH 43230 Phone 614-428-7750 Job No.: 17-0005-579 Date: 04/23/2018

DELAWARE COUNTY ENGINEER Map Department

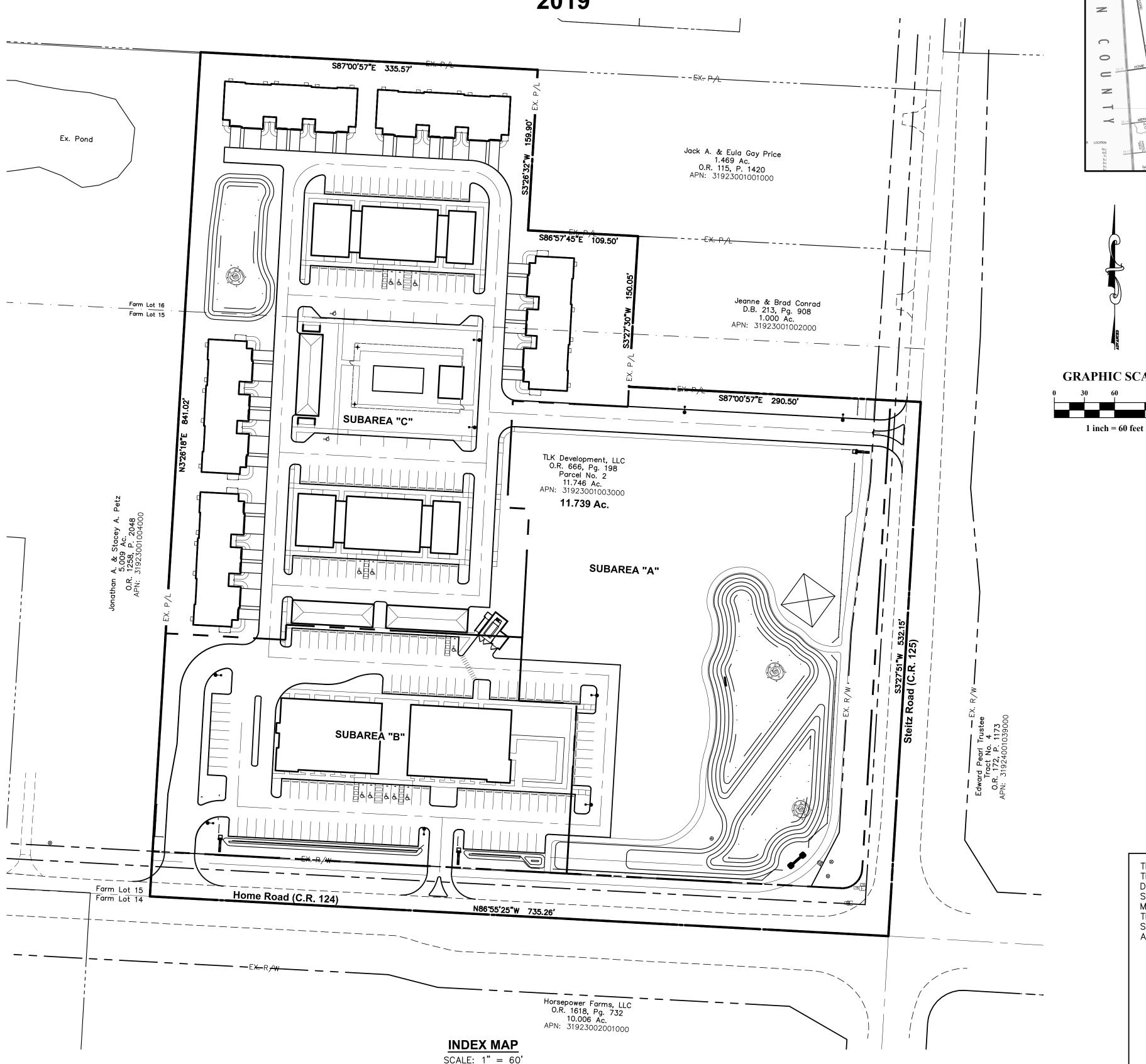
I hereby certify the within to be a true copy of the original on file in the Map Department

> Chris E. Bauserman, P.E., P.S., **County Engineer**

> > Supervisor

FINAL DEVELOPMENT PLAN **FOR** MIDDLEBURY CROSSING

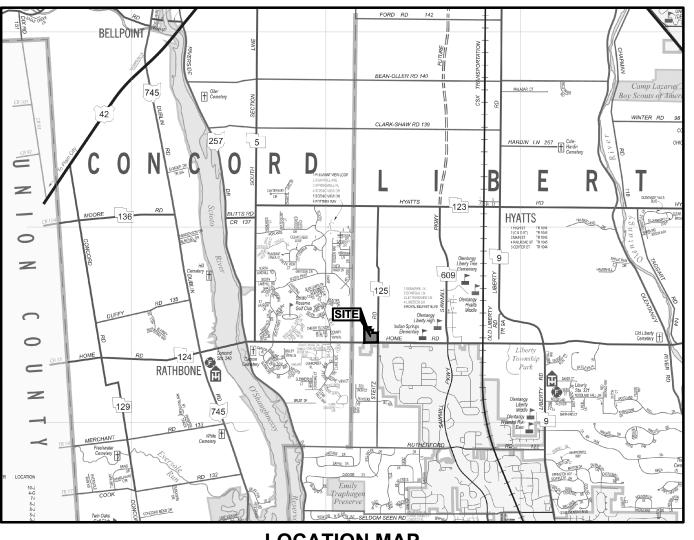
CITY OF POWELL, DELAWARE COUNTY, OHIO 2019



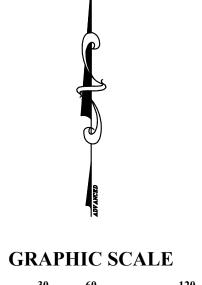
Utilities Protection

800-362-2764 or 8-1-1 www.oups.org

SERVICE



LOCATION MAP SCALE: 1" = 3500'



SHEET INDEX TITLE SHEET. GENERAL NOTES SHEET EXISTING CONDITIONS AND DEMOLITION PLAN SITE PLAN GRADING PLAN.. EX.B: SUB-AREA PLAN. EX.E: SITE LANDSCAPE PLAN EX.F2: SUB-AREA 'B' LANDSCAPE PLAN EX.F3: POOL AREA LANDSCAPE PLANS EX.G1: LANDSCAPE DETAILS.

DEVELOPER

EX.G2: LANDSCAPE DETAILS.

EX.G3: LANDSCAPE DETAILS. EX.H1: PHOTOMETRIC PLAN

267 N. LIBERTY STREET POWELL, OH 43065 PHONE: 614–323–7800

OWNER TKL DEVELOPMENT, LLC C/O LIM KELSIK 5751 KOLB ROAD TUSCON, AZ 85750 PHONE: 614-260-0649

LANDSCAPE ARCHITECT

G2 PLANNING AND DESIGN C/O GARY SMITH 720 E. BROAD STREET, SUITE 200 COLUMBUS, OH 43215 PHONE: 614-390-6149 GSMITH@G2PLANNING.COM

ENGINEER/SURVEYOR

ADVANCED CIVIL DESIGN, INC. C/O TOM WARNER 422 BEECHER ROAD GAHANNA, OHIO 43230 PHONE: 614-428-7750 TWARNER@ADVANCEDCIVILDESIGN.COM

SITE STATISTICS

HOME ROAD & STEITZ ROAD, DELAWARE COUNTY, OHIO SITE LOCATION: 040-0055551-00 & 040-005555-00 PARCEL NUMBER: GROSS ACREAGE: ±11.75 AC.

RIGHT-OF-WAY: ±1.20 AC. NET ACREAGE: ±10.55 AC.

SUBAREA "A" - FUTURE DELAWARE COUNTY LIBRARY BRANCH NET ACREAGE: ±3.71 AC.

SUBAREA "B" - COMMERCIAL ESTABLISHMENTS OR OFFICE

NET ACREAGE:

REGISTERED ENGINEER

SUBAREA "C" - MULTI-FAMILY RESIDENCES NET ACREAGE: 62 DWELLING UNITS

THIS IS TO CERTIFY THAT GOOD ENGINEERING PRACTICES HAVE BEEN UTILIZED IN THE DESIGN OF THIS PROJECT AND THAT ALL OF THE MINIMUM STANDARDS AS DELINEATED IN THE GROVE CITY DESIGN, CONSTRUCTION AND SURVEYING STANDARDS MANUAL HAVE BEEN MET, INCLUDING THOSE STANDARDS GREATER THAN MINIMUM WHERE, IN MY OPINION, THEY ARE NEEDED TO PROTECT THE SAFETY OF THE PUBLIC. ANY VARIANCES TO THE ABOVE STANDARDS ARE CONSISTENT WITH SOUND ENGINEERING PRACTICE AND ARE NOT DETRIMENTAL TO THE PUBLIC SAFETY AND CONVENIENCE.

ADVANCED CIVIL DESIGN, INC.

GAHANNA, OH 43230

422 BEECHER ROAD

V: 614.428.7750 F: 614.428.7755 DATE

Date: 04/22/2019

Scale: AS NOTED Drawn By: | Checked By:

FOR

CROSSING EVELOPMENT

BURY SE DEV

MIDDLEE MIXED US

Project Number: 19-0009-579

Drawing Number:

GENERAL NOTES

- THE REGULATIONS AND CONSTRUCTION STANDARDS OF THE CITY OF POWELL, TOGETHER WITH THE CURRENT CONSTRUCTION AND MATERIAL SPECIFICATIONS OF THE CITY OF COLUMBUS (COLS) AND THE OHIO DEPARTMENT OF TRANSPORTATION (ODOT), INCLUDING ALL SUPPLEMENTS THERETO, SHALL GOVERN ALL CONSTRUCTION ITEMS THAT ARE A PART OF THIS PLAN UNLESS OTHERWISE NOTED.
- 2. THE CITY ENGINEER OR HIS AUTHORIZED REPRESENTATIVE WILL MAKE INSPECTION OF THE WORK. THE CITY ENGINEER WILL REQUIRE AT LEAST 48 HOURS WRITTEN NOTICE BEFORE ANY WORK TAKES PLACE. FAILURE TO REQUEST THE NECESSARY INSPECTION MAY RESULT IN THE REJECTION OF THE WORK AND THE
- 3. IT IS THE INTENTION OF THE PLANS TO PROVIDE AND REQUIRE A COMPLETED PROJECT READY FOR OPERATION. ANY WORK ITEMS OMITTED FROM THE PLANS, WHICH ARE CLEARLY NECESSARY FOR COMPLETION OF THE WORK, AND ITS APPURTENANCES SHALL BE CONSIDERED A PART OF SUCH WORK, THOUGH NOT DIRECTLY SPECIFIED OR CALLED FOR IN THE PLANS. THIS INCLUDES, BUT IS NOT LIMITED TO SUCH INCIDENTAL ITEMS AS RELOCATION OF MAILBOXES, SAW CUTTING, AND REMOVAL AND/OR RELOCATION OF SIGNS, SPRINKLERS, OR OTHER MISCELLANEOUS ITEMS.
- 4. ALL ITEMS OF WORK CALLED FOR ON THE PLANS FOR WHICH NO SPECIFIC METHOD OF PAYMENT IS PROVIDED SHALL BE PERFORMED BY THE CONTRACTOR WITH THE COST TO BE INCLUDED IN THE UNIT PRICE BID FOR THE VARIOUS RELATED ITEMS.
- 5. THE CONTRACTOR OR DEVELOPER SHALL DEPOSIT THE TOTAL ESTIMATED COST FOR INSPECTIONS, AS DETERMINED BY THE CITY ENGINEER, WITH THE CITY OF POWELL PRIOR TO THE START OF CONSTRUCTION.
- THE CONTRACTOR SHALL PROVIDE THE CITY OF POWELL, A SURETY, ACCEPTABLE TO THE CITY OF POWELL, EQUAL TO 100% OF CONSTRUCTION COSTS. THE SURETY SHALL GUARANTEE THE WORK FOR ONE YEAR AFTER ACCEPTANCE BY THE CITY.
- 7. THE CITY ENGINEER WILL NOT BE RESPONSIBLE FOR MEANS, METHODS, PROCEDURES, TECHNIQUES, OR SEQUENCES OF CONSTRUCTION THAT ARE NOT SPECIFIED HEREIN. THE CITY ENGINEER WILL NOT BE RESPONSIBLE FOR SAFETY ON THE WORK SITE, OR THE FAILURE BY THE CONTRACTOR TO PERFORM WORK
- 8. APPROVAL OF THESE PLANS IS CONTINGENT UPON ALL EASEMENTS REQUIRED FOR CONSTRUCTION OF THE WORK BEING SECURED AND SUBMITTED TO THE CITY OF POWELL FOR RECORDING PRIOR TO COMMENCEMENT OF WORK. NO WORK, WHICH REQUIRES AN EASEMENT, WILL BE ALLOWED TO PROCEED UNTIL THIS IS DONE.
- 9. THE CONTRACTOR SHALL PERFORM ALL WORK IN ACCORDANCE WITH ALL APPLICABLE FEDERAL, STATE AND LOCAL SAFETY REQUIREMENTS INCLUDING THE OCCUPATIONAL SAFETY AND HEALTH ACT OF 1970. THE CONTRACTOR SHALL EXERCISE PRECAUTION ALWAYS FOR THE PROTECTION OF PERSONS (INCLUDING EMPLOYEES) AND PROPERTY. IT SHALL ALSO BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO INITIATE, MAINTAIN AND SUPERVISE ALL SAFETY REQUIREMENTS, PRECAUTION AND PROGRAMS IN CONNECTION WITH THE WORK, INCLUDING THE REQUIREMENTS FOR CONFINED SPACES PER 29 CFR 1910.146.
- 10. THE CONTRACTOR/DEVELOPER SHALL BE RESPONSIBLE TO OBTAIN ALL NECESSARY PERMITS.
- 11. THE CONTRACTOR SHALL CONFINE HIS ACTIVITIES TO THE PROJECT SITE, EXISTING RIGHT-OF-WAYS, TEMPORARY AND PERMANENT EASEMENTS, AND SHALL NOT ENTER UPON OTHER PROPERTIES WITHOUT WRITTEN PERMISSION OF THE OWNER. IF THE PROPOSED WORK REQUIRES ENTERING EASEMENTS UPON OTHER PROPERTIES, THE CONTRACTOR SHALL NOTIFY THE OWNER(S) IN WRITING NO LESS THAN 72 HOURS IN ADVANCE OF THE COMMENCEMENT OF THE WORK, AND COPY THE CITY ON ALL CORRESPONDENCE. FAILURE TO NOTIFY AFFECTED PROPERTY OWNERS MAY SUBJECT. THE CONTRACTOR TO THE PENALTIES ASSOCIATED WITH THE VIOLATION OF POWELL CITY CODE, SECTION 541.05, CRIMINAL TRESPASS.
- 12. THE CONTRACTOR SHALL CAREFULLY PRESERVE BENCHMARKS, PROPERTY CORNERS, REFERENCE POINTS, STAKES AND OTHER SURVEY REFERENCE MONUMENTS OR MARKERS. IN CASES OF WILLFUL OR CARELESS DESTRUCTION, THE CONTRACTOR SHALL BE RESPONSIBLE. RESETTING THE MARKERS SHALL BE PERFORMED BY AN OHIO PROFESSIONAL SURVEYOR AS APPROVED BY THE CITY ENGINEER AT THE CONTRACTOR'S
- 13. PROPERTY BOUNDARIES, INCLUDING PROPERTY LINES AND ROAD RIGHT-OF-WAY, ARE SHOWN FROM THE BEST INFORMATION AVAILABLE AND ARE NOT NECESSARILY COMPLETE OR CORRECT.
- 14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR HAVING THE FINISHED WORK CONFORM TO THE LINES. GRADES, ELEVATIONS AND DIMENSIONS CALLED FOR ON THE DRAWINGS AND TYPICAL SECTIONS.
- 15. ANY DEVICE SHALL NOT BE OPERATED AT ANY TIME IN SUCH A MANNER THAT THE NOISE CREATED SUBSTANTIALLY EXCEEDS THE NOISE CUSTOMARILY AND NECESSARILY ATTENDANT TO THE REASONABLE AND EFFICIENT PERFORMANCE OF SUCH EQUIPMENT. PURSUANT TO POWELL CITY CODE, SECTION 509.08. CONSTRUCTION ACTIVITY IS ONLY PERMITTED BETWEEN THE HOURS OF 7:30 A.M. AND 7:00 P.M. ANY CONSTRUCTION ACTIVITY BEYOND THESE HOURS REQUIRES A WRITTEN REQUEST TO THE DIRECTOR OF PUBLIC SERVICE MEETING THE REQUIREMENTS OF POWELL CITY CODE, SECTION 509.08(B).
- 16. PAVEMENTS SHALL BE CUT IN NEAT, STRAIGHT LINES THE FULL DEPTH OF THE EXISTING PAVEMENT OR AS REQUIRED BY THE CITY ENGINEER.
- 17. ALL SOIL SUBGRADE SHALL BE PREPARED AND COMPACTED IN ACCORDANCE WITH ITEM 204 TO A DEPTH OF 12 INCHES BELOW THE SUBGRADE SURFACE. SUBGRADE SHALL BE SCARIFIED AND CONTAIN SUFFICIENT MOISTURE TO MEET ITEM 203 COMPACTION REQUIREMENTS.
- 18. THE CONTRACTOR IS NOT PERMITTED TO USE ANY RECLAIMED MATERIALS IN ITEM 304.
- 19. NON-RUBBER TIRED VEHICLES SHALL NOT BE MOVED ON OR ACROSS PUBLIC STREETS OR HIGHWAYS WITHOUT THE WRITTEN PERMISSION OF THE CITY ENGINEER.
- 20. TRACKING OR SPILLING MUD, DIRT, OR DEBRIS UPON STREETS, RESIDENTIAL OR COMMERCIAL DRIVES, SIDEWALKS OR BIKE PATHS IS PROHIBITED PER POWELL CITY CODE. SECTION 905.12 AND ANY SUCH OCCURRENCE SHALL BE CLEANED UP IMMEDIATELY BY THE CONTRACTOR. IF THE CONTRACTOR FAILS TO REMOVE SAID MUD, DIRT, DEBRIS, OR SPILLAGE, THE CITY OF POWELL RESERVES THE RIGHT TO REMOVE THESE MATERIALS AND CLEAN AFFECTED AREAS, THE COST OF WHICH SHALL BE PAID BY THE CONTRACTOR/DEVELOPER PER POWELL CITY CODE, SECTION 905.13.
- 21. DURING CONSTRUCTION THE CONTRACTOR SHALL PROVIDE ADEQUATE DRAINAGE AND PROPER SOIL EROSION CONTROL MEASURES FOR PROTECTION OF ALL ADJACENT ROADS AND LANDS, PER COLS ITEM
- 22. THE CONTRACTOR SHALL RESTORE ALL DISTURBED AREAS TO EQUAL OR BETTER THAN EXISTED BEFORE CONSTRUCTION. DRAINAGE DITCHES OR WATERCOURSES THAT ARE DISTURBED BY CONSTRUCTION SHALL BE RESTORED TO THE GRADES AND CROSS-SECTIONS THAT EXISTED BEFORE CONSTRUCTION.
- 23. THE CONTRACTOR SHALL CONDUCT HIS OPERATIONS SO AS TO MAINTAIN AT ALL TIMES SEWER, DRAIN, AND DITCH FLOWS THROUGH EXISTING FACILITIES TO REMAIN IN PLACE AND THROUGH EXISTING FACILITIES TO BE REPLACED UNTIL NEW FACILITIES ARE COMPLETED AND PUT INTO SERVICE. THE CONTRACTOR, TO A CONDITION SATISFACTORY TO THE CITY ENGINEER, SHALL RESTORE THE FLOW OF ALL SEWERS, DRAINS, AND OTHER WATERCOURSES DISTURBED DURING THE PROSECUTION OF THE WORK.
- 24. ANY MODIFICATION OF THE WORK AS SHOWN ON THESE DRAWINGS MUST HAVE PRIOR WRITTEN APPROVAL BY THE POWELL CITY ENGINEER.
- 25. THE CONTRACTOR SHALL CALL TOLL FREE, THE OHIO UTILITIES PROTECTION SERVICE (OUPS) AT 1-800-362-2764 SEVENTY-TWO (72) HOURS IN ADVANCE OF THE ANTICIPATED START OF CONSTRUCTION, AND SHALL NOTIFY ALL UTILITY COMPANIES AT LEAST FORTH-EIGHT (48) HOURS PRIOR TO WORK IN THE VICINITY OF THEIR LINES.

UTILITY	OWNER	TELEPHONE
TELEPHONE	AT&T 111 NORTH FRONT STREET COLUMBUS, OH 43215 CONTACT: RON C HARRISON	(614) 223-6790
ELECTRIC	AMERICAN ELECTRIC POWER 850 TECH CENTER DRIVE GAHANNA, OH 43230—6605 CONTACT: ANDY WAINWRIGHT	(614) 883–6821
GAS	COLUMBIA GAS 920 WEST GOODALE BLVD. COLUMBUS, OH 43215 CONTACT: JODY BEAVER	(614) 460-5400
CABLE TELEVISION	TIME WARNER CABLE 1266 DUBLIN ROAD P.O. BOX 2553 COLUMBUS, OH 43216-2553 CONTACT: GREG MILLER	(614) 348–1539

- 26. EXISTING UTILITIES SHOWN ON THE PLAN ARE FROM THE BEST AVAILABLE RECORDS AND FIELD INVESTIGATION AND ARE NOT NECESSARILY COMPLETE OR CORRECT. THE CONTRACTOR IS RESPONSIBLE FOR THE INVESTIGATION, LOCATION, SUPPORT, PROTECTION AND RESTORATION OF ALL EXISTING UTILITIES AND APPURTENANCES WHETHER SHOWN OR NOT.
- 27. THE CONTRACTOR SHALL EXPOSE AND VERIFY THE LOCATION AND ELEVATION OF ANY UTILITIES WITHIN THE LIMITS OF THE PROPOSED CONDUIT PATH, PRIOR TO STARTING ANY EXCAVATION. IN CASE OF CONFLICT, ADJUSTMENTS IN LOCATION AND ELEVATION OF THE PROPOSED UTILITIES MAY BE MADE IF APPROVED PER GENERAL NOTE #24, OR ARRANGEMENTS SHALL BE MADE TO MOVE THE EXISTING UTILITY TO PROVIDE ADEQUATE CLEARANCE.

- 28. MAINTAIN THREE (3) FEET HORIZONTAL AND ONE (1) FOOT VERTICAL SEPARATION FROM ALL SEWER AND
- 29. EXISTING DRAIN TILES ENCOUNTERED DURING CONSTRUCTION SHALL BE RECONNECTED OR CONNECTED TO THE STORM SEWER SYSTEM BY THE CONTRACTOR, AS APPROVED BY THE CITY ENGINEER. THE COST OF SAID WORK TO BE INCLUDED IN THE UNIT PRICE BID FOR THE VARIOUS RELATED ITEMS.
- 30. ALL TRENCHES WITHIN PAVEMENT, BERM, AND SHOULDER LIMITS SHALL BE BACKFILLED OR SECURELY PLATED DURING NON-WORKING HOURS.
- 31. ACCESS TO ALL ADJOINING PROPERTIES SHALL BE MAINTAINED AT ALL TIMES. AREAS WITH MULTIPLE DRIVES SHALL HAVE AT LEAST HALF OF THE DRIVES OPEN AT ALL TIMES. PROPERTIES WITH A SINGLE ACCESS WILL REQUIRE STAGED CONSTRUCTION; SHORT-TERM FULL CLOSURE OF A SINGLE ACCESS WILL BE PERMITTED WITH THE PROPERTY OWNER AND/OR TENANT'S AGREEMENT. SUCH FULL CLOSURES SHALL BE SCHEDULED AND COORDINATED WITH THE PROPERTY OWNER/TENANT.
- 32. AT ALL UTILITY CROSSINGS THE TRENCH BACKFILL SHALL CONSIST OF COMPACTED GRANULAR MATERIAL COLS ITEM 912, BETWEEN THE DEEPER AND SHALLOWER PIPE.
- 33. COMPACTED GRANULAR MATERIAL, COLS ITEM 912 SHALL CONSIST OF NATURAL, BROKEN OR CRUSHED STONE, CRUSHED GRAVEL, OR CRUSHED SLAG. SYNTHETIC OR MAN-MADE MATERIALS ARE UNACCEPTABLE.
- 34. BACKFILL FOR TRENCHES UNDER PAVEMENT AND WITHIN THE RIGHT-OF-WAY SHALL BE COMPACTED GRANULAR MATERIAL, COLS ITEM 912, TO THE PAVEMENT SUBGRADE. WHERE TRENCHES CROSS THE PAVEMENT, COLS ITEM 912 SHALL EXTEND THE FULL WIDTH OF THE RIGHT-OF-WAY, AND TO WITHIN 6 INCHES OF FINISHED GRADE WHERE NOT UNDER PAVEMENT.
- 35. BACKFILL FOR TRENCHES PARALLEL TO THE PAVEMENT AND WITHIN THE RIGHT-OF-WAY, WITH THE TOP OF THE TRENCH 3 FEET OR CLOSER TO THE BACK OF CURB OR EDGE OF PAVEMENT OR LOCATED UNDER A PEDESTRIAN PATHWAY, SHALL BE COMPACTED GRANULAR MATERIAL, COLS ITEM 912, TO WITHIN 6 INCHES OF FINISHED GRADE. BACKFILL FOR ALL OTHER TRENCHES WITHIN THE RIGHT-OF-WAY PARALLEL TO THE PAVEMENT, SHALL BE SHALL BE COMPACTED BACKFILL, COLS ITEM 911, EXCEPT THAT COMPACTION SHALL BE TO MINIMUM 9 PERCENT MAXIMUM DRY DENSITY. PRIOR TO CONSTRUCTION OF THE STREETS, THE CITY ENGINEER MAY REQUIRE SOIL TESTS ON THE BACKFILL. WHERE TEST RESULTS INDICATE THAT THE BACKFILL DOES NOT MEET COMPACTION REQUIREMENTS THE BACKFILL SHALL BE REMOVED, REPLACED, AND RE-TESTED UNTIL MEETING THOSE REQUIREMENTS.
- 36. THE CONTRACTOR SHALL INSTALL STREET LIGHTS AT THE LOCATIONS SHOWN ON THE PLANS, INCLUDING ALL WIRING AND DISCONNECTS AND PROVIDE A COMPLETE OPERATING LIGHTING SYSTEM THAT COMPLIES WITH THE CITY OF POWELL SPECIFICATIONS.
- 37. ALL AREAS FOR UNDERGROUND ELECTRIC AND STREET LIGHTING ELECTRIC, TELEPHONE, AND CABLE TV INSTALLATIONS SHALL BE BROUGHT TO FINISHED GRADE, AS SHOWN ON THE GRADING PLAN, PRIOR TO THEIR BEING INSTALLED. ALL FILL REQUIRED SHALL BE COMPACTED IN ACCORDANCE WITH COLS ITEM 203.12, CONDITION 1. THIS WORK SHALL BE PERFORMED AS PART OF THIS PLAN AND THE COST SHALL BE INCLUDED UNDER ITEM 203.
- 38. ALL SEEDING SHALL BE APPLIED AT THE RATE OF 8 POUNDS (LB.) PER 1,000 SQUARE FEET (SF) AND SHALL USE THE FOLLOWING SEED MIXTURE:
 - 40% TITIAN TALL FESCUE 40% TARHEEL TALL FESCUE 10% DENIM KENTUCKY BLUEGRASS
 - 10% RENAISSANCE PERENNIAL RYE GRASS
- 39. COMMERCIAL-GRADE COMPLETE FERTILIZER OF NEUTRAL CHARACTER, CONSISTING OF FAST, AND SLOW RELEASE NITROGEN. 50% DERIVED FROM NATURAL ORGANIC SOURCES OF UREA-FORM, PHOSPHOROUS, AND POTASSIUM AND WITH THE FOLLOWING COMPOSITION SHALL BE APPLIED:

COMPOSITION: 13% NITROGEN, 26% PHOSPHOROUS, AND 12% POTASSIUM BY WEIGHT

- FERTILIZER SHOULD BE APPLIED AT THE RATE OF 6 POUNDS (LB.) PER 1,000 SQUARE FEET (SF).
- 40. ALL SIGNS, LANDSCAPING, STRUCTURES OR OTHER APPURTENANCES DISTURBED OR DAMAGED DURING CONSTRUCTION SHALL BE REPLACED OR REPAIRED TO THE SATISFACTION OF THE CITY ENGINEER. THE CONTRACTOR SHALL PAY FOR THE COST OF THIS WORK.

GENERAL NOTES FOR STORM SEWERS

- 1. ANY MODIFICATION OF THE WORK AS SHOWN ON THESE DRAWINGS MUST HAVE PRIOR WRITTEN APPROVAL
- 2. THE PROPOSED SANITARY SEWERS AND SERVICES ARE TO BE CONSTRUCTED UNDER PLAN P PRIOR TO STREET IMPROVEMENTS.
- 3. BEFORE THE CONTRACTOR STARTS ANY WORK ON THE PROJECT AND AGAIN BEFORE FINAL ACCEPTANCE OF ANY WORK, REPRESENTATIVES OF THE CITY AND THE CONTRACTOR SHALL MAKE AN INSPECTION OF ALL EXISTING SEWERS THAT ARE TO REMAIN IN SERVICE AND WHICH MAY BE AFFECTED BY THE WORK. THE CONDITION OF THE EXISTING CONDUITS AND THEIR APPURTENANCES SHALL BE DETERMINED FROM FIELD OBSERVATIONS. THE CITY SHALL KEEP RECORDS OF THE INSPECTION IN WRITING.
- 4. ALL EXISTING SEWERS INSPECTED INITIALLY BY THE ABOVE MENTIONED PARTIES SHALL BE MAINTAINED AND LEFT IN A CONDITION REASONABLY COMPARABLE TO THAT DETERMINED BY THE ORIGINAL INSPECTION. THE CONTRACTOR, TO THE SATISFACTION OF THE CITY ENGINEER, SHALL CORRECT ANY CHANGE IN THE CONDITION RESULTING FROM THE CONTRACTOR'S OPERATIONS.
- 5. PAYMENT FOR ALL OPERATIONS DESCRIBED ABOVE SHALL BE INCLUDED IN THE CONTRACT PRICE FOR THE PERTINENT CONTRACT ITEMS.
- 6. ALL STORM SEWERS CONSTRUCTED UNDER THIS PLAN SHALL MEET THE REQUIREMENTS OF COLS ITEM
- 7. ALL NEW CONDUITS, CATCH BASINS AND MANHOLES CONSTRUCTED, AS A PART OF THE PROJECT SHALL BE FREE OF ALL FOREIGN MATTER AND IN A CLEANED CONDITION BEFORE THE CITY WILL ACCEPT THE
- 8. ALL INLETS, CATCH BASINS, AND MANHOLES SHALL BE CHANNELIZED.

901, WITH A MINIMUM INSIDE DIAMETER OF 12 INCHES.

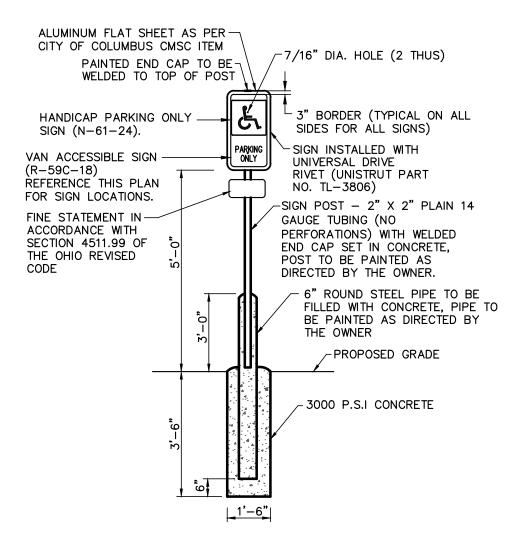
- 9. ROADWAY UNDERDRAINS SHALL BE DISCHARGED INTO THE NEAREST STORM STRUCTURE AVAILABLE ALONG THE LINE OF FLOW UNLESS SHOWN OTHERWISE ON THE PLANS.
- 10. THE CONTRACTOR SHALL ADJUST ALL EXISTING AND PROPOSED CASTINGS TO MATCH THE SURROUNDING FINISHED GRADE. THE COST OF ALL CASTING ADJUSTMENTS SHALL BE INCLUDED IN THE VARIOUS SEWER
- 11. ALL DRAINAGE SWALES ALONG REAR LOT LINES, SHOWN ON THE GRADING PLAN, SHALL BE CONSTRUCTED TO FINISHED GRADE AS PART OF THIS PLAN AND THE COST SHALL BE INCLUDED UNDER COLS ITEM 203.
- 12. ALL STORM WATER DETENTION/RETENTION AREAS SHOWN ON THE GRADING PLAN SHALL BE CONSTRUCTED TO FINISHED GRADE PER COLS ITEM 203, HYDRO-SEEDED, AND HYDRO-MULCHED PER COLS ITEM 659, AS PART OF THIS PLAN. THE COST SHALL BE INCLUDED AS A LUMP SUM UNDER SPECIAL, DETENTION/RETENTION AREA CONSTRUCTION.
- 13. THE CONTRACTOR SHALL PROVIDE TWO ROOF DRAIN OPENINGS IN THE CURB FOR EACH LOT, LOCATED AS DIRECTED BY THE OWNER.
- 14. ALL CATCH BASINS ARE TO BE EQUIPPED WITH EAST JORDAN #5110, TYPE M3 GRATES, OR APPROVED
- 15. UPON COMPLETION OF CONSTRUCTION FOR EACH PHASE OF STORM SEWER WORK, THE DEVELOPER, THROUGH ITS ENGINEER, SHALL FURNISH THE CITY ENGINEER A TABULATION OF STRUCTURE NUMBERS, THE ELEVATION OF THE TOP OF CASTING AS PROPOSED ON THE PLANS, AND THE ELEVATION OF THE TOP OF CASTING. AS BUILT, FURTHER, A TABULATION OF STATIONING AND TOP OF CURB FLEVATION, AS BUILT, AT THE ENDS OF ALL STREETS, THAT ARE TO BE EXTENDED IN THE FUTURE, SHALL BE SUBMITTED. THE CITY ENGINEER WILL DETERMINE ADJUSTMENTS, IF ANY, THAT ARE NECESSARY AND ALL NECESSARY ADJUSTMENTS SHALL BE DONE PRIOR TO SUBMITTAL OF "AS BUILT" DRAWINGS.
- 16. THE ORIGINAL TRACINGS, REVISED "AS BUILT", AND TWO SETS OF PRINTS SHALL BE GIVEN TO THE CITY PRIOR TO ANY TAP PERMITS BEING ISSUED, OR ACCEPTANCE BY THE CITY FOR THE ONE YEAR MAINTENANCE PERIOD. THE INFORMATION SHOWN ON THE "AS BUILT" PLANS SHALL BE FROM FIELD MEASUREMENTS. WATER SERVICES AND MAIN LINE VALVES SHALL BE LOCATED BY STREET STATIONING. TOP OF CASTING ELEVATIONS FOR ALL STORM SEWER STRUCTURES AND ANY VARIANCE IN THE HORIZONTAL LOCATION OF THE UTILITIES FROM THAT SHOWN ON THE APPROVED PLANS, SHALL BE SHOWN.

DEL-CO WATER COMPANY

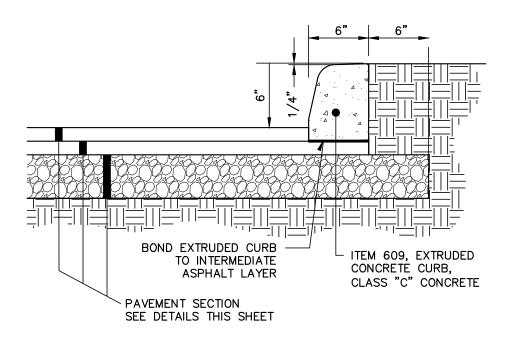
- GENERAL NOTES FOR WATER LINE CONSTRUCTION REVISED: MAY 3, 2016 1. GENERAL NOTES AS MODIFIED BY DEL-CO AND SHOWN ON THE APPROVED CONSTRUCTION DRAWINGS SHALL SUPERSEDE THE REQUIREMENTS OF THE DEL-CO WATER COMPANY CONSTRUCTION STANDARDS MANUAL WHEREVER DISCREPANCIES OCCUR.
- STANDARD GENERAL NOTES:
- A. WATER LINE DESIGN, MATERIALS, AND INSTALLATION METHODS SHALL CONFORM TO APPLICABLE SECTIONS OF RECOMMENDED STANDARDS FOR WATER WORKS (TEN STATES STANDARDS), AMERICAN WATER WORKS ASSOCIATION (AWWA) STANDARDS, AND THE DEL-CO WATER COMPANY CONSTRUCTION STANDARDS MANUAL. CONTRACTOR SHALL OBTAIN A COPY OF THE STANDARDS AND HAVE IN THEIR POSSESSION AT ALL TIMES DURING CONSTRUCTION. COORDINATE WORK WITH DEL-CO WATER (740) 548-7746.
- DEL-CO.'S SIGNATURE ON THIS PLAN SIGNIFIES ONLY CONCURRENCE WITH THE GENERAL PURPOSE AND LOCATION OF THE PROPOSED WATER LINE IMPROVEMENTS. ALL TECHNICAL DETAILS REMAIN THE RESPONSIBILITY OF THE PROFESSIONAL ENGINEER WHO PREPARED AND CERTIFIED THESE PLANS. DEL-CO WATER COMPANY TAKES NO RESPONSIBILITY, FINANCIAL OR OTHERWISE, REGARDING ERRORS IN THIS PLAN.
- CORRECTION OF ERRORS SHALL BE TO THE APPLICABLE DEL-CO WATER COMPANY STANDARD, AND THE SOLE RESPONSIBILITY OF THE DEVELOPER. ALL CORRECTIONS, OR REVISIONS THAT AFFECT DEL-CO.'S WATER LINE PLANS, DIRECTLY OR INDIRECTLY SHALL BE SUBMITTED TO, AND APPROVED BY DEL-CO WATER COMPANY PRIOR TO REVISIONS BEING ISSUED.
- GPS COORDINATES SHALL BE PROVIDED TO DEL-CO WATER AT THE COMPLETION OF THE WATERLINE INSTALLATION. THESE COORDINATES SHALL INCLUDE ALL MATERIALS, EQUIPMENT AND LABOR NECESSARY TO OBTAIN HORIZONTAL AND VERTICAL (NORTHING, EASTING AND ELEVATION) SURVEY COORDINATES FOR THE WATER MAIN IMPROVEMENTS. THE SURVEY COORDINATES SHALL BE OBTAINED AT THE COMPLETION OF THE WATER MAIN INSTALLATION AND SHALL INCLUDE ALL VALVES, TEES, FIRE HYDRANTS, BENDS, PLUGS, REDUCERS, TAPPED TEES, CURB STOPS, AIR RELEASES, 2" END OF LINE FIRE HYDRANTS, ENDS OF CASING PIPE, SERVICE SADDLES AND CORPORATIONS. ADDITIONAL GPS COORDINATES ARE REQUIRED ON THE WATER MAIN EVERY 200' WHERE NO FITTINGS OR SERVICE SADDLES ARE TO BE
- GPS COORDINATES SHALL BE REFERENCED TO THE APPLICABLE COUNTY ENGINEER'S MONUMENTS AND SHALL BE BASED ON THE NORTH AMERICAN DATUM OF 1983 (NAD 83) WITH THE NSRS2007 ADJUSTMENT, WITH FURTHER REFERENCE MADE TO THE OHIO STATE PLANE NORTH COORDINATE SYSTEM, NORTH ZONE, WITH ELEVATIONS BASED ON NAVD 88 DATUM. ALL COORDINATES (NORTHING, EASTING AND ELEVATION) SHALL BE REFERENCED TO THE NEAREST HUNDREDTH. ALL SURVEY COORDINATES SHALL BE ACCURATE TO WITHIN 0.6 FOOT OR LESS HORIZONTAL AND VERTICAL.
- THE GPS COORDINATES SHALL BE SUBMITTED TO THE DEL-CO WATER ENGINEERING DEPARTMENT IN DIGITAL SPREADSHEET FORM AND SHALL INCLUDE THE APPLICABLE ITEM, STATION, NORTHING, EASTING AND ELEVATION COORDINATES. THE ABOVE LISTED GPS COORDINATE INFORMATION SHALL BE SUBMITTED TO THE DEL-CO WATER ENGINEERING DEPARTMENT AS PART OF THE AS-BUILT DRAWING SUBMITTAL.
- AS-BUILT DRAWINGS ARE REQUIRED FOLLOWING THE COMPLETION OF CONSTRUCTION. ONE SET OF DRAWINGS MARKED "AS-BUILT" SHALL BE SUBMITTED BY THE DEVELOPER TO DEL-CO'S INSPECTION DEPARTMENT FOR REVIEW AND APPROVAL. WATER MAINS CANNOT RECEIVE A FINAL ACCEPTANCE UNTIL AS-BUILT DRAWINGS HAVE BEEN APPROVED. PLEASE NOTE: TAPS MAY NOT BE PURCHASED NOR INSTALLED UNTIL WATER LINES HAVE RECEIVED A FINAL ACCEPTANCE.
- F. WATER MAINS SHALL BECOME THE OWNERSHIP OF DEL-CO WATER UPON FINAL ACCEPTANCE.
- WATER LINE CONSTRUCTION PLANS ARE APPROVED FOR A PERIOD OF ONE YEAR FROM THE DATE OF THE APPROVAL LETTER OR SIGNED PLANS. IF CONSTRUCTION HAS NOT STARTED WITHIN ONE YEAR OF THE DATE OF APPROVAL, PLANS SHALL BE RESUBMITTED TO DEL-CO WATER COMPANY FOR APPROVAL.
- WATER LINE CONSTRUCTION: A. WATER LINES SHALL BE NSF 61 APPROVED, AND COMPLIANT WITH ASTM D2241 & OHIO EPA ENG-08-001
- B. USE THE FOLLOWING TYPE AND CLASS OF PIPE UNLESS OTHERWISE INDICATED ON THE DRAWINGS: I. 2-INCH WATER LINE PIPE: CLASS 200 SDR 21 YELOMINE PVC (RESTRAINED JOINT).
- II. 4-INCH WATER LINE PIPE: CLASS 200 SDR 21 PVC.
- III. 6-INCH WATER LINE PIPE: CLASS 200 SDR 21 PVC.
- IV. 8-INCH TO 12-INCH WATER LINE PIPE: CLASS 160 SDR 26 PVC. V. 16-INCH AND LARGER WATER LINE PIPE: AWWA C151 CLASS 52 DIP.
- VI. 4-INCH PIPE AND LARGER USED FOR FIRE SERVICE: AWWA C900 DR 18 (150 PSI) PVC.
- VII. ALL SIZES OF DEL-CO-OWNED WATER LINES USED ON MASTER METER PROJECTS: CLASS 200 SDR 21
- C. ALL FITTINGS SHALL BE MECHANICAL JOINT CONFORMING TO AWWA C153.
- D. CROSSES SHALL NOT BE USED WITHOUT APPROVAL OF DEL—CO WATER COMPANY.
- ALL VALVES SHALL BE MECHANICAL JOINT CONFORMING TO AWWA WITH AISI 304 STAINLESS STEEL EXTERNAL HARDWARE. VALVES 12-INCH AND SMALLER SHALL BE RESILIENT-SEATED GATE VALVES PER AWWA C509 AND VALVES 16-INCH AND LARGER SHALL BE PRATT GROUNDHOG BUTTERFLY VALVES PER
- PROVIDE HEAVY-DUTY VALVE BOXES ON ALL HOT-TAPS AND AT VALVES LOCATED UNDER GRAVEL OR
- TOP OF VALVE BOX SHALL BE FLUSH WITH FINISHED GRADE IN PAVED AREAS, AND 4 INCHES ABOVE FINISHED GRADE IN NON-PAVED AREAS.
- H. MAINTAIN A MINIMUM 10-FOOT HORIZONTAL AND 1.5-FOOT VERTICAL SEPARATION BETWEEN WATER LINES AND SANITARY AND STORM SEWERS.
- ALL OTHER BURIED UTILITIES SHALL MAINTAIN A MINIMUM 5-FOOT HORIZONTAL SEPARATION. AND 2-FOOT
- VERTICAL SEPARATION FROM THE CENTERLINE OF WATER LINES AS FINALLY LAID AND CONSTRUCTED.
- J. PROVIDE CONCRETE THRUST BLOCKING FOR ALL FITTINGS, VALVES, ANCHOR TEES, AND HYDRANTS. K. BURY WATER LINES A MINIMUM DEPTH OF 48-INCHES TO THE TOP OF PIPE.
- L. ALL ENGINEERED FILL TO BE PLACED OVER OR UNDER DEL-CO WATER LINES SHALL BE IN PLACE PRIOR TO THE CONSTRUCTION OF THE WATER LINES.
- M. PLACE A 5-FOOT STEEL FENCE POST OR 4"X4" WOOD POST AT VALVES AND THE ENDS OF LINES. PAINT
- N. TRACER WIRE:
- INSTALL COPPERHEAD® OR EQUAL 12-GAUGE HIGH STRENGTH 452LB BREAK STRENGTH 30 MIL HDPE JACKET, COPPER-CLAD, STEEL REINFORCED TRACER WIRE ON ALL WATER MAIN AND SERVICE LINES INSTALLED BY TRENCHING METHODS.
- INSTALL COPPERHEAD® OR EQUAL 12-GAUGE EXTRA HIGH STRENGTH 1150LB BREAK STRENGTH 45 MIL HDPE JACKET, COPPER-CLAD, STEEL REINFORCED TRACER WIRE ON ALL WATER MAIN AND SERVICE LINES INSTALLED BY BORING METHODS.
- III. FASTEN WIRE TO PIPE IN TWO PLACES PER PIPE SECTION. EXTEND TRACER WIRE TO GROUND SURFACE AT ALL VALVES AND HYDRANTS AS SHOWN IN THE DEL-CO STANDARD DETAIL. SPLICE WIRES USING BURNDY COPPER SPLIT BOLT KS-15. THOROUGHLY WRAP THE CONNECTOR AND BARE WIRES WITH 3M TEMFLEX 2155 RUBBER SPLICING TAPE, COVER ENTIRE CONNECTION WITH SCOTCH SUPER 88 HEAVY DUTY GRADE ELECTRICAL TAPE.
- IV. CONNECT ALL SERVICE LINE WIRES TO MAIN LINE WIRES USING BURNDY COPPER SPLIT BOLT KS-15. THOROUGHLY WRAP THE CONNECTOR AND BARE WIRES WITH 3M TEMFLEX 2155 RUBBER SPLICING TAPE, COVER ENTIRE CONNECTION WITH SCOTCH SUPER 88 HEAVY DUTY GRADE ELECTRICAL TAPE.

CONTRACTOR SHALL TEST THE CONTINUITY OF ALL WIRE USING A THIRD-PARTY TESTER.

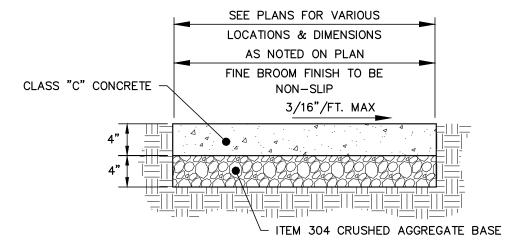
22 **a b** 27 **a b** 0 őŌ S U 2 MEDIXE **Date:** 04/22/2019 Scale: AS NOTED Drawn By: | Checked By: DDD **Project Number:** 19-0009-579 Drawing Number:



ACCESSIBLE PARKING SIGN DETAIL NO SCALE

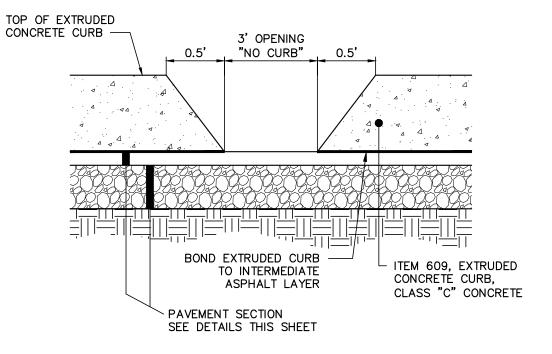


EXTRUDED CONCRETE CURB NO SCALE

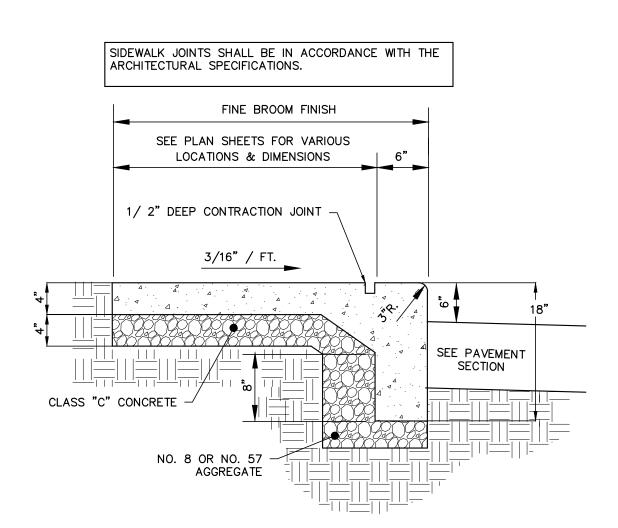


CONCRETE SIDEWALK SECTION

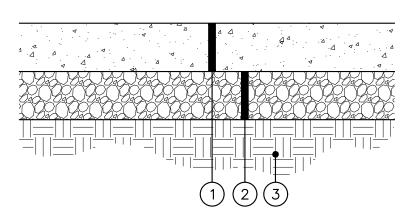
NO SCALE



CURB OPENING DETAIL NO SCALE



COMBINED CURB AND WALK NO SCALE



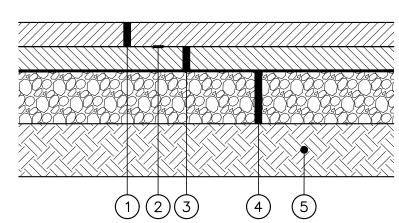
- 1 ITEM 451, 8" REINFORCED P.C. CONCRETE PAVEMENT (CLASS C)
- 2) ITEM 304, 6" CRUSHED AGGREGATE BASE
- 3 ITEM 204, SUBGRADE COMPACTION

NOTES:

1. ALL PAVEMENT MATERIAL SHALL CONFORM TO THE STATE OF OHIO DEPARTMENT OF TRANSPORTATION CONSTRUCTION AND MATERIAL SPECIFICATIONS.

THE PAVEMENT SECTIONS ARE A RECOMMENDATION ASSUMING SUITABLE SOIL CONDITIONS. ACD RECOMMENDS THAT A GEOTECHINCAL ENGINEER REVIEWS THE SOIL CONDITIONS AND MAKES A FINAL RECOMMENDATION FOR THE SUBGRADE AND PAVEMENT SECTION.

CONCRETE DUMPSTER PAD SECTION NO SCALE



ITEM 441, 1-1/2" ASPHALT CONCRETE SURFACE

COURSE, MEDIÚM TRAFFIC, PG64-22

- 1 ITEM 441, 1-1/2" ASPHALT CONCRETE SURFACE COURSE, MEDIUM TRAFFIC, PG64-22
- (2) ITEM 407, TACK COAT (0.15-0.20 GAL/SQ. YD.)
- 2) ITEM 407, TACK COAT (0.15-0.20 GAL/SQ. YD.)
- O ITEM 441 1 1/2" ASPILALT CONODETE

CONSTRUCTION AND MATERIAL SPECIFICATIONS.

- 3 ITEM 441, 1-1/2" ASPHALT CONCRETE INTERMEDIATE COURSE, MEDIUM TRAFFIC, PG64-22 (3) ITEM 441, 2-1/2" ASPHALT CONCRETE INTERMEDIATE COURSE, MEDIUM TRAFFIC, PG64-22
- 4 ITEM 304, 8" CRUSHED AGGREGATE BASE
- (4) ITEM 304, 10" CRUSHED AGGREGATE BASE
- 5 ITEM 204, SUBGRADE COMPACTION

 LIGHT DUTY (LD)
- (5) ITEM 204, SUBGRADE COMPACTION

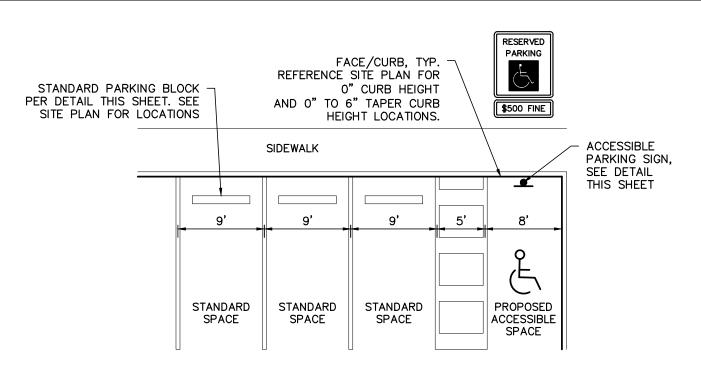
 HEAVY DUTY (HD)

S: THE TACK COAT MAY BE ELIMINATED IF THE INTERMEDIATE COURSE IS CLEAN AND THE SURFACE IS PLACED IMMEDIATELY FOLLOWING THE PLACEMENT OF THE INTERMEDIATE COURSE.

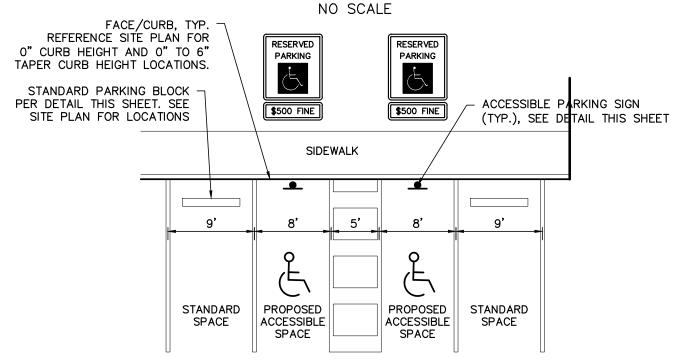
- 2. ALL PAVEMENT MATERIAL SHALL CONFORM TO THE STATE OF OHIO DEPARTMENT OF TRANSPORTATION
- 3. THE PAVEMENT SECTIONS ARE A RECOMMENDATION ASSUMING SUITABLE SOIL CONDITIONS. ACD RECOMMENDS THAT A GEOTECHINCAL ENGINEER REVIEWS THE SOIL CONDITIONS AND MAKES A FINAL RECOMMENDATION FOR THE SUBGRADE AND PAVEMENT SECTION.

PAVEMENT SECTION

NO SCALE

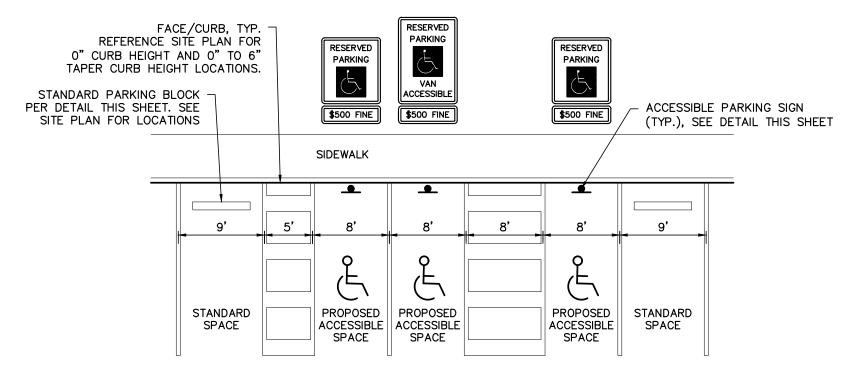


ACCESSIBLE PARKING DETAIL, TYPICAL OF 1



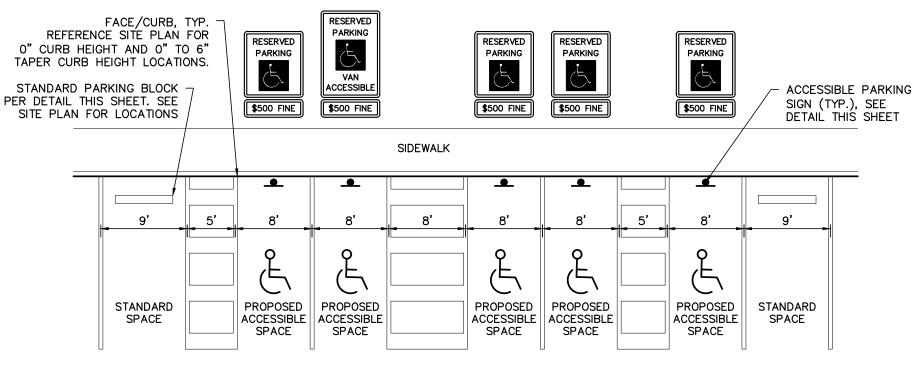
ACCESSIBLE PARKING DETAIL, TYPICAL OF 2

NO SCALE

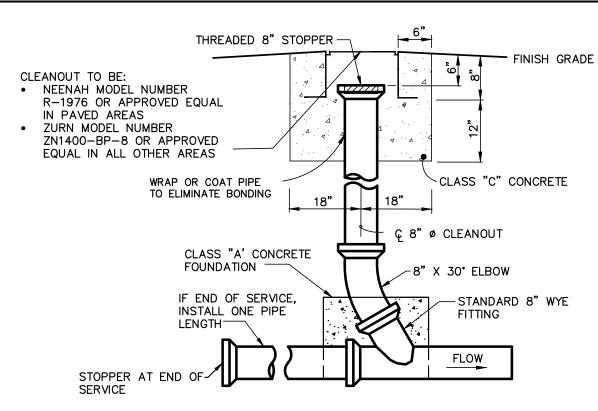


ACCESSIBLE PARKING DETAIL, TYPICAL OF 3

NO SCALE

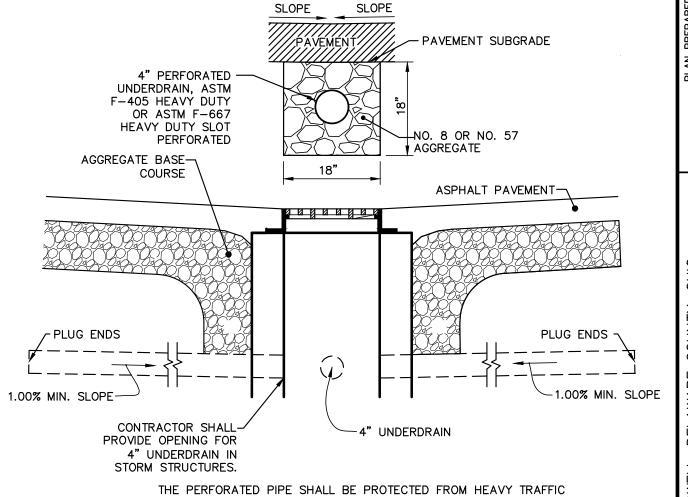


ACCESSIBLE PARKING DETAIL, TYPICAL OF 5 NO SCALE



SEWER CLEANOUT

NO SCALE



AFTER INSTALLATION PRIOR TO PLACEMENT OF PROPOSED PAVING.

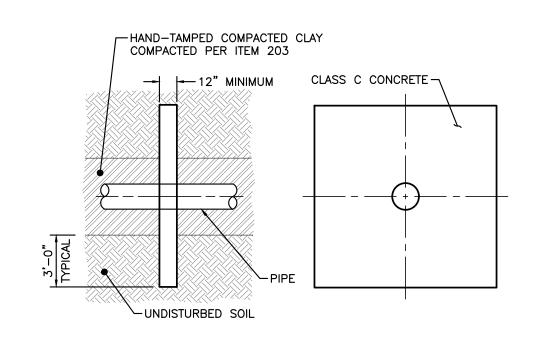
UNDERDRAIN SHALL BE INSTALLED IN ALL DIRECTIONS FOR THE

UNDERDRAIN SHALL BE INSTALLED IN ALL DIRECTIONS FOR TH SHORTEST OF THE THREE CONDITIONS BELOW: 1. FOR A LENGTH OF 20' 2. TO THE END OF PAVEMENT

4" UNDERDRAIN AT CATCH BASIN DETAIL

3. TO THE PAVEMENT HIGH POINT

NO SCALE

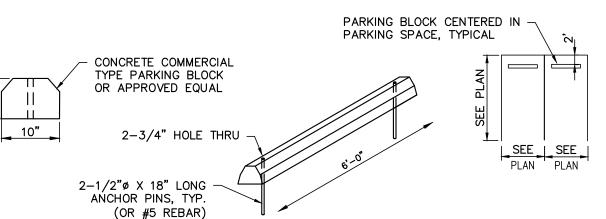


NOTES: SPACING BETWEEN ADJACENT COLLARS SHALL BE A MINIMUM OF 5 FEET WITH THE FIRST COLLAR BEING A MINIMUM OF 5 FEET FROM THE INLET.

FURNISH A MINIMUM OF 2 COLLARS PER OUTLET CONDUIT.

ANTI-SEEP CONCRETE COLLAR DETAIL

NO SCALE

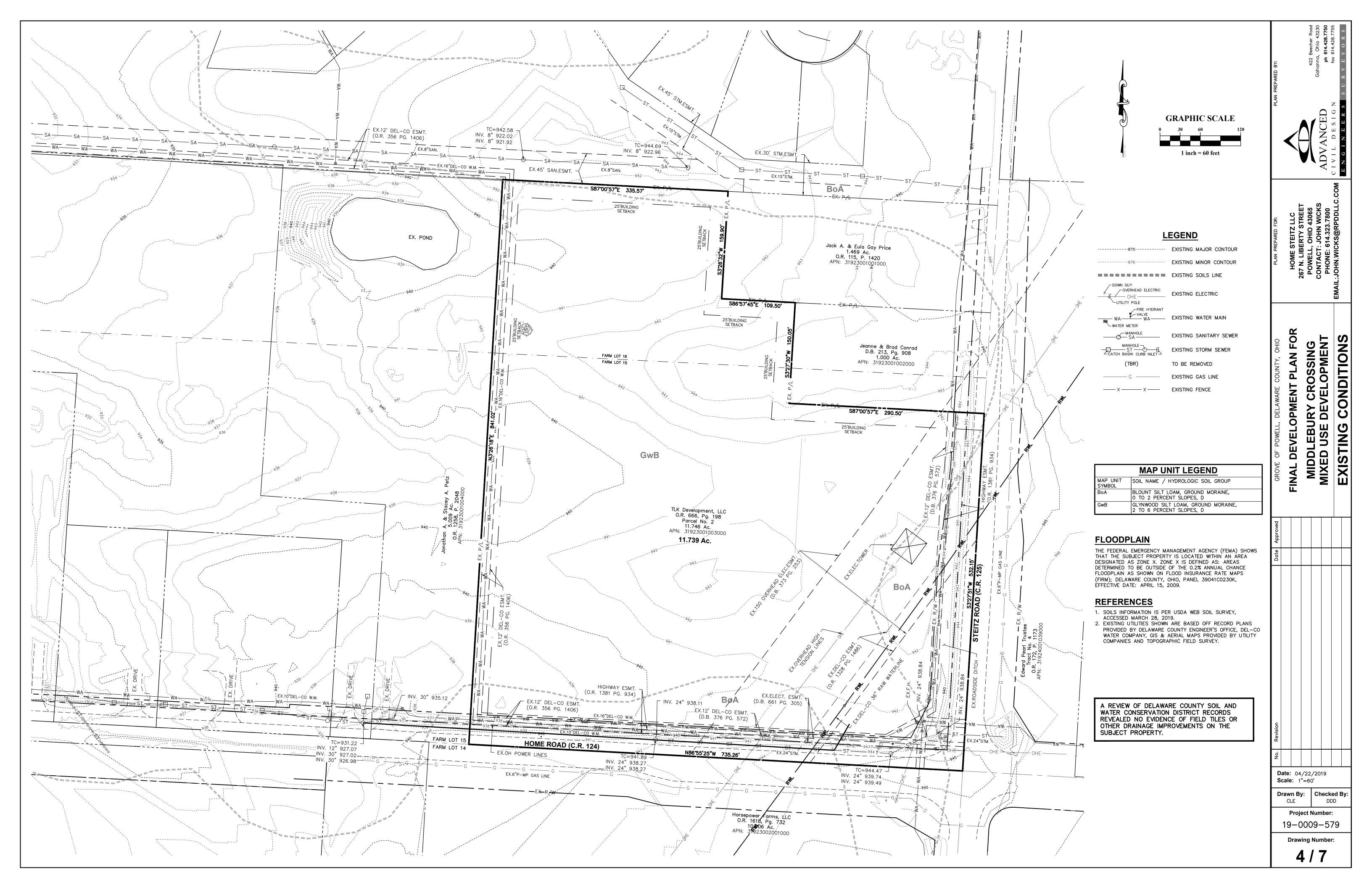


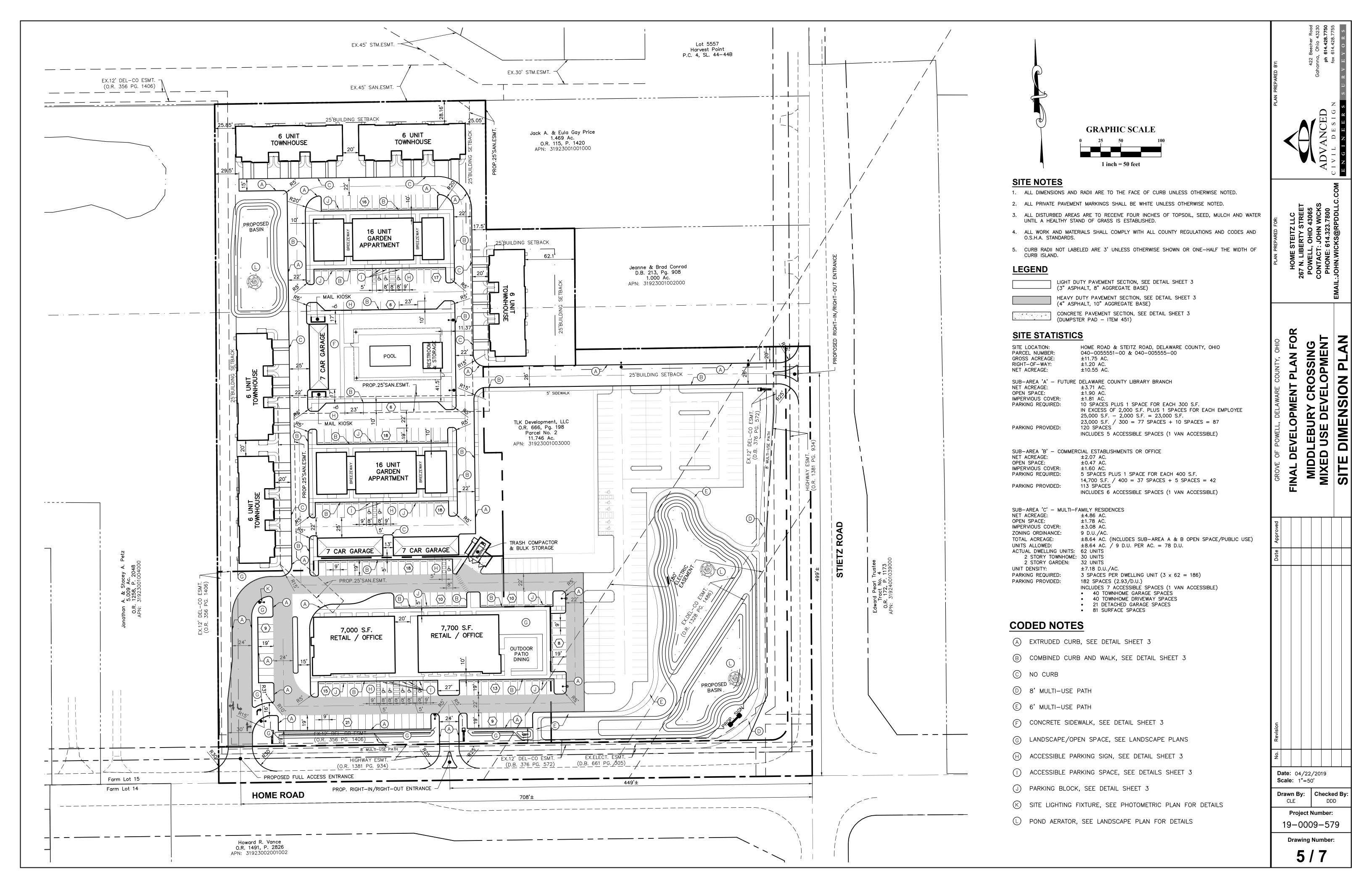
STANDARD PARKING BLOCK DETAIL

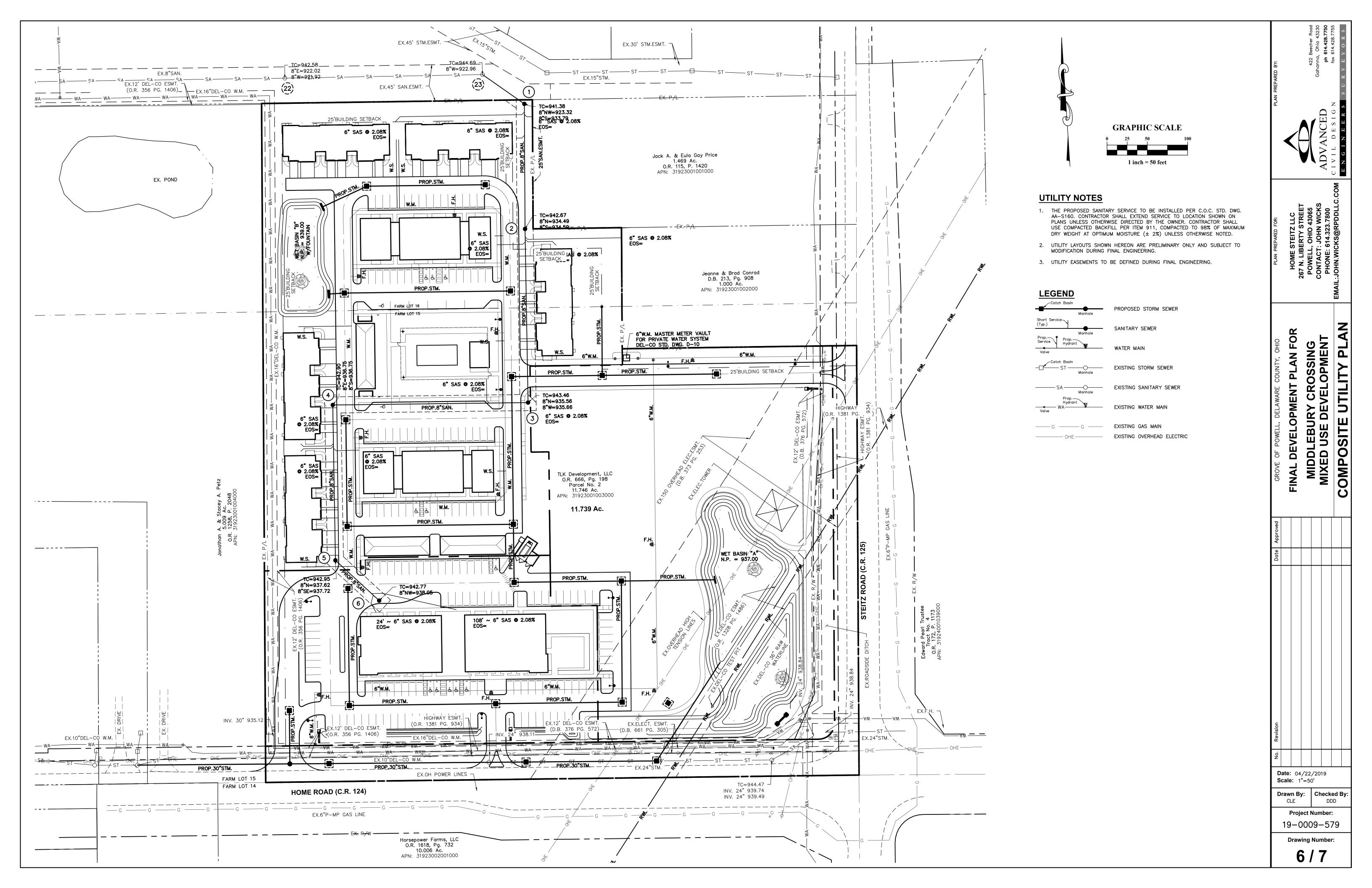
NO SCALE

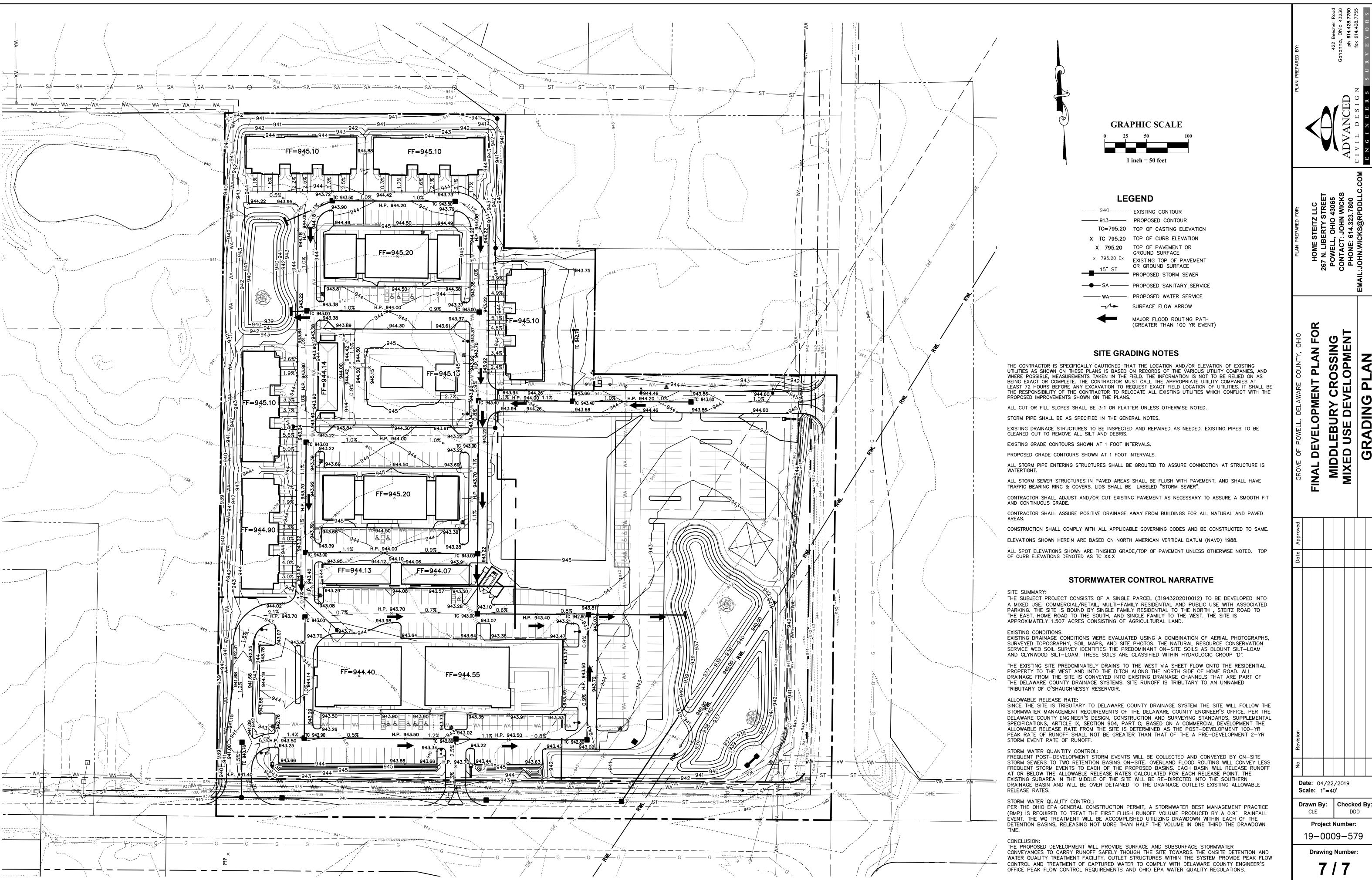
FOR SSING **25** 単 MIXE Date: 04/22/2019 Scale: AS NOTED Checked By Drawn By: DDD **Project Number:** 19-0009-579 **Drawing Number:** 3 / 7

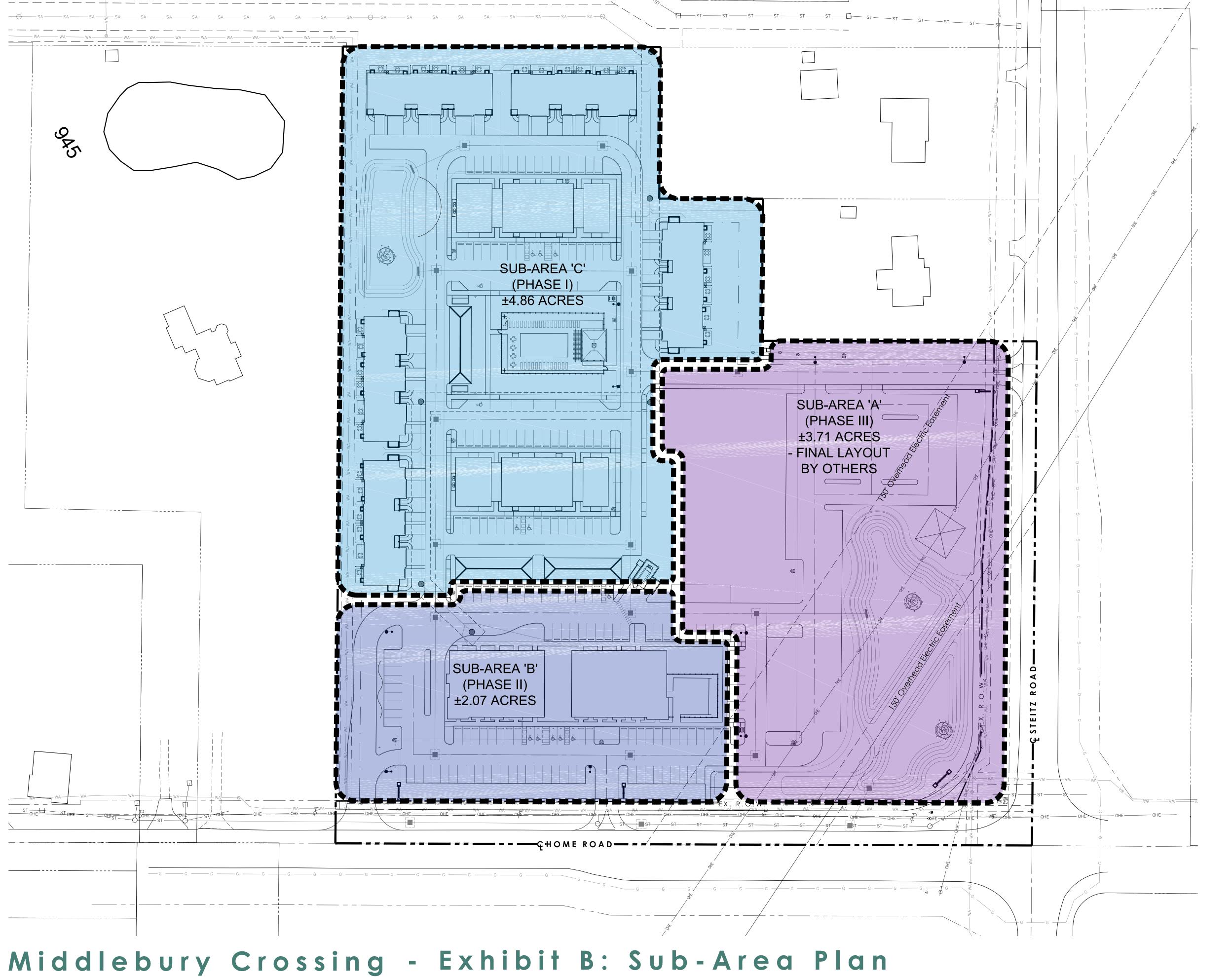
22 Ina **ph** fax











SITE DATA

Planned Commercial District Sub-Areas

Sub-Area 'A' - Future Delaware County Library Branch

± 3.71Acres

Sub-Area 'B' - Commercial Establishments or Office

± 2.07 Acres

Sub-Area 'C' - Multi-Family Residences

± 4.86 Acres

62 Dwelling Units

Residential Density per 1143.09(c)(10)(C)

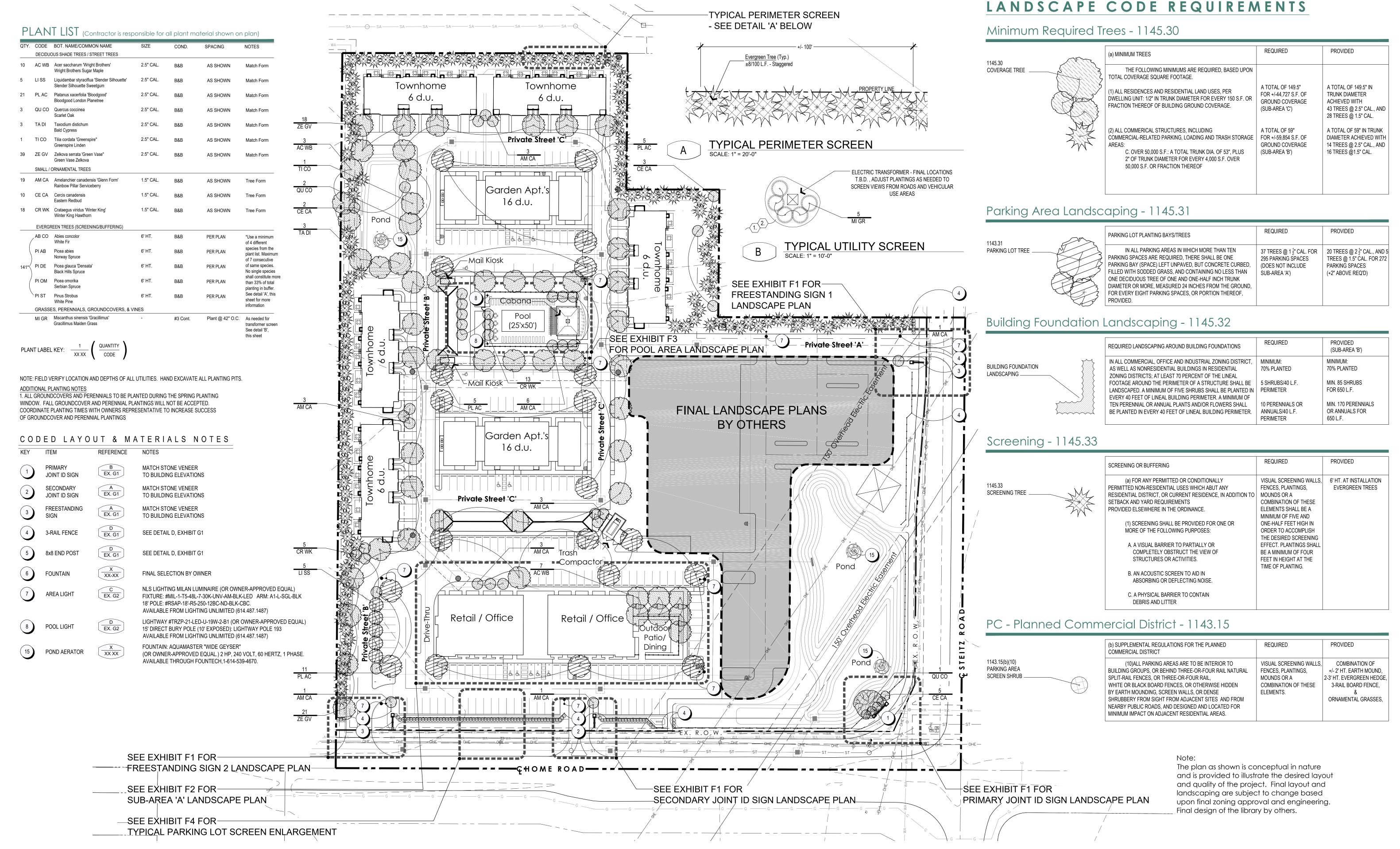
Dwelling Units	62 D.U.
Total Acreage Sub-Area C acreage (4.86 Ac.) + Sub-Area A&B Open Space/Public Use (3.78 Ac.)	± 8.64 Acres
	± 7.18 D.U./Acre

Residential Units Permitted by Zoning Ordinance

Total Acreage	± 8.64 Acres
Sub-Area C acreage (4.86 Ac.) + Sub-Area A&B Open Space/Public Use (3.78 Ac.)	
	78 D.U.

The plan as shown is conceptual in nature and is provided to illustrate the desired layout and quality of the project. Final layout and landscaping are subject to change based upon final zoning approval and engineering. Final design of the library by others.

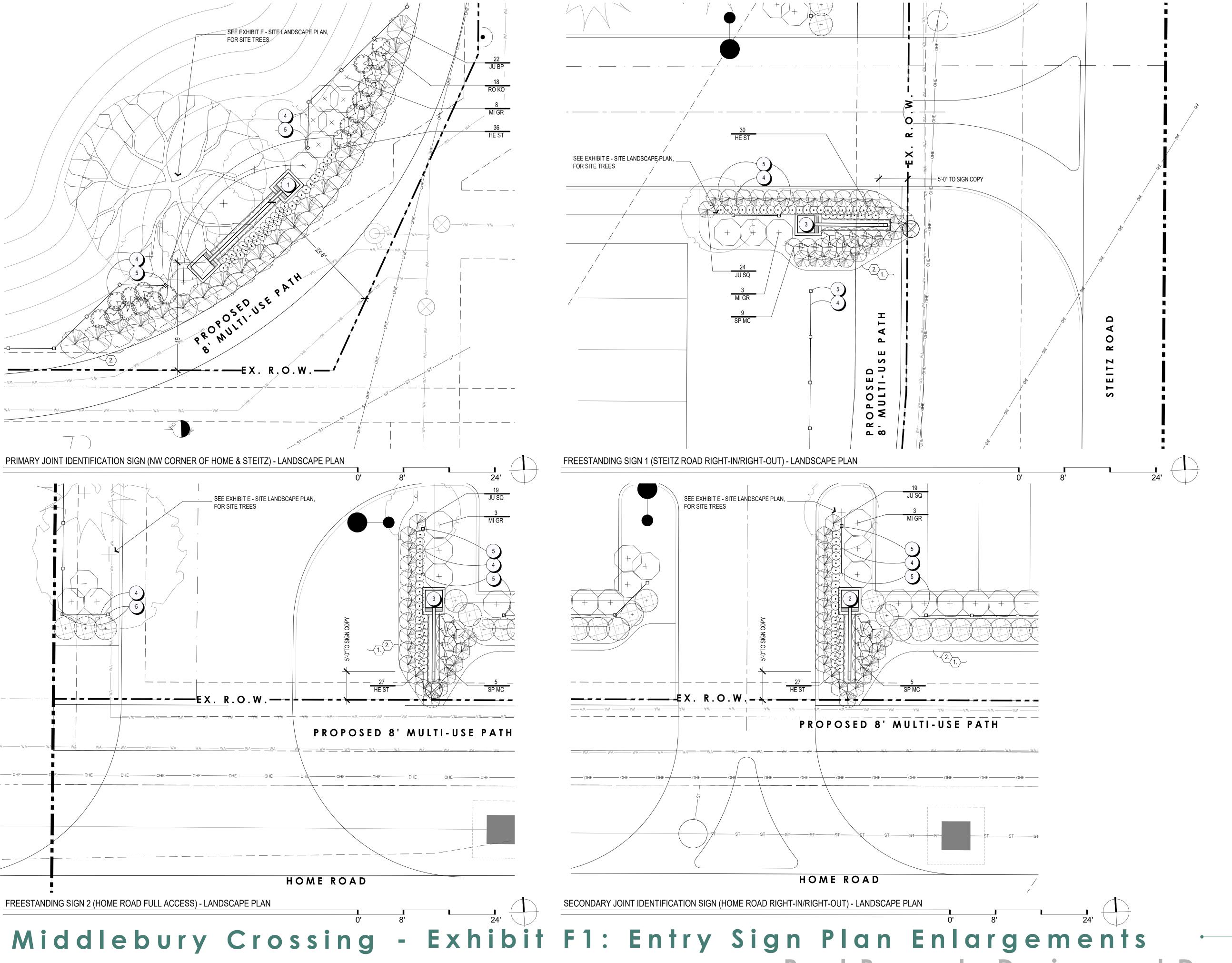




Middlebury Crossing - Exhibit E: Site Landscape Plan

0 50 150 N

Planning
Urban Design
Landscape Architecture
Planning + Design
Ph: 614.583.9230
Web: www.g2planning.com



PLANT LIST (Contractor is responsible for all plant material shown on plan)

	/ (1	Confidence is les	porisible for	ali piarii m	arenai sriowi i	on plan
QTY.	CODE	BOT. NAME/COMMON NAME	SIZE	COND.	SPACING	NOTES
FLOWE	ERING / DEC	CIDUOUS SHRUBS				
18	RO KO	Rosa x 'Radtko' Double Knock Out Rose	24" HT.	#3 Cont.	PER PLAN	
19	SP MC	Spirea japonica 'Walbuma' Magic Carpet Spirea	24" SPRD.	#3 Cont.	PER PLAN	
EVERG	REEN SHR	UBS				
22	JU BP	Juniperus conferta 'Blue Pacific' Blue Pacific Juniper	24" SPRD.	B&B	PER PLAN	
62	JU SQ	Juniperus Squamata 'Blue Star' Blue Star Juniper	24" SPRD.	B&B	PER PLAN	
GRASS	ES, PEREN	NIALS, GROUNDCOVERS, & VINES				
84	HE ST	Hemerocallis 'Stella d'Oro' Stella d'Oro Daylily	=	#1 Cont.	PER PLAN	
		Missonthus sinansis (Crasillimus)				

PLANT LABEL KEY: $\frac{1}{XXXX}$ QUANTITY CODE

NOTE: FIELD VERIFY LOCATION AND DEPTHS OF ALL UTILITIES. HAND EXCAVATE ALL PLANTING PITS

ADDITIONAL PLANTING NOTES

THE PLAN AS SHOWN IS CONCEPTUAL IN NATURE AND IS PROVIDED TO ILLUSTRATE THE DESIRED LAYOUT AND QUALITY OF THE PROJECT. FINAL LAYOUT AND LANDSCAPING ARE SUBJECT TO CHANGE

CODED LANDSCAPE NOTES

2. LANDSCAPE BED - PROVIDE 3" DEPTH HARDWOOD MULCH, POSITIVE DRAINAGE IN ALL DIRECTIONS. HAND SPADE EDGE.

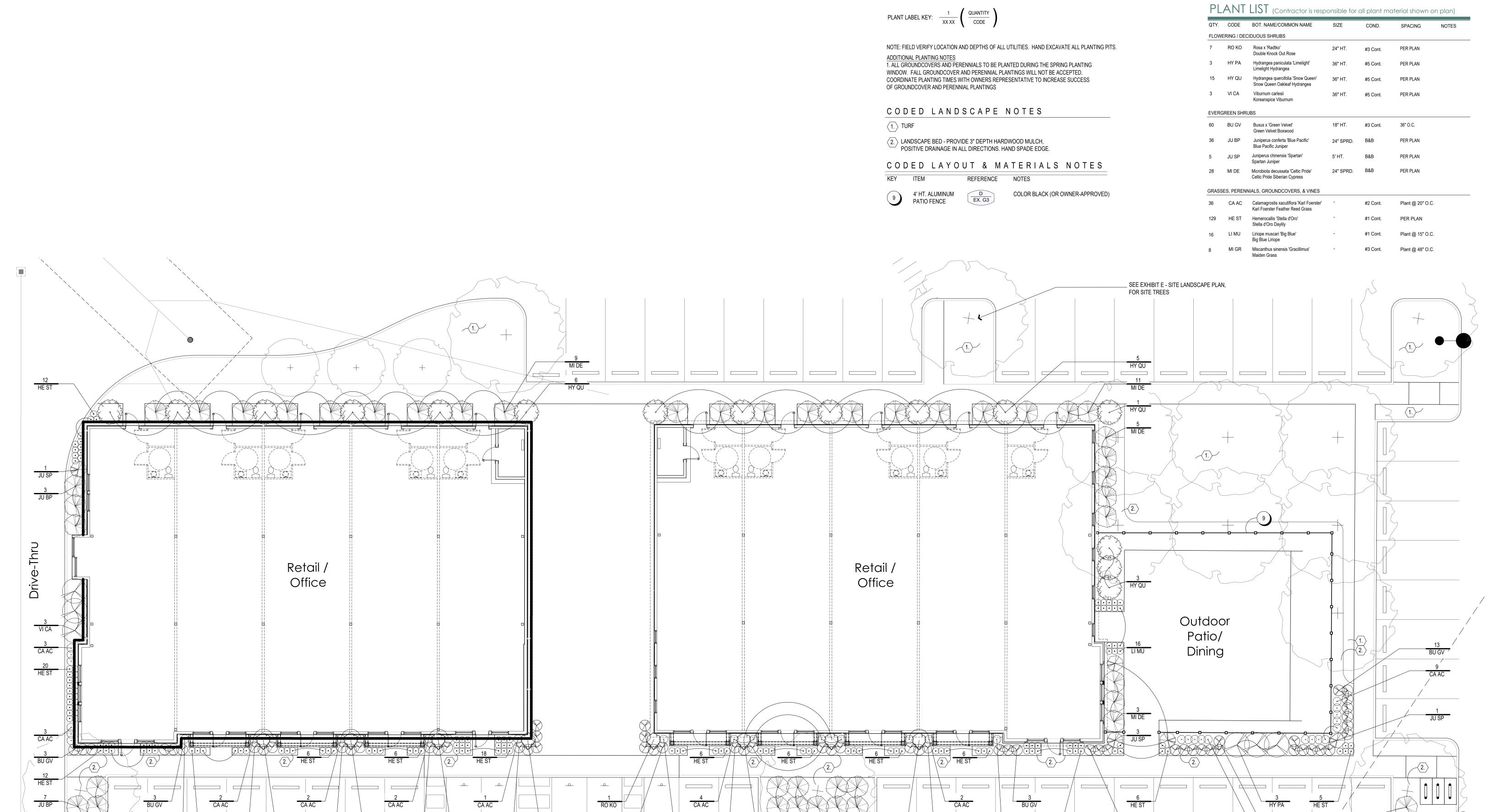
CODED LAYOUT & MATERIALS NOTES

KEY	IIEM	REFERENCE	NOTES
1	PRIMARY JOINT ID SIGN	B EX. G1	MATCH STONE VENEER TO BUILDING ELEVATIONS
2	SECONDARY JOINT ID SIGN	A EX. G1	MATCH STONE VENEER TO BUILDING ELEVATIONS
3	FREESTANDING SIGN	A EX. G1	MATCH STONE VENEER TO BUILDING ELEVATIONS
4	3-RAIL FENCE	D EX. G1	SEE DETAIL D, EXHIBIT G1
5	8x8 END POST	D FY C1	SEE DETAIL D. EXHIBIT G1

City of Powell, Ohio 04.23.2019

Real Property Design and Development



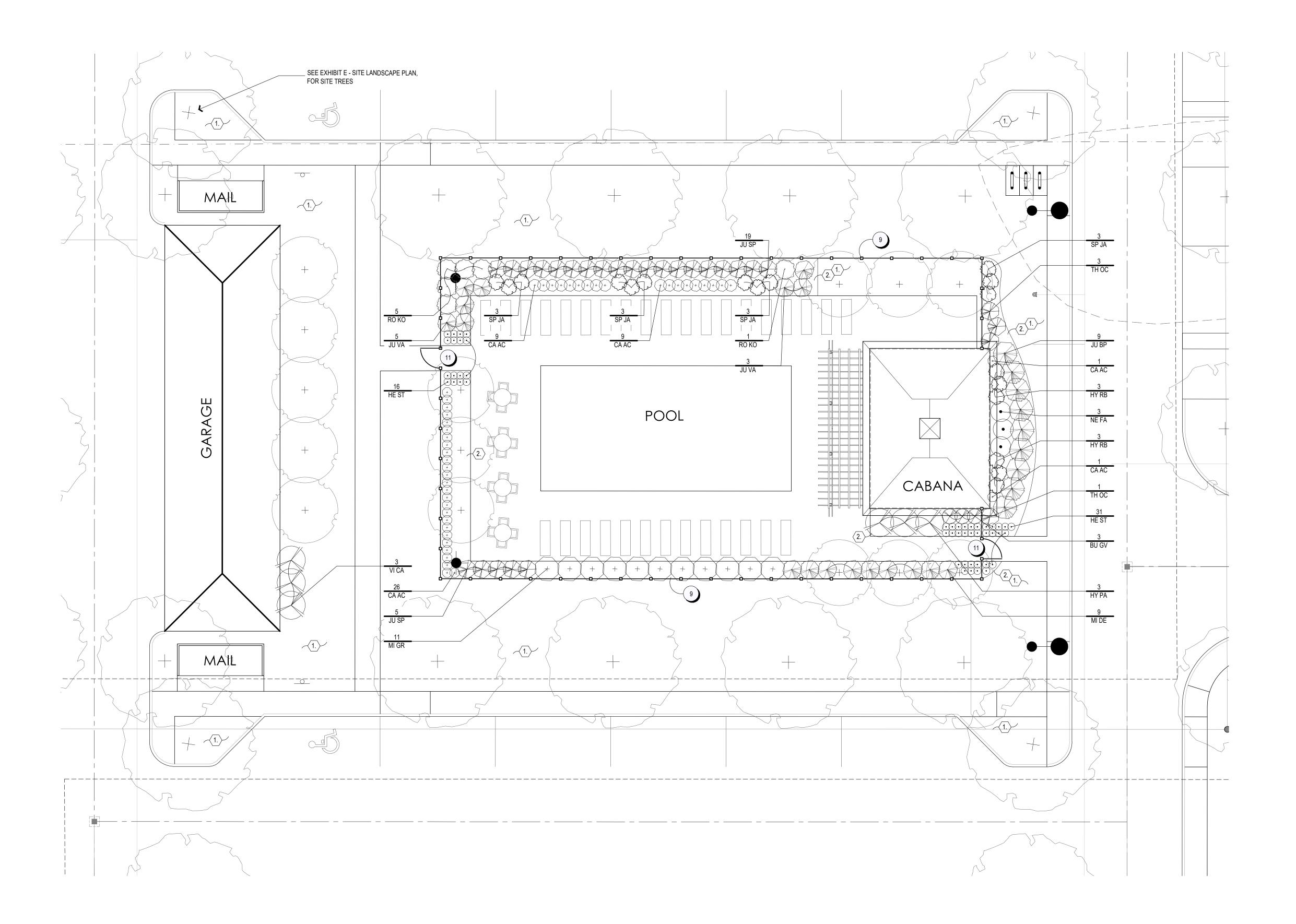


Middlebury Crossing - Exhibit F2: Sub-Area 'B' Landscape Plan Real Property Design and Development City of Powell, Ohio 04.23.2019

5 BU GV

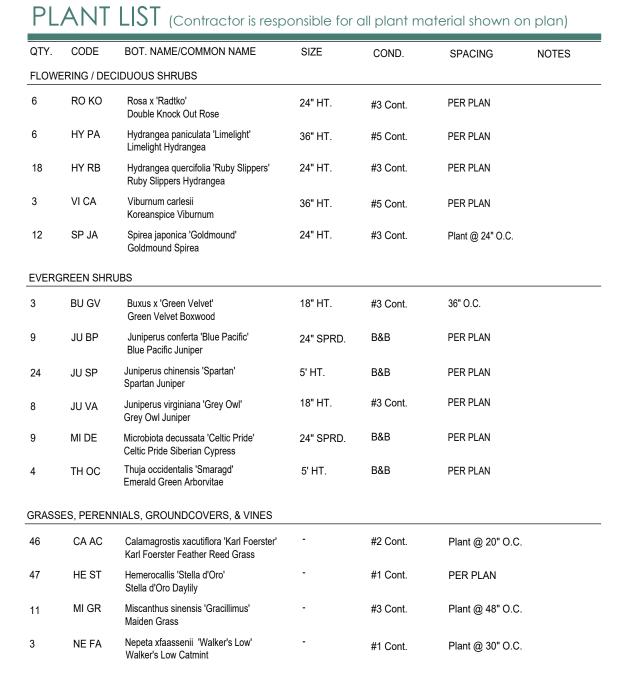


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Web: www.g2planning.com



Middlebury Crossing - Exhibit F3: Pool Area Landscape Plan 5 15 30 18 Real Property Design and Development City of Powell, Ohio 04.23.2019





PLANT LABEL KEY: $\frac{1}{XXXX} \left(\frac{QUANTITY}{CODE} \right)$

NOTE: FIELD VERIFY LOCATION AND DEPTHS OF ALL UTILITIES. HAND EXCAVATE ALL PLANTING PITS.

ADDITIONAL PLANTING NOTES

1. ALL GROUNDCOVERS AND PERENNIALS TO BE PLANTED DURING THE SPRING PLANTING WINDOW. FALL GROUNDCOVER AND PERENNIAL PLANTINGS WILL NOT BE ACCEPTED. COORDINATE PLANTING TIMES WITH OWNERS REPRESENTATIVE TO INCREASE SUCCESS

THE PLAN AS SHOWN IS CONCEPTUAL IN NATURE AND IS PROVIDED TO ILLUSTRATE THE DESIRED LAYOUT AND QUALITY OF THE PROJECT. FINAL LAYOUT AND LANDSCAPING ARE SUBJECT TO CHANGE BASED UPON FINAL ENGINEERING AND HOMEOWNER SELECTIONS.

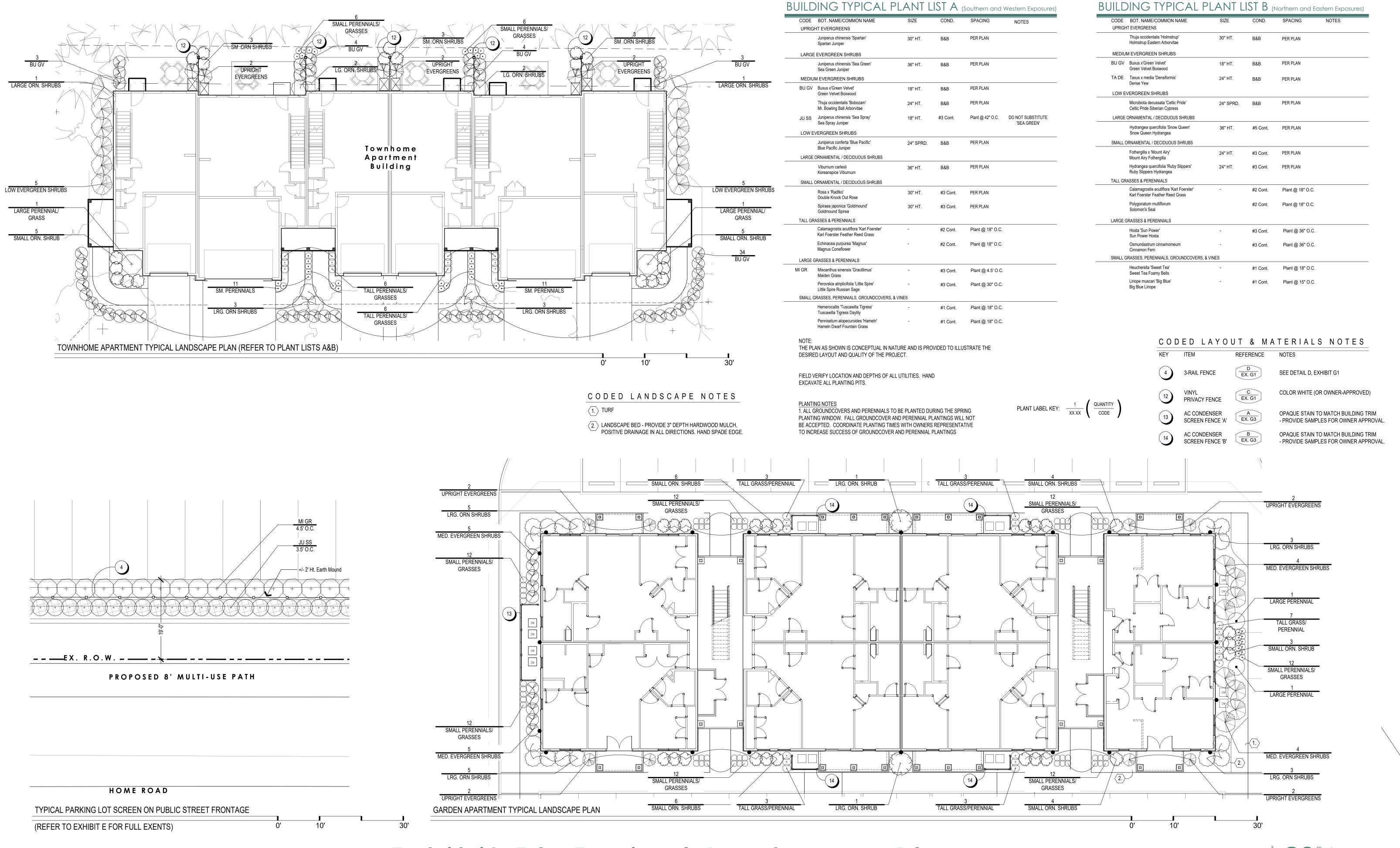
CODED LANDSCAPE NOTES

2. LANDSCAPE BED - PROVIDE 3" DEPTH HARDWOOD MULCH, POSITIVE DRAINAGE IN ALL DIRECTIONS. HAND SPADE EDGE.

NE I	I I □IVI	REFERENCE	NOTES
4	3-RAIL FENCE	D EX. G1	SEE DETAIL D, EXHIBIT G1
9	4' HT. ALUMINUM PATIO FENCE	D EX. G3	COLOR BLACK (OR OWNER-APPROVED)
10	5' HT. ALUMINUM POOL FENCE	C EX. G3	COLOR BLACK (OR OWNER-APPROVED)

COLOR BLACK (OR OWNER-APPROVED)

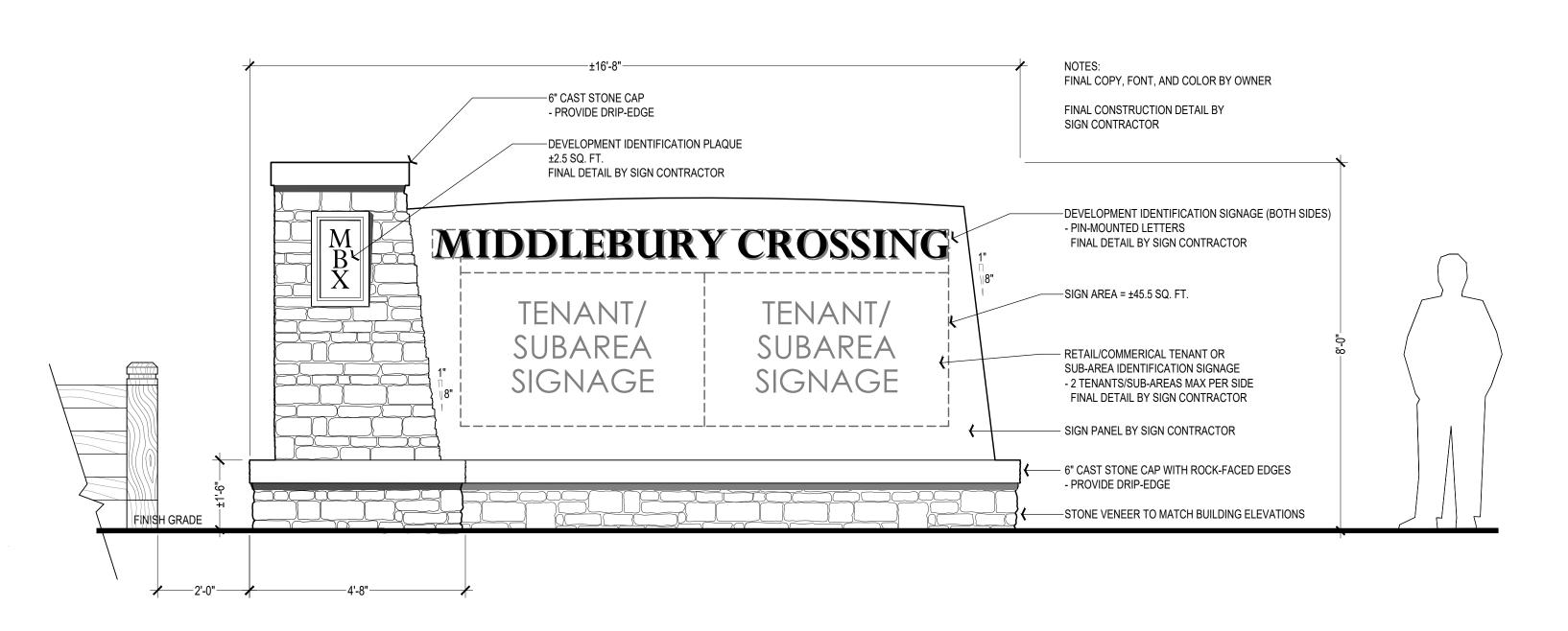
PROVIDE SELF-CLOSING HINGES AND MAGNETIC LATCH COLOR WHITE (OR OWNER-APPROVED)

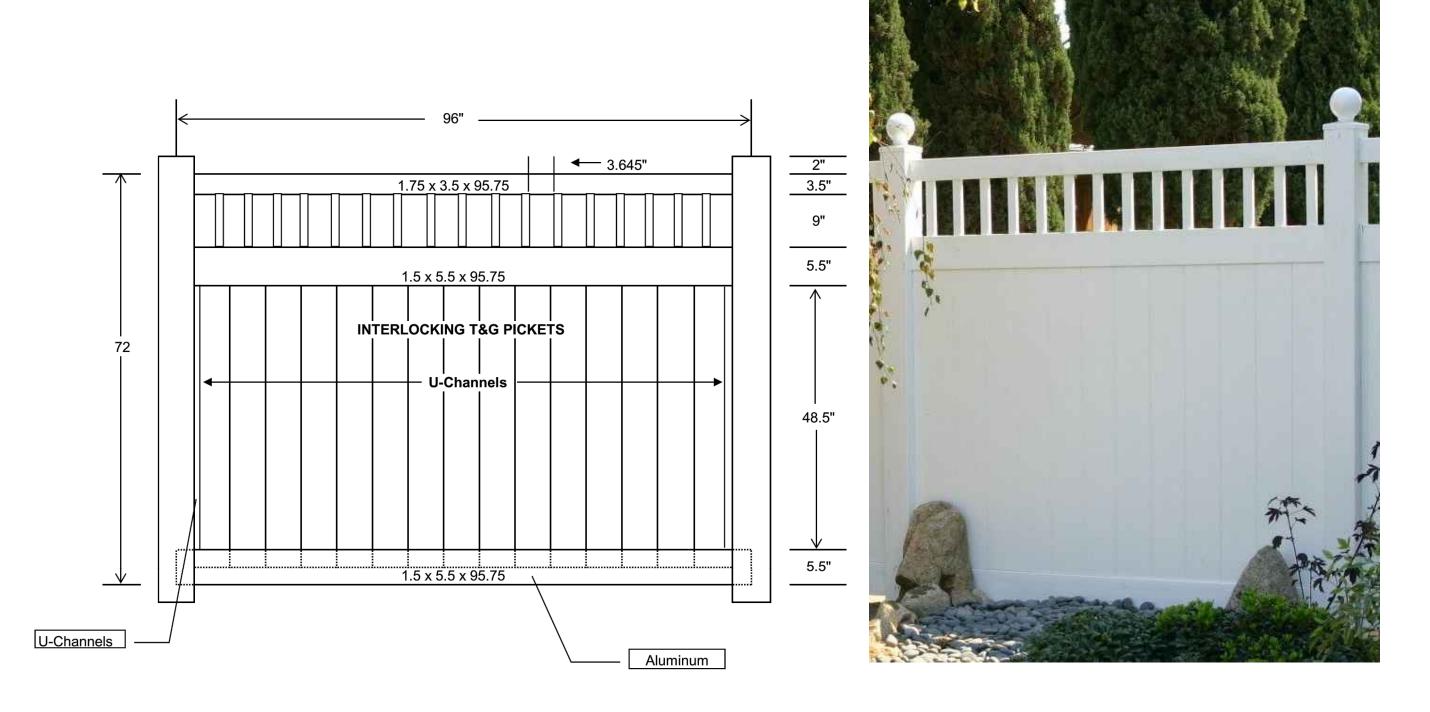


Middlebury Crossing - Exhibit F4: Typical Landscape Plans



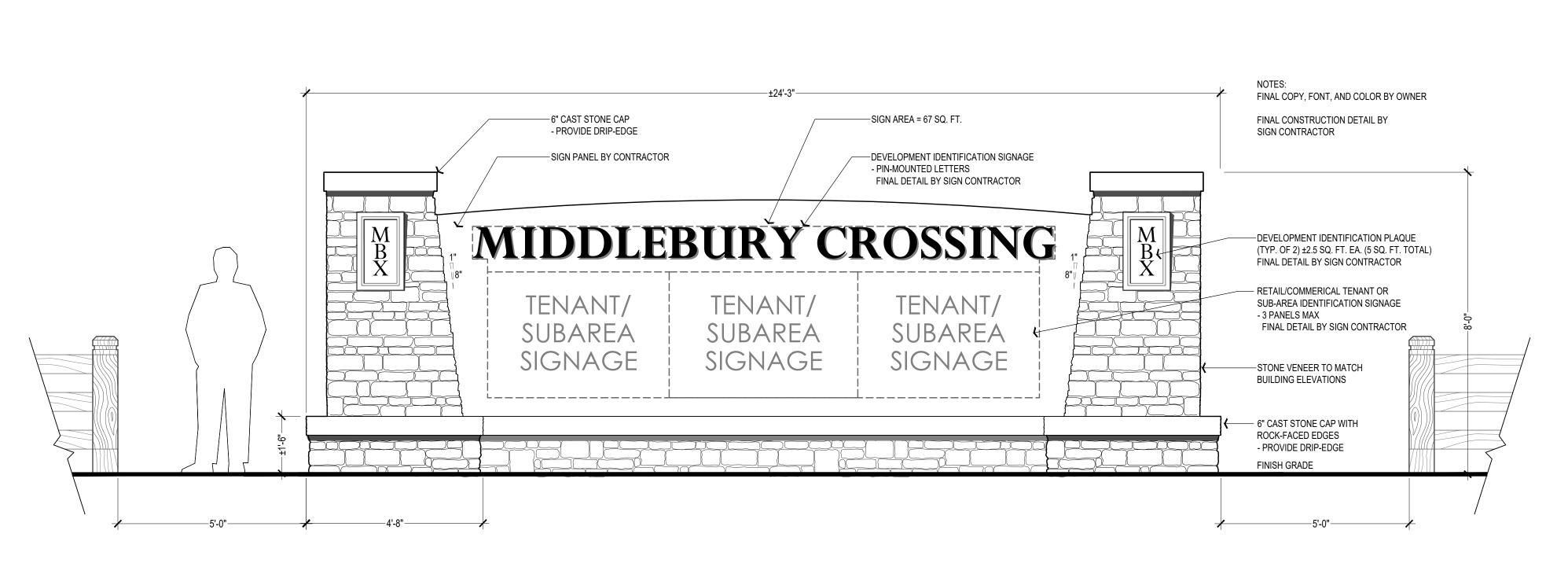


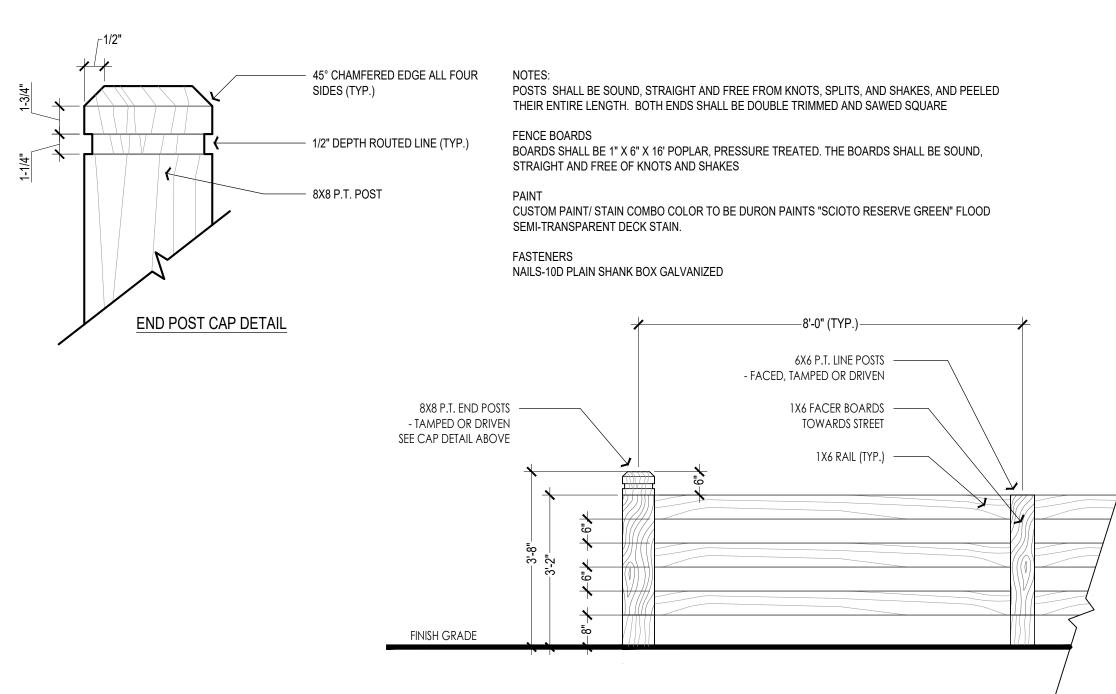




A DOUBLE-SIDED SECONDARY JOINT IDENTIFICATION SIGN/FREESTANDING SIGN

SCALE: 1/2" = 1'-0"





D

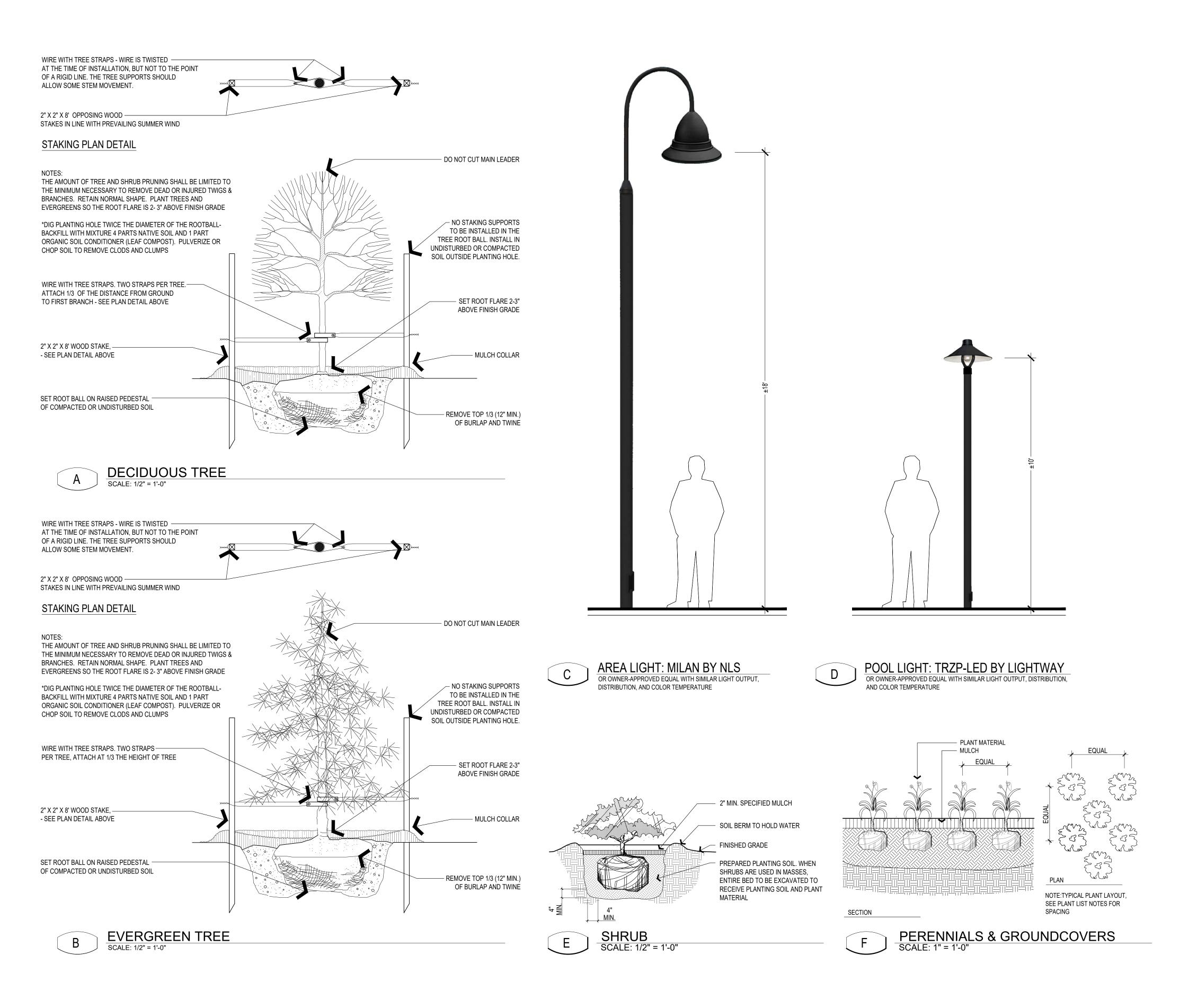
VINYL PRIVACY FENCE

3-RAIL BOARD FENCE
SCALE: 1/2" = 1'-0"





PRIMARY JOINT IDENTIFICATION SIGN

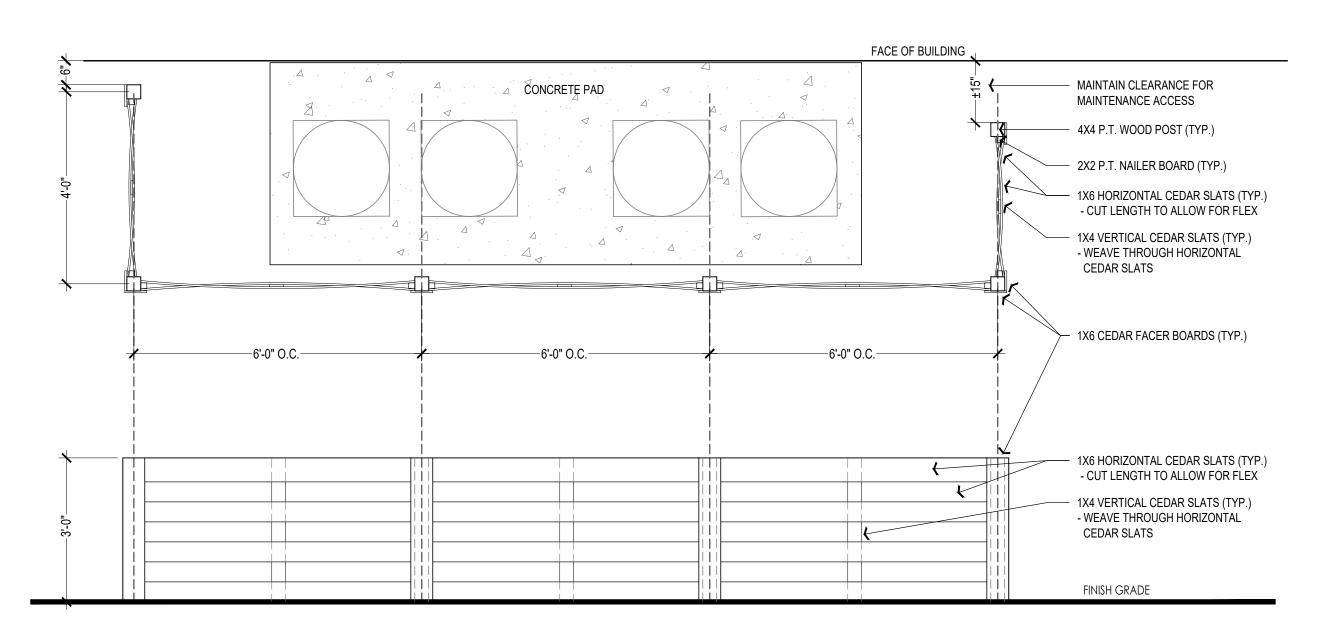


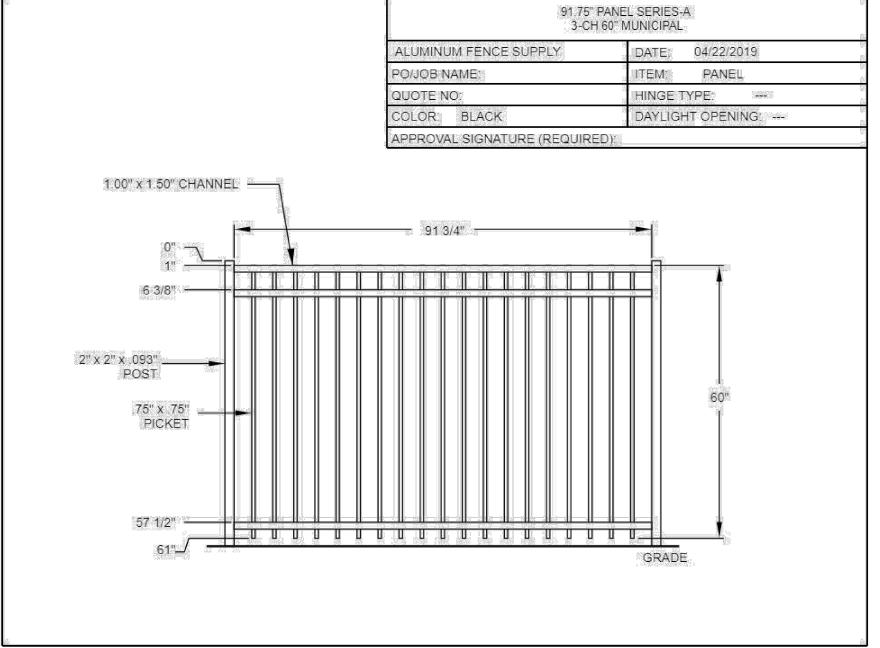
POSTS SHALL BE SOUND, STRAIGHT AND FREE FROM KNOTS, SPLITS, AND SHAKES, AND PEELED THEIR ENTIRE LENGTH. BOTH ENDS SHALL BE DOUBLE TRIMMED AND SAWED SQUARE

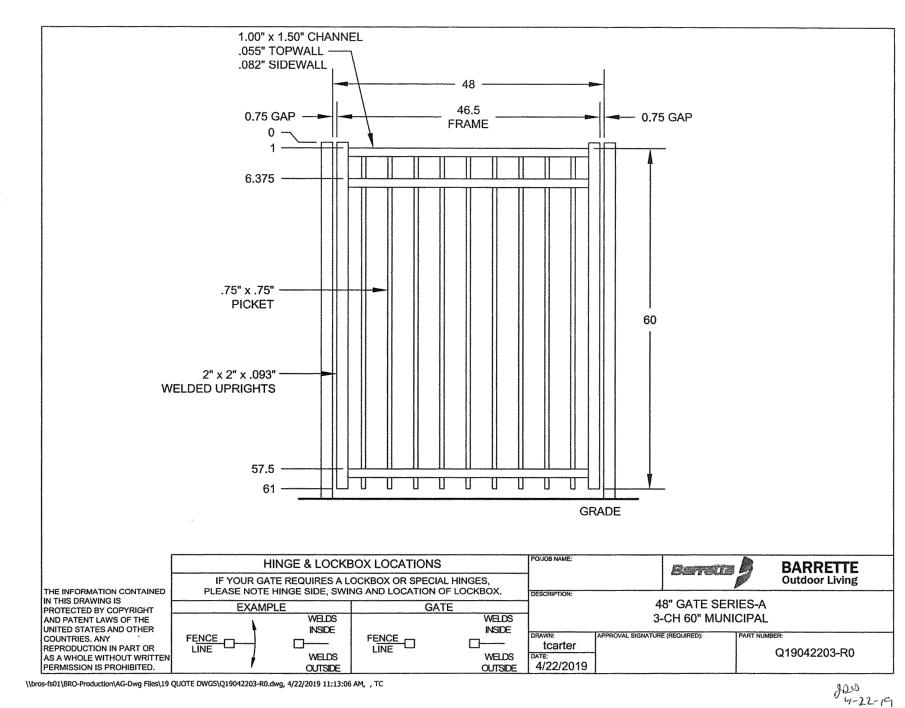
BOARDS SHALL BE 1" X 6" X 16' POPLAR, PRESSURE TREATED. THE BOARDS SHALL BE SOUND, STRAIGHT AND FREE OF KNOTS AND SHAKES

OPAQUE STAIN TO MATCH BUILDING TRIM - PROVIDE SAMPLES FOR OWNER APPROVAL.

FASTENERS NAILS-10D PLAIN SHANK BOX GALVANIZED



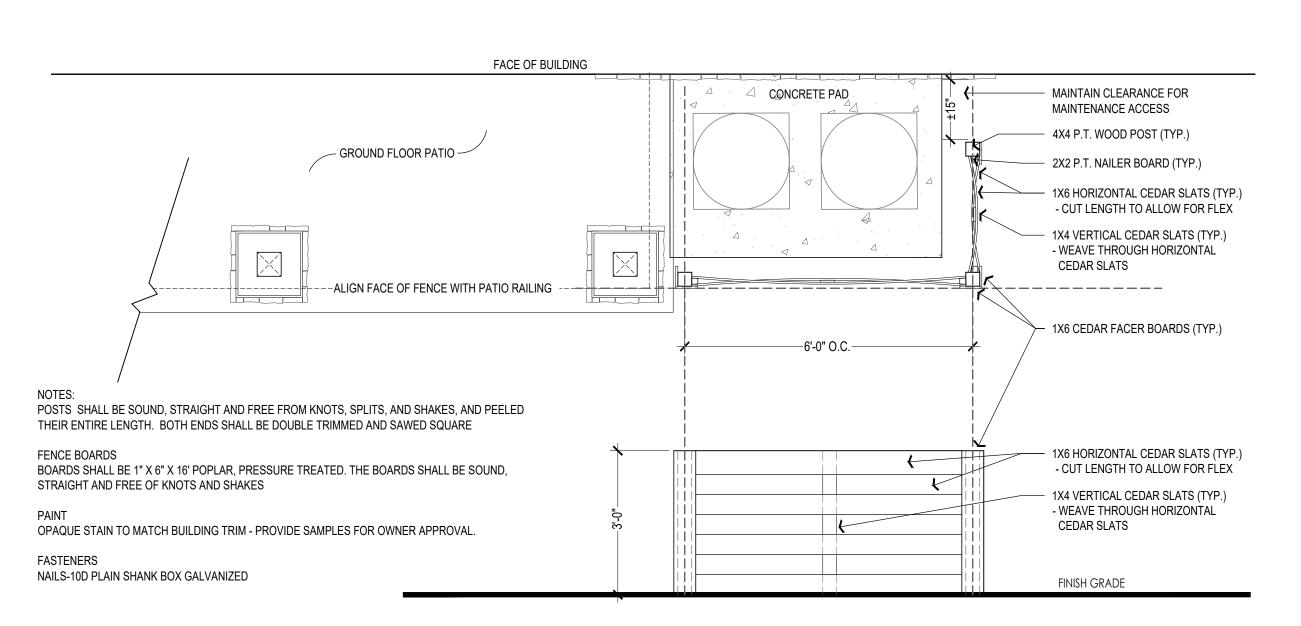


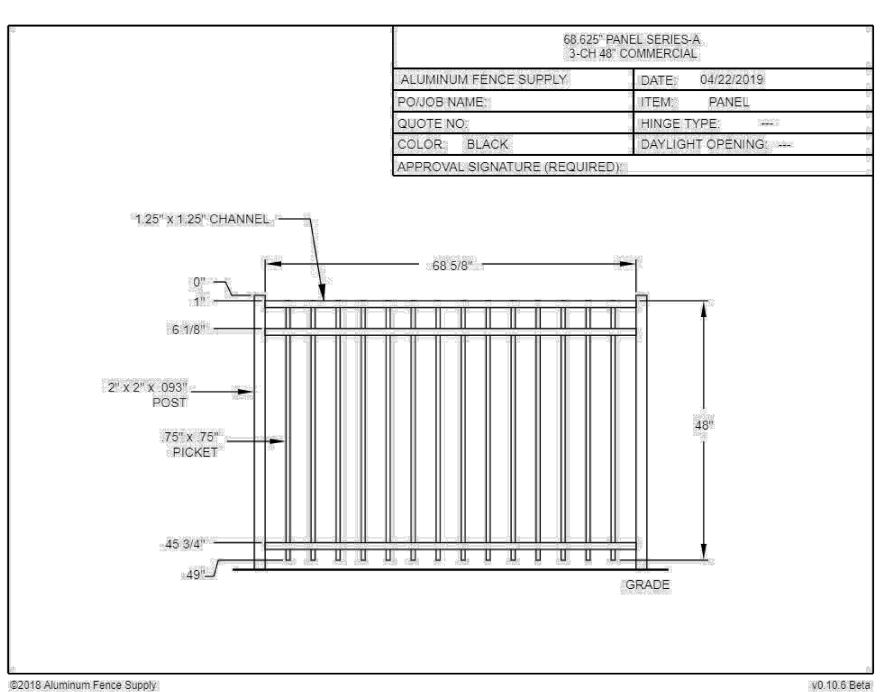


AC CONDENSER SCREEN FENCE 'A'

ALUMINUM POOL FENCE (OR OWNER- APPROVED EQUAL)

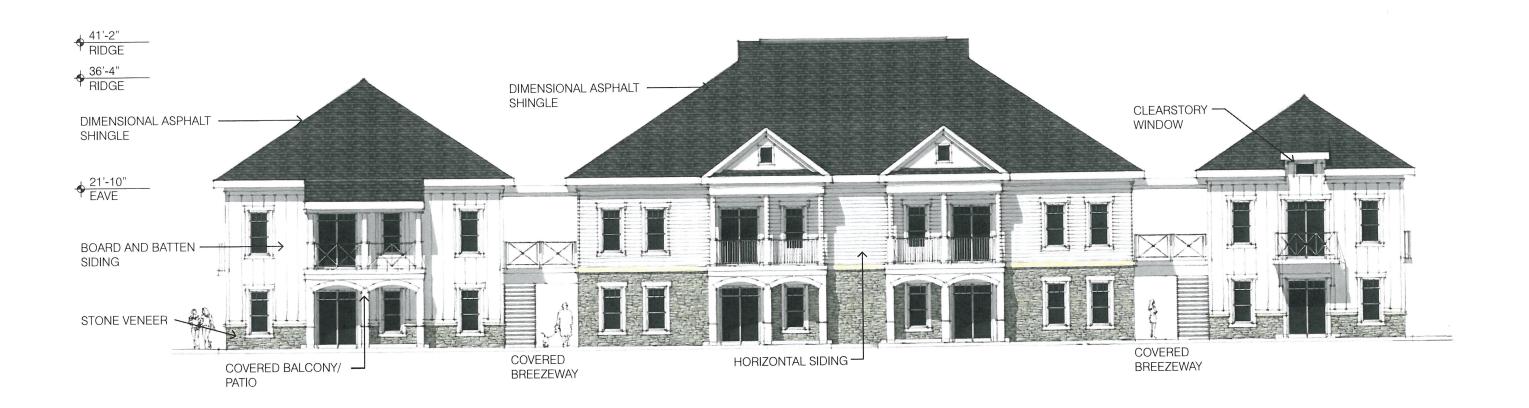
©2018 Aluminum Fence Supply



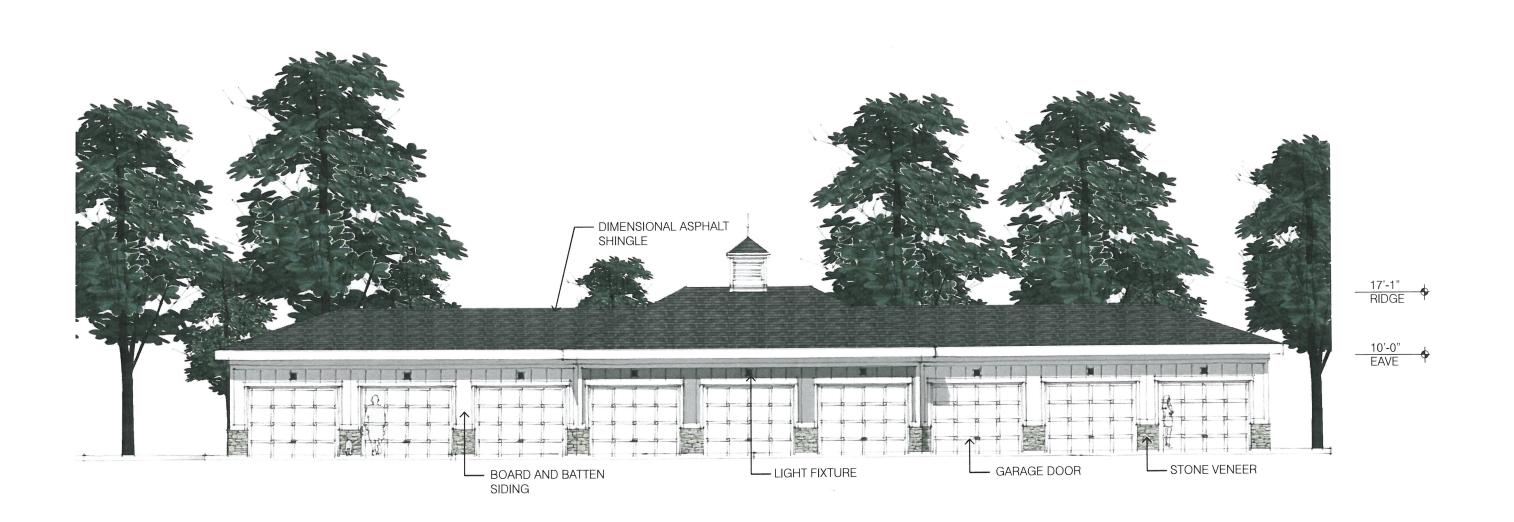


AC CONDENSER SCREEN FENCE 'B'

ALUMINUM PATIO FENCE (OR OWNER- APPROVED EQUAL)
SCALE: N.T.S.



16- UNIT GARDEN APARTMENT BUILDING MIDDLEBURY CROSSING



EXAMPLE GARAGE UNITS MIDDLEBURY CROSSING



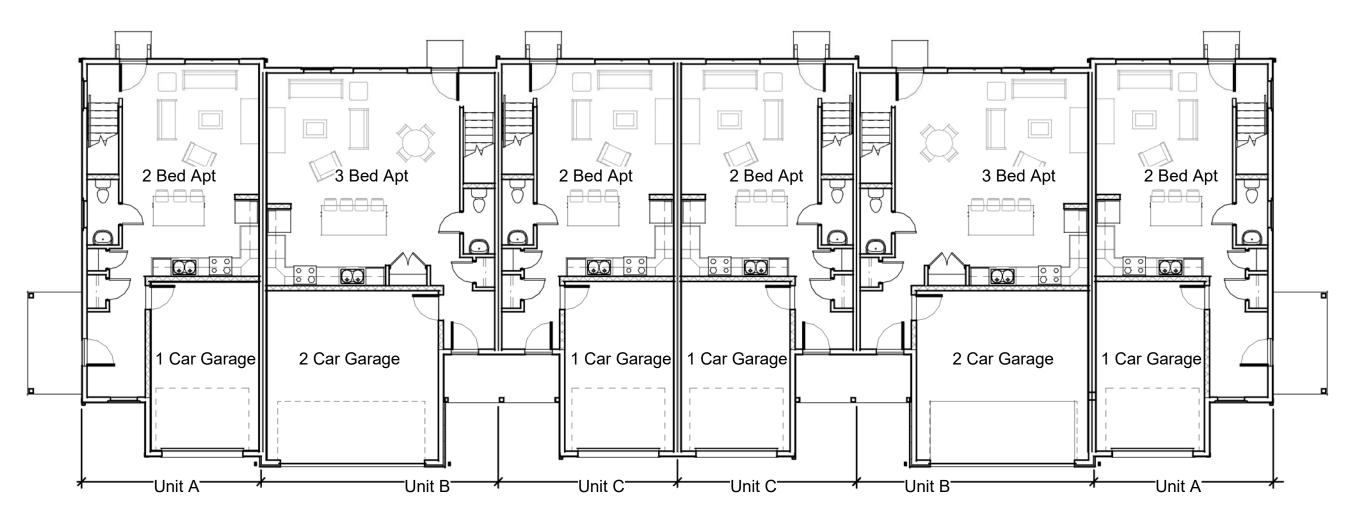
	Color Scheme	
Material	Manufacturer	Color
Horizontal Siding Shake Siding Board and Batten Siding Stone Entry Doors Overhead Garage Doors All Exterior Cementitious Trim All Trim Aluminum Wrapped Soffits	Cementitious 6" Lap Siding Cementitious Shake Cementitious Board and Batten Prestige Sherwin Williams Sherwin Williams Sherwin Williams Aluminum Cementitious	Anew Grey Backdrop Pure White Grandview Limestone Rockwood Red Pure White Pure White White White White
Gutters & Downspouts Roof Shingles	Aluminum Certainteed	White Weather Wood





Color Scheme





First Floor Plan

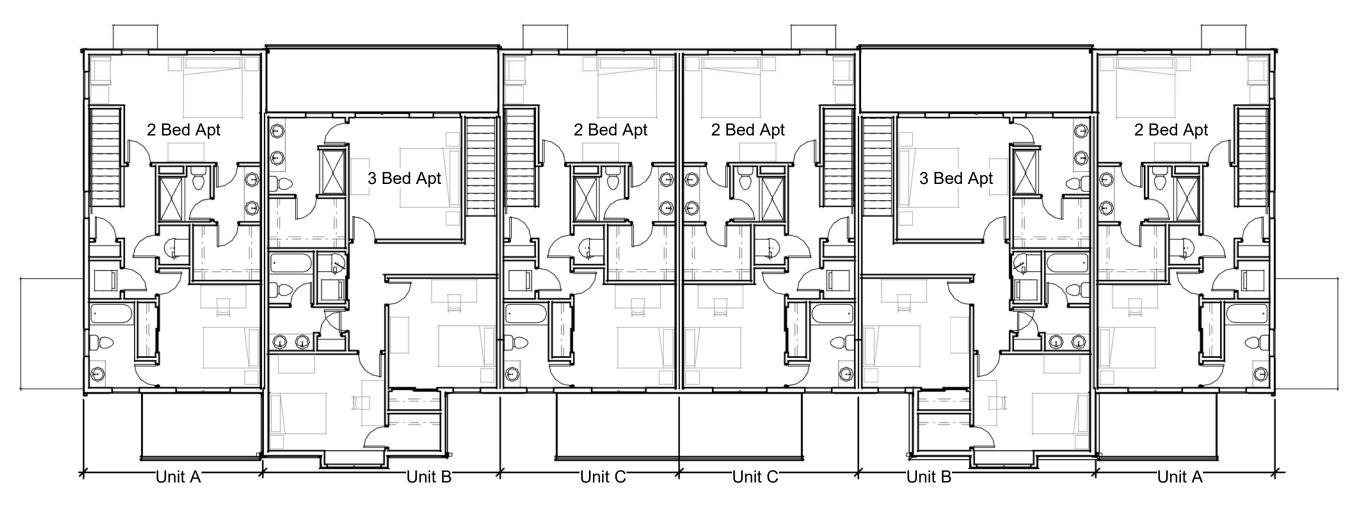
3/32" = 1'-0"

Unit A		Unit B		Unit C		
First Floor sf	594	First Floor sf	690	First Floor sf	549	
Second Floor sf	721	Second Floor sf	909	Second Floor sf	716	
Total sf	1315	Total sf	1518	Total sf	1265	

Middlebury Crossing

Town House Plans





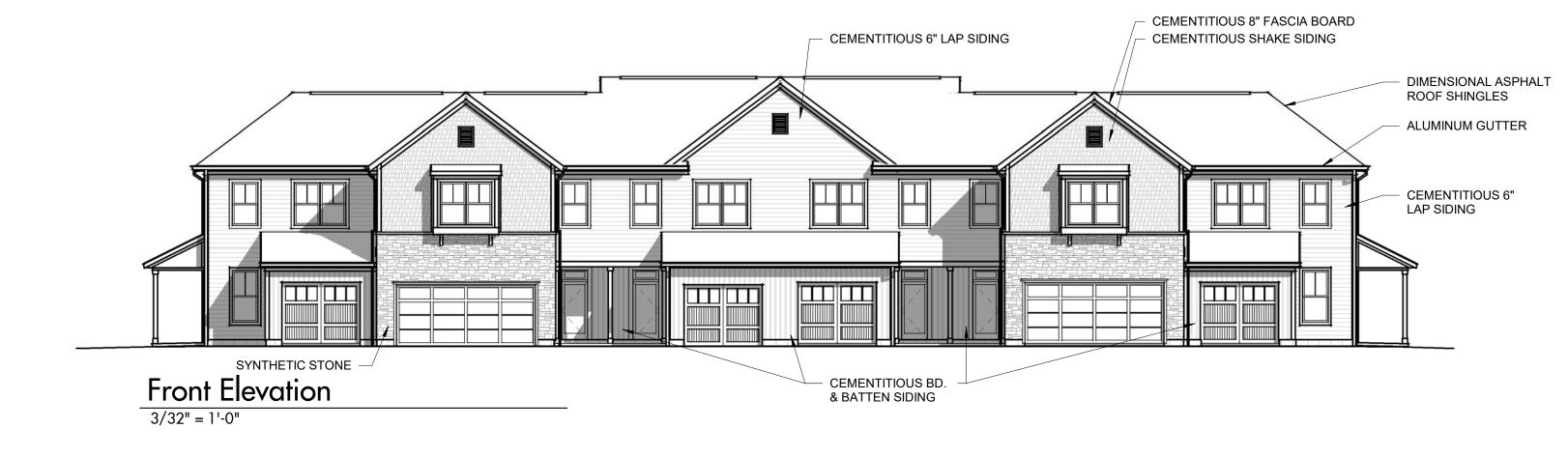
Second Floor Plan

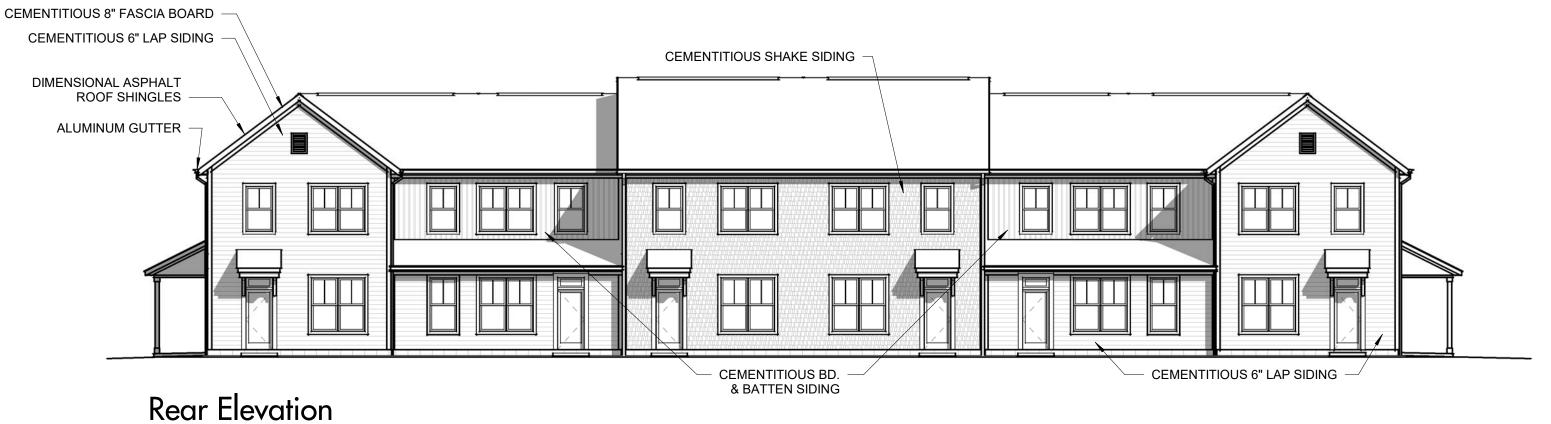
3/32" = 1'-0"









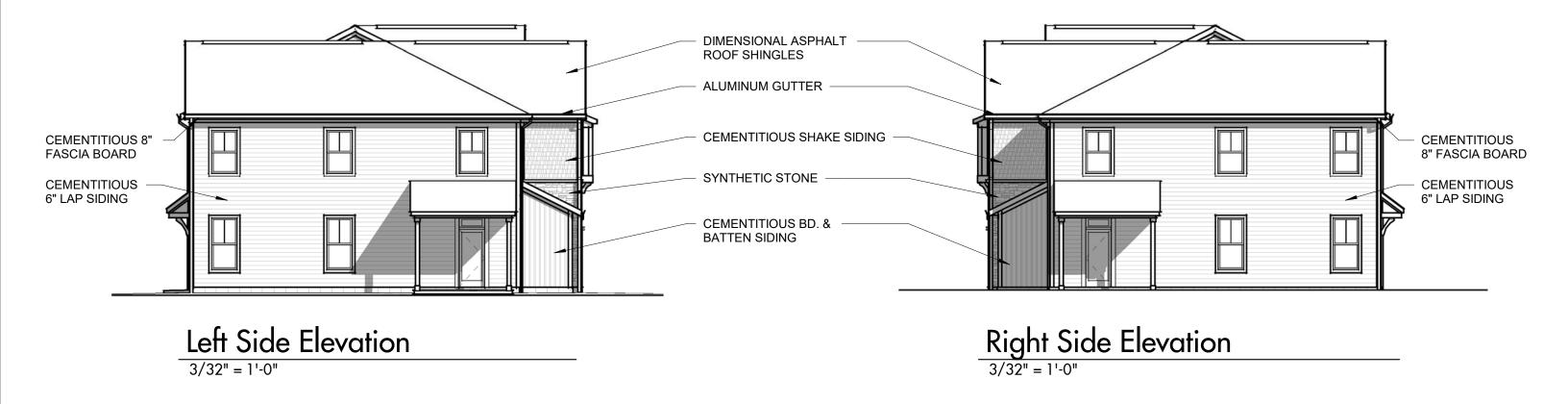


Middlebury Crossing

3/32" = 1'-0"

Town House Elevations



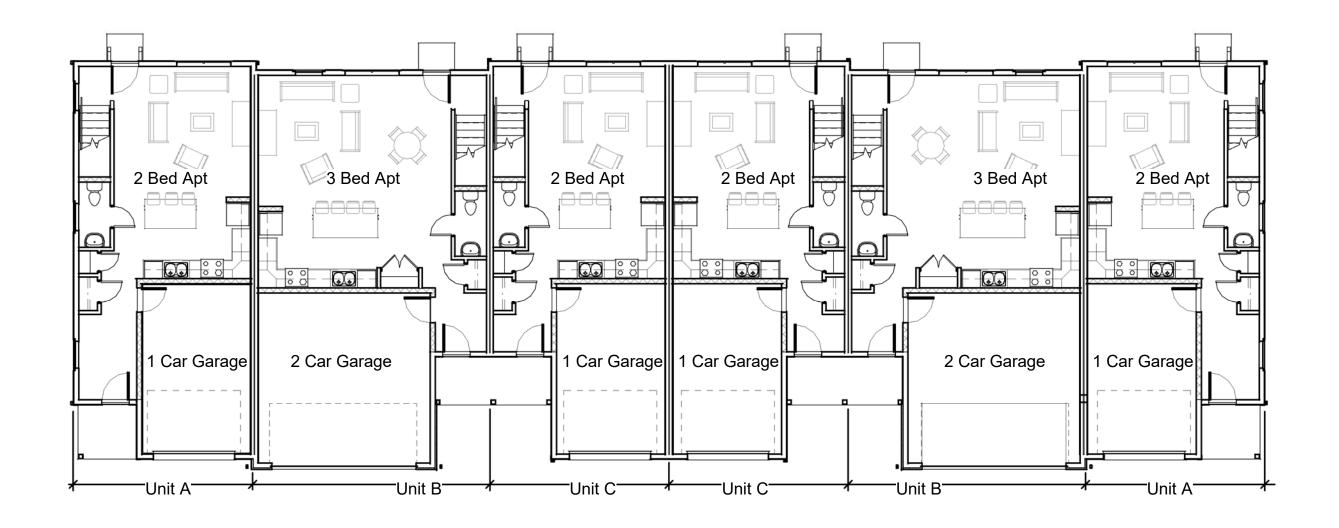


Middlebury Crossing

Town House Elevations



City of Powell, Ohio 04/23/2019

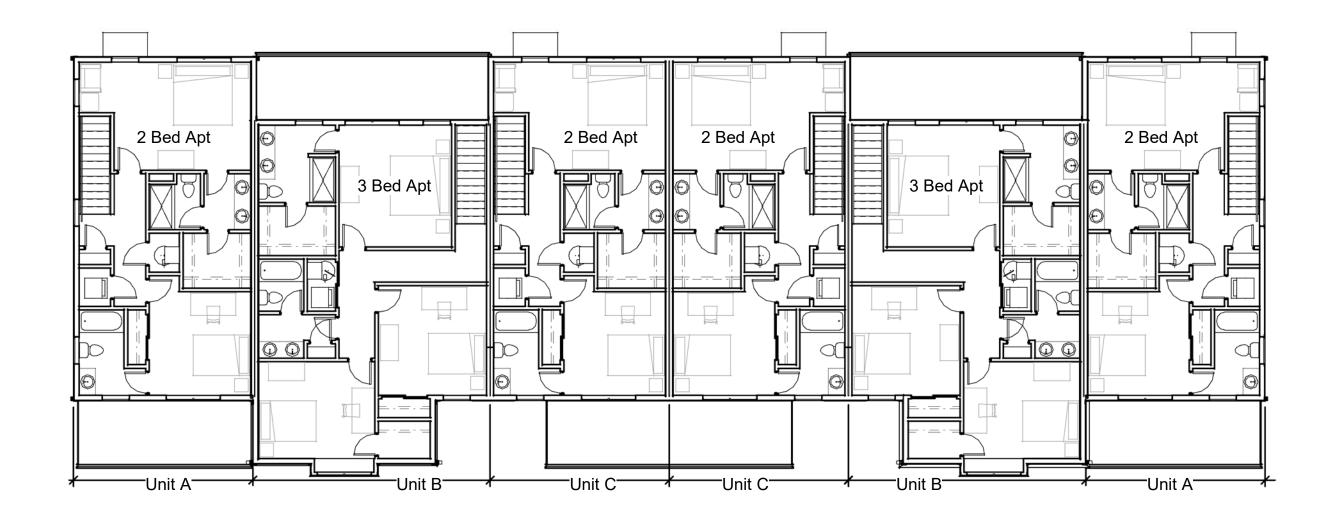


First Floor Plan Option 2 3/32" = 1'-0"

Middlebury Crossing

Town House Plans





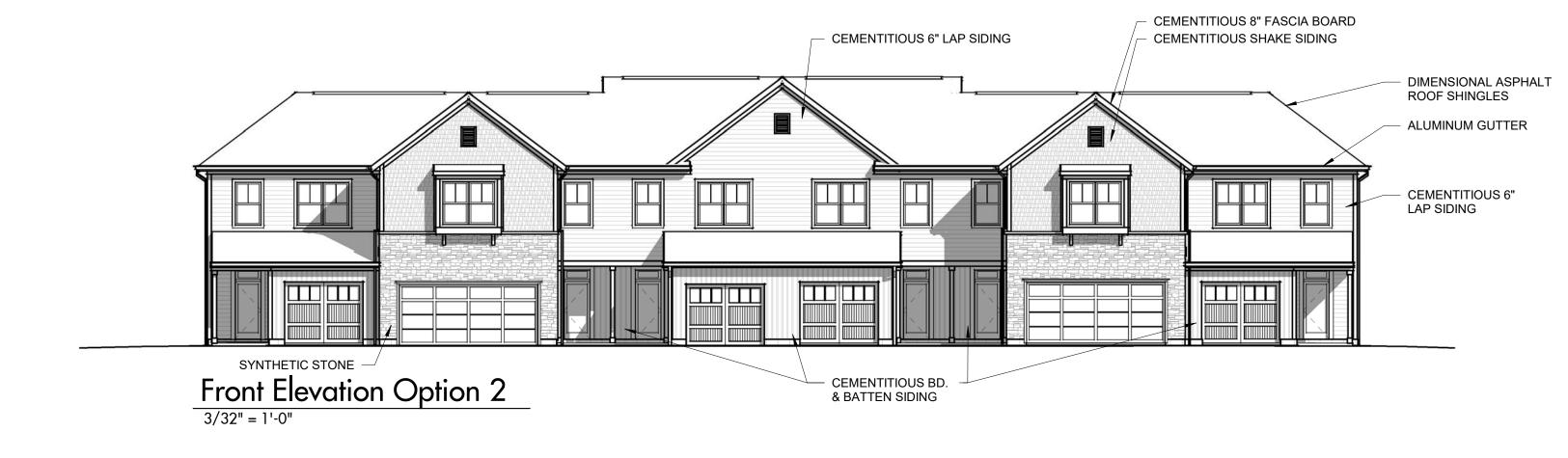
Second Floor Plan Option 2 3/32" = 1'-0"

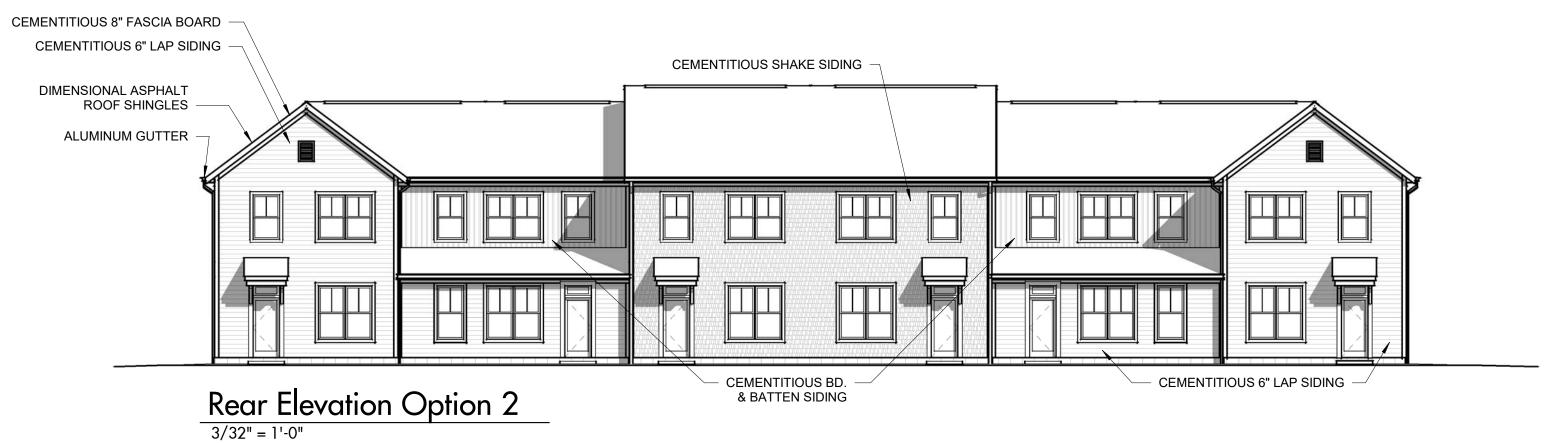
Middlebury Crossing

Town House Plans



City of Powell, Ohio 04/23/2019



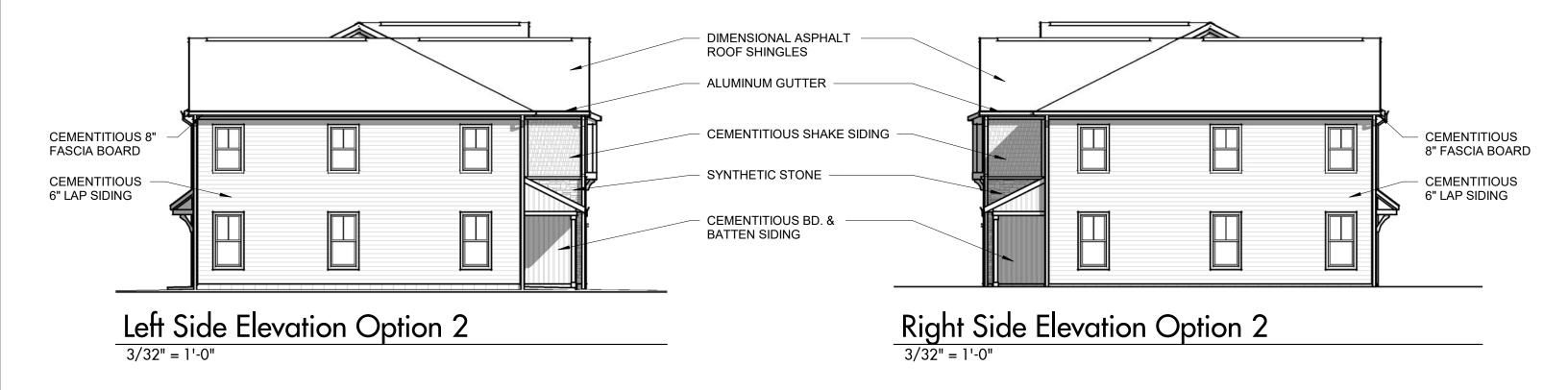


Middlebury Crossing

Town House Elevations



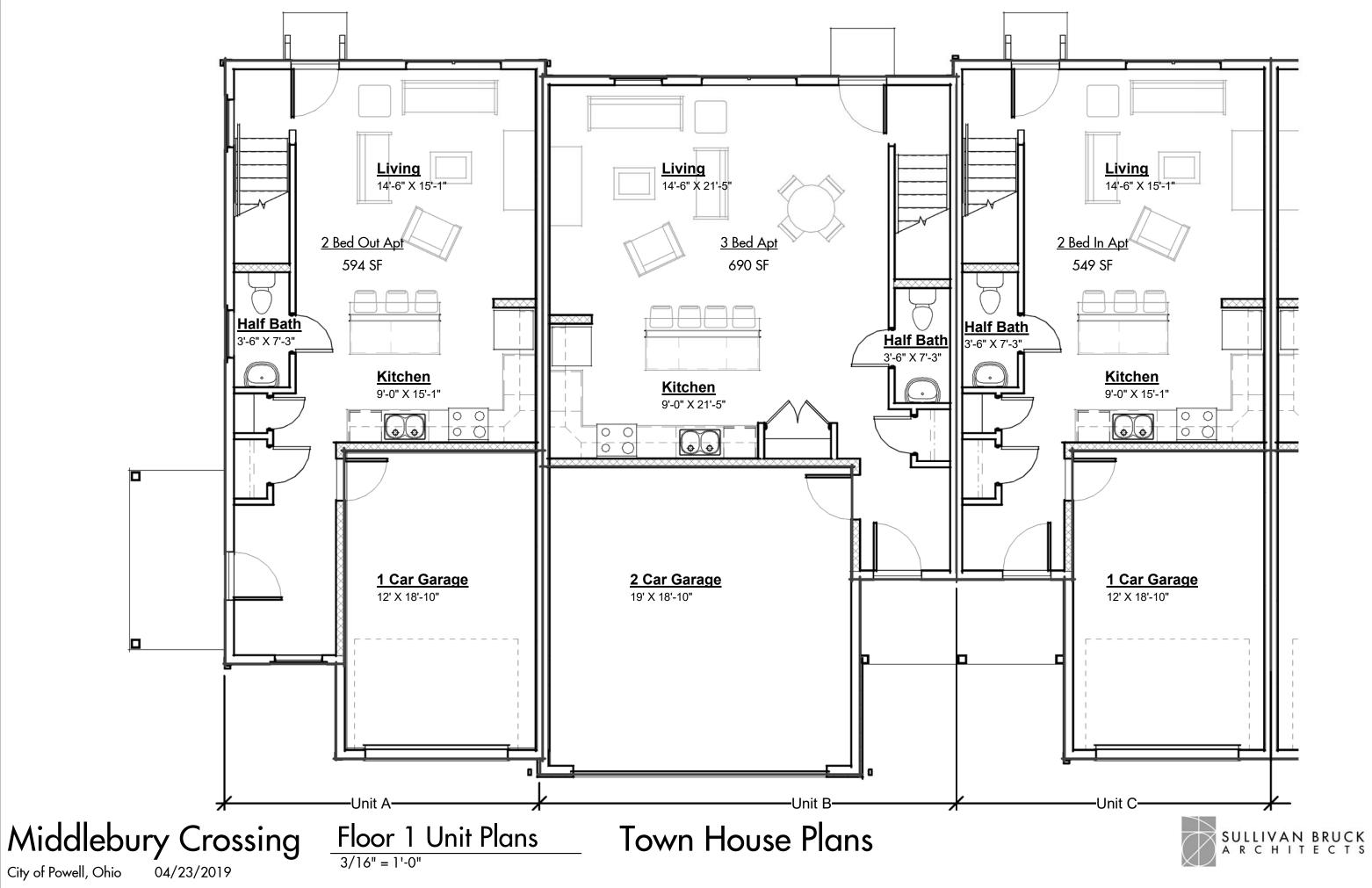
City of Powell, Ohio 04/23/2019

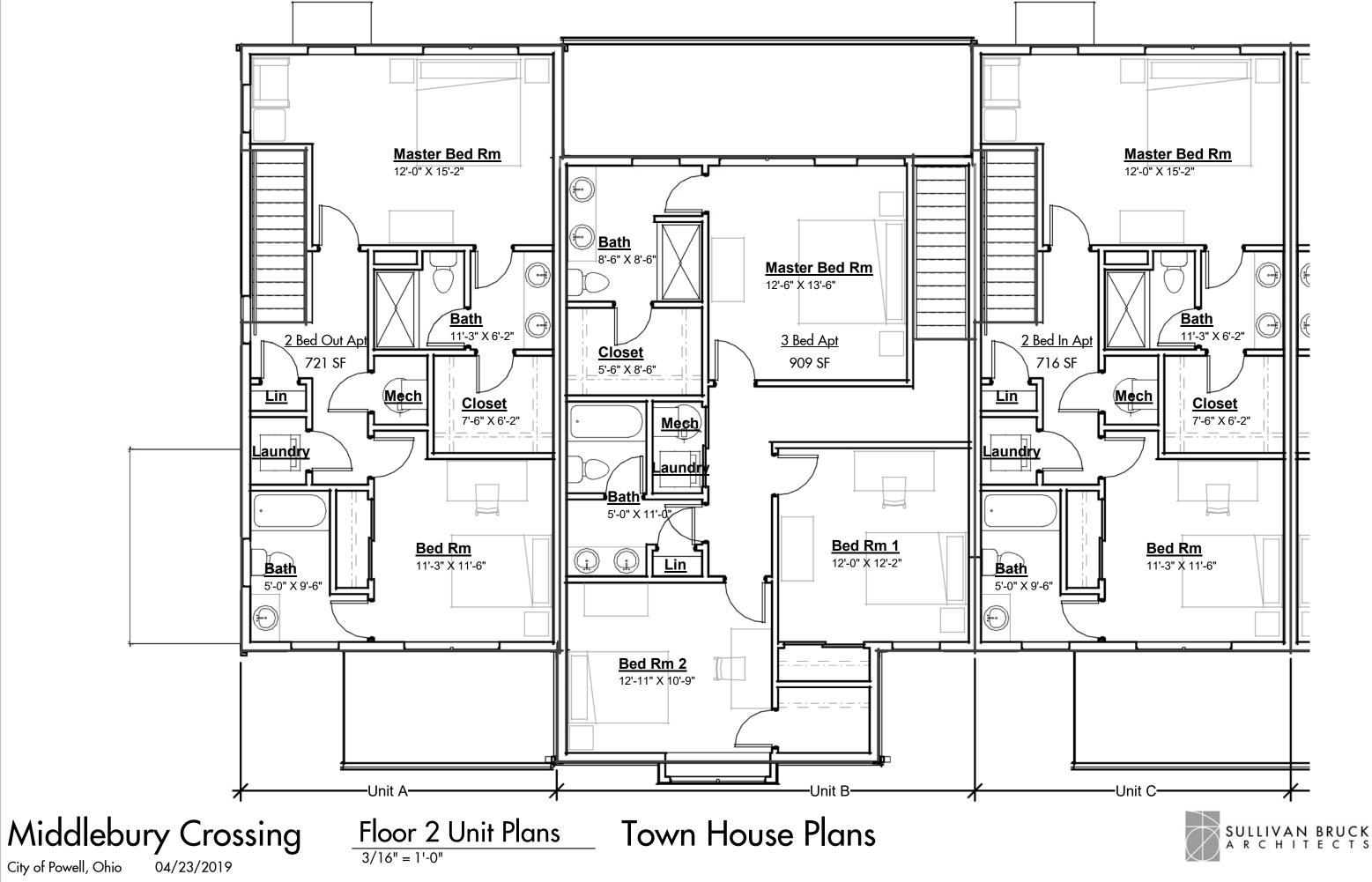


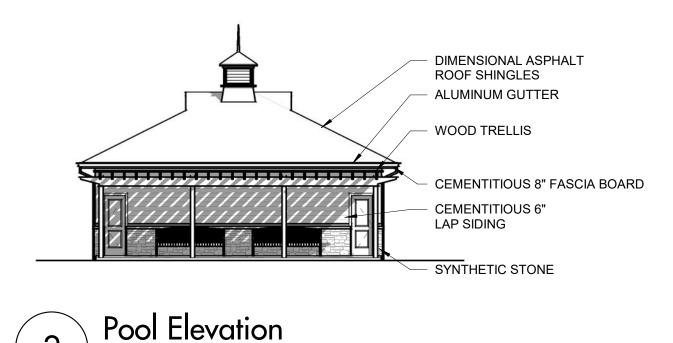
Middlebury Crossing
City of Powell, Ohio 04/23/2019

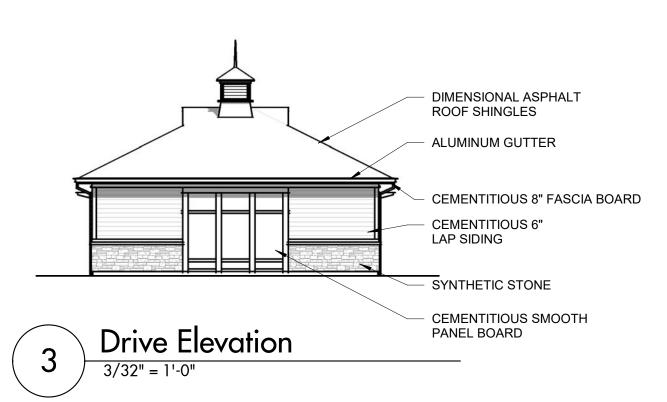
Town House Elevations

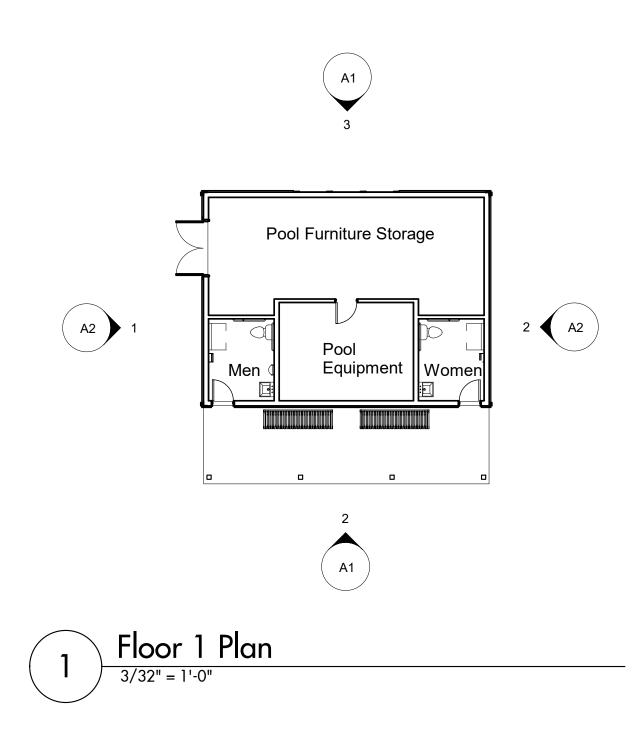










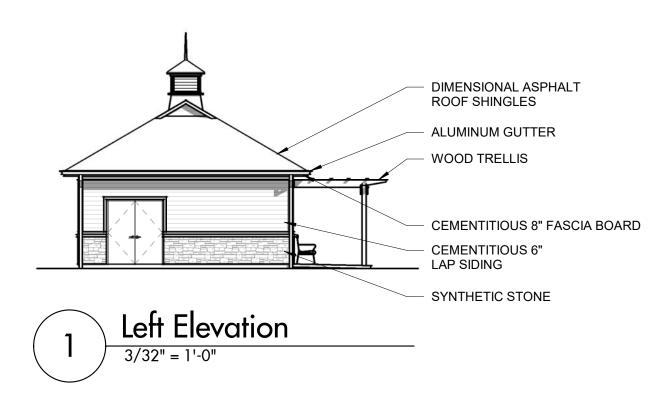


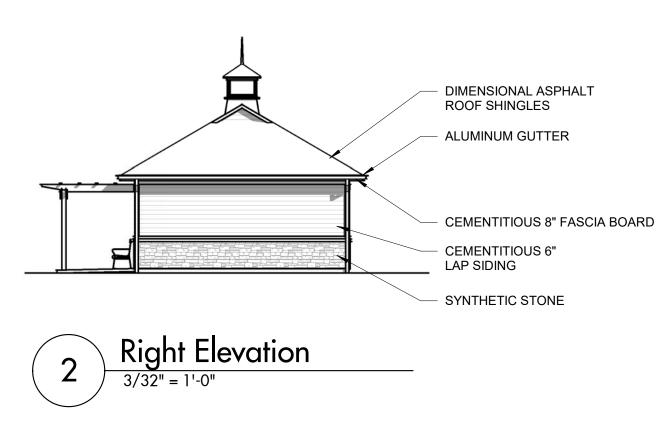
Middlebury Crossing

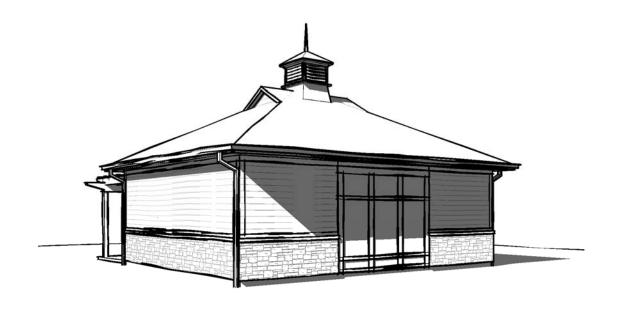
Pool House



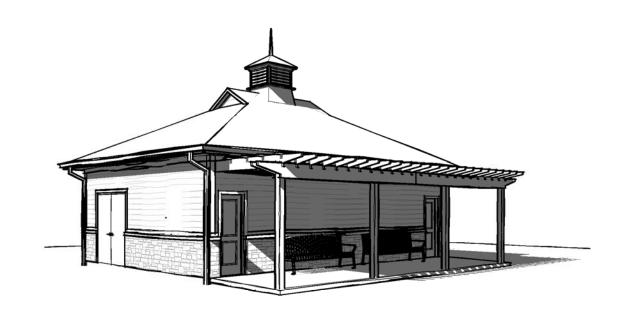
3/32" = 1'-0"







3 3D View 1



3D View 2

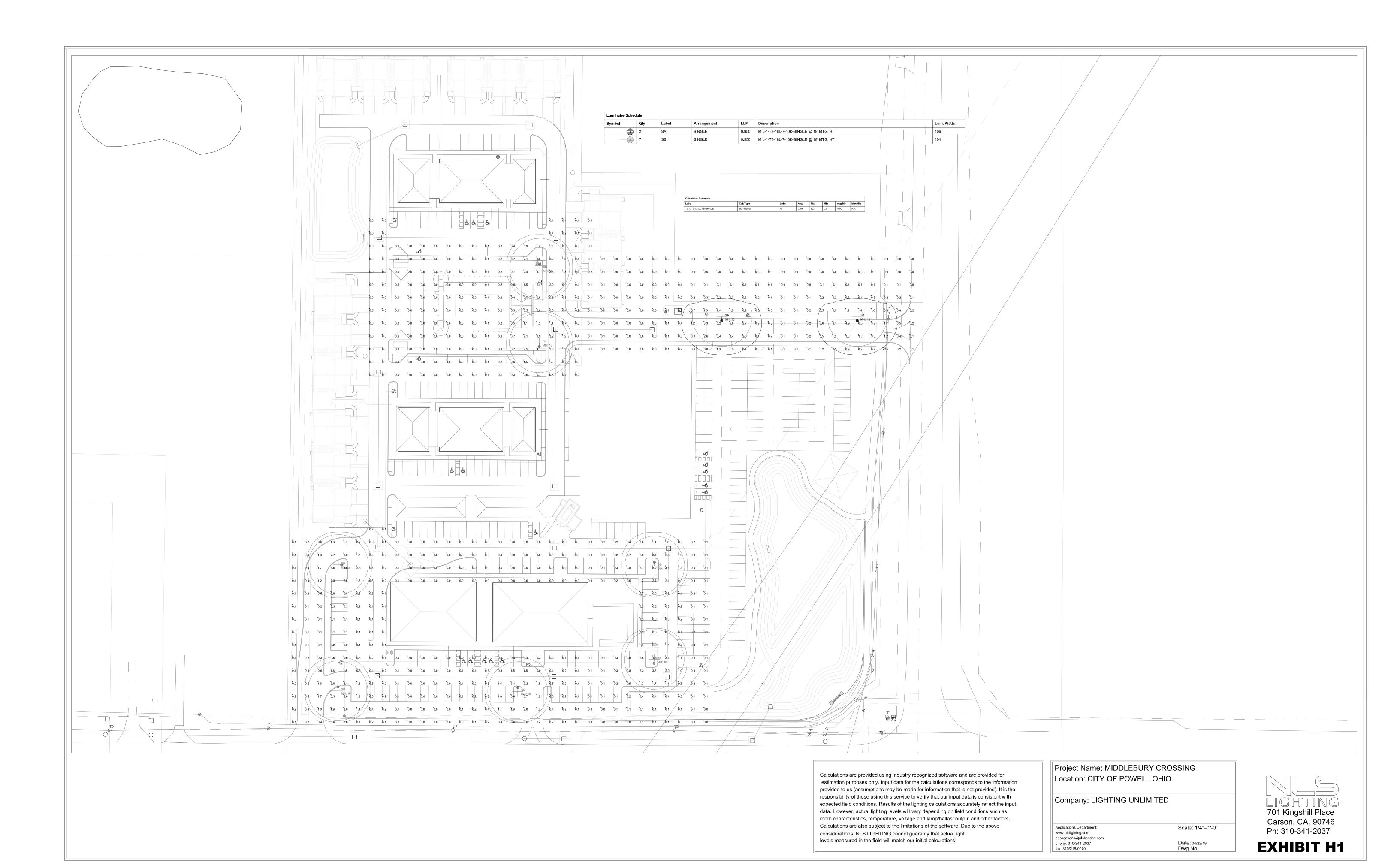
Middlebury Crossing

City of Powell, Ohio

04/18/2019

Pool House







Home & Steitz Mixed-Use

Real Property Design & Development Traffic Impact Study

April 22, 2019



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I. Purpose of Report & Study Objectives

The purpose of this traffic analysis and report is to document the potential traffic impacts of the proposed Home and Steitz Mixed-Use Development on the northwest quadrant of the intersection of Home Road and Steitz Road in Delaware County, Ohio. This analysis and report are being required by City of Powell and Delaware County as part of the development approval process. A Memorandum of Understanding (MOU) was provided to the City of Powell and Delaware County via email. This MOU and comments on said MOU can be seen in **Appendix A**.

II. Proposed Development

A. Off-Site Developments

The study area is located north of the current City of Powell limits in Delaware County, but the site is being annexed into the City limits. The surrounding area includes residential communities and land planned for future development.

B. On-Site Development

Location

The site is located on the northwest quadrant of the intersection of Home Road and Steitz Road in Delaware County, Ohio. **Figure 1** shows the location of the proposed site in central Ohio and **Figure 2** shows the study area.

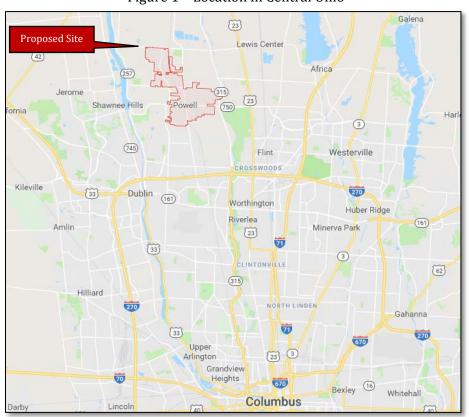


Figure 1 - Location in Central Ohio



Proposed Site

Proposed Site

Home Rd

Figure 2 – Location of Proposed Development (Yellow), Site Drives, & Study Intersections

Land Use & Intensity

The site is currently undeveloped, agricultural land.

The site is proposed to develop as a mixed-use development containing a 25,000 SF library, two office buildings with approximate sizes of 7,000 SF and 7,700 SF for a total of 14,700 SF, and 62 multifamily residential units.

Site Plan

There are two proposed access points along Home Road and one proposed access point on Steitz Road. The westernmost access on Home Road is proposed as full access. The east access on Home Road is proposed as a right-in/right-out and the access on Steitz Road is proposed as a right-in/left-out/right-out. A full site plan can be found in **Appendix A**.

III. Area Conditions

A. Area of Influence

The intersections under study are the following (numbers correspond to **Figure 2**):

- 1. Home Road & Steitz Road
- 2. Home Road & Site Drive 1 (Full Access)
- 3. Home Road & Site Drive 2 (Right-In, Right-Out)
- 4. Home Road & Site Drive 3 (Right-In, Left-Out, Right-Out)



B. Jurisdictions

The proposed site currently resides in Delaware County, but will be annexed into the City of Powell. The traffic study will be reviewed by both agencies.

C. Traffic Volumes & Conditions

AM and PM Peak Hour turning movement counts were collected at the intersection of Home Road & Stietz Road from 7-9 AM and 4-6 PM on a typical weekday in January of 2019. This count data can be found in **Appendix B**.

IV. Projected Traffic

A. Background Traffic

For analysis, the Opening Year of the development is 2019 and the two Design or Horizon Years are 2029 and 2039 (Powell requires a 10-year Horizon and Delaware County requires a 20-year Horizon). In order to project the count data to the Horizon Years, growth rates were obtained from Mid-Ohio Regional Planning Commission (MORPC). The linear, annual growth rates can be seen in **Table 1**.

LocationLinear Annual Growth RateHome Road e/o Steitz Road2.50%Steitz Road n/o Home Road2.00%Home Road w/o Steitz Road2.50%Steitz Road s/o Home Road2.00%

Table 1 - MORPC Growth Rates

Growth rates were applied to the count data to produce Background or No Build traffic for the Opening (2019) and Horizon (2029 and 2039) Years. MORPC correspondence can be seen in **Appendix B**.

B. Site Traffic

Trip Generation

Trips for the proposed site development were generated using standard Institute of Transportation Engineers (ITE) practices and the *Trip Generation Manual*, 10th edition, data via the OTISS program¹. Land Use Codes (*LUC*) 220 – *Multi-Family Housing (Low Rise)*, 590 – *Library*, and 710 – *General Office Building* were used to generate trips for the proposed development. Pass-by and internal capture rates do not apply to this development. **Table 2** shows the trip generation of the proposed development. The full trip generation details can be found in **Appendix C**.

¹ Online Traffic Impact Study Software developed by ITE and Transoft Solutions.



Table 2 – Proposed Site Trip Generation Summary

Land Use	Size	AM I	Peak	PM Peak			
Lanu Use	Size	Entry	Exit	Entry	Exit		
220 - Multi-Family Housing (Low Rise)	62 Dwelling	7	23	24	14		
Non-Pass-By	Units	7	23	24	14		
590 - Library	25,000 SF Gross	21	8	104	112		
Non-Pass-By	Floor Area	21	8	104	112		
710 - General Office Building	7,000 SF Gross	28	5	1	8		
Non-Pass-By	Floor Area	28	5	1	8		
710 - General Office Building	7,700 SF Gross	29	5	2	8		
Non-Pass-By	Floor Area	29	5	2	8		
TOTAL		85	41	131	142		
Non-Pass-By		85	41	131	142		

Trip Distribution & Traffic Volumes

Site traffic was distributed to/from the site based on the count data, knowledge of the surrounding area, and engineering judgment. Site traffic was added to No Build traffic to produce Build traffic for the proposed development. The full volume calculations can be found in **Appendix D**.

V. Traffic Analysis

A. Turn Lane Warrant Analysis

Turn lane warrant analyses were conducted at all site drives using standard Ohio Department of Transportation (ODOT) turn lane warrant graphs. If a turn lane was warranted in any particular scenario, the length was calculated using methodologies in the ODOT Location and Design (L&D) Manual. Warranted turn lanes are represented in the capacity analysis.

B. Capacity Analysis

The Highway Capacity Manual (HCM) module of Synchro Version 10 software was used to analyze capacity at all study intersections shown in **Figure 2**. A minimum Level of Service (LOS) of D for the overall intersection and LOS D for each individual movement during peak traffic hours was considered acceptable at each intersection. If an intersection fell below these criteria, mitigation strategies were developed to bring each movement back to an acceptable LOS.

VI. Results

A. Turn Lane Warrant Analysis

Based on the results of the turn lane warrant analysis, the intersection of Home Road & Site Drive 1 warrants 345' left and right turn lanes in all Build scenarios. The intersection of Home Road & Site Drive 2 also warrants a 345' right turn lane in all Build scenarios. All turn lanes are inclusive of a 50' diverging taper. The turn lane warrant graphs leading to these results can be seen in **Appendix E**.



B. Capacity Analysis

Total

Results of the baseline capacity analysis for the study intersections are provided in **Table 3**. Baseline capacity analysis assumes existing geometry plus warranted turn lanes. The total delay for stop-controlled intersections is represented by the worst approach LOS. The full capacity analysis can be found in **Appendix F**.

2019 2029 2039 Intersection Approach AM AM AM No PM No PM AM No PM No PM AM No AM PM No **PM** Build EB B/13.5 B/15.4 B/11.9 B/13.5 B/13.9 B/12.9 B/12.5 B/11.9 B/12.4 B/12.3 B/10.5 B/10.0 WB B/12.3 B/14.2 B/14.7 B/17.4 B/12.8 B/11.9 B/16.2 B/16.2 B/11.5 B/11.5 B/14.5 B/14.5 Home Road & NB A/9.1 A/9.5 A/9.2 B/10.9 B/13.1 B/13.2 B/13.1 B/15.3 B/18.4 B/18.7 B/18.0 C/20.9Steitz Road SB B/10.9 A/8.6 A/9.2A/8.9 B/12.2 B/12.6 B/12.6 B/15.2 B/16.8 B/17.6 B/17.2 C/20.6B/13.0 B/12.9 **Total** B/12.5 B/14.0B/15.1 B/13.4 B/12.6 B/14.5 B/14.5 B/12.9 B/13.3 B/13.6 EΒ A/0.5A/1.1 A/0.4A/1.0 A/0.4A/0.9 ------------WB A/0.0 A/0.0 A/0.0 A/0.0 ---A/0.0A/0.0Home Road & ---------------C/20.4 Site Drive 1 SB B/14.0 ---C/23.4 C/16.7D/34.9 F/59.2 C/20.4Total B/14.0 C/23.4 F/59.2 ---C/16.7D/34.9 ---------EB A/0.0 A/0.0 A/0.0 A/0.0 A/0.0 A/0.0 WB A/0.0 A/0.0 A/0.0 A/0.0 A/0.0 Home Road & A/0.0Site Drive 2 SB B/10.4 B/14.2 B/10.9 C/16.3B/11.6 C/19.4Total B/10.4 B/14.2 B/10.9C/16.3B/11.6 C/19.4EΒ A/8.6 A/8.7 A/8.6 A/8.8 A/8.7 A/8.8 NB A/0.0Home Road & A/0.0A/0.0A/0.0A/0.0A/0.0Site Drive 3 SB A/0.0A/0.0A/0.0A/0.0A/0.0A/0.0

Table 3 - Baseline Capacity Analysis Summary

As seen in **Table 3** above, the southbound movement of Site Drive 1 in the 2039 PM Build is the only scenario with unacceptable LOS. All other intersections have acceptable LOS in all analysis scenarios.

A/8.6

A/8.8

A/8.7

A/8.8

A/8.7

VII. Recommendations and Conclusions

A/8.6

Based on the results of the turn lane warrant and capacity analysis, it is recommended that right turn lanes be provided at both access points along Home Road. Based on the locations of the proposed access points, it is recommended that a 200' westbound right turn lane, inclusive of a 50' diverging taper, be installed along Home Road at Site Drive 1. It is also recommended that a 300' westbound right turn lane, inclusive of a 50' diverging taper, be installed along Home Road at Site Drive 2. It is recommended that the warranted eastbound left turn lane at Site Drive 1 not be installed. There is currently a two-way left turn lane at this location which can sufficiently service the Site Drive 1 access point without negatively affecting surrounding area developments.

No additional improvements are required and thus, none are recommended.



VIII. Appendices

Appendix A - MOU & Site Plan

Appendix B - Count Data & MORPC Correspondence

Appendix C - Trip Generation

Appendix D - Volume Development

Appendix E - Turn Lane Warrant Analysis

Appendix F - Capacity Analysis



Appendix A MOU & Site Plan



From: Gina Balsamo
To: Love, Michael

Cc: <u>David Betz; Drew Laurent; Chris Huber; Piccin, John</u>

Subject: RE: Home & Steitz TIS MOU

Date: Tuesday, January 15, 2019 11:39:22 AM

Attachments: <u>image001.png</u>
TripGeneration.pdf

TripGeneration.pdf Home Steitz Site Plan.pdf

Mike,

Thank you for the response, Mike. We have revised the MOU, see table below.

Attached is the site plan and also the trip generation. It looks like we are under 400 trips. The City is requesting a 20-year Horizon, so we will complete a 10-year and 20-year Horizon analysis. We comply with all other comments.

Chris and Mike,

Please respond to this email signifying your concurrence with this MOU or reply with comments if necessary.

Data Collection	Collect 2-hours AM and 2-hours PM Peak hour turning movement count data at the intersection of Home Road & Steitz Road.
Analysis	Generate traffic for the proposed site. Assign traffic to the site drives according to a traffic distribution which will be determined from counts, area knowledge/travel patterns, and engineering judgment.
Analysis	Develop Opening Day (2019) and Horizon Year (2029 & 2039) traffic plates for Build, AM and PM peaks based on growth data (that will be obtained from MORPC or calculated from previous traffic studies or count data) and the traffic distribution developed above.
Analysis	Perform left and right turn lane warrants at the three proposed access points using ODOT and DCEO methodologies. Design lengths for any turn lanes which meet these warrants per ODOT L&D manual.
Analysis	Perform a Synchro unsignalized capacity analysis at all site drive intersections with Home Road & Steitz Road. Perform signalized capacity analysis at the intersection of Home Road & Steitz Road. If acceptable LOS is not obtained under Build or No Build conditions, determine what improvements are necessary to obtain acceptable LOS.
Report	Develop a report that documents what is necessary to satisfy the City of Powell & Delaware County which typically includes analysis, results, conclusions, and recommendations. An evaluation for the need of two site drives on Home Rd and the one site drive on Steitz Rd, the permitted movements at each site drive, and their proximity to each other and the traffic signal/turn lanes will be included.

Gina Balsamo, PE

Carpenter Marty Transportation 614.656.2429

From: Love, Michael <mlove@co.delaware.oh.us>

Sent: Monday, January 14, 2019 8:02 PM **To:** Gina Balsamo <gbalsamo@cmtran.com>

Cc: David Betz <DBetz@cityofpowell.us>; Drew Laurent <dlaurent@cmtran.com>; Chris Huber

<CHuber@cityofpowell.us>; Piccin, John <jpiccin@co.delaware.oh.us>

Subject: RE: Home & Steitz TIS MOU

Gina

A couple items

- 1. All development needs to be submitted to John Piccin (he is copied on this email). John is in charge of Development for DCEO. Any submittal to DCEO will need to follow DCEO development process (i.e. Review Submittal Coversheet, fees, electronic files, etc.)
- 2. If you are under 400 peak hour trips, then a 10 year horizon (build) is required. Over 400 trips requires a 20 year horizon
- 3. Turn lane warrants shall be as per DCEO standards, Not ODOT standards.
- 4. Need to include turn lane warrants for right turns and left turns
- 5. Turn lane lengths shall be per ODOT L&D manual
- 6. Can you include the current site plan with the MOU?
- 7. The proposed drive on Steitz Road will need evaluated. In this situation, a RI-RO (or LI-RI-RO) will not work. The location is relative to the signal and the existing turn lane (SB) is critical. The drive cannot be located in any shift taper, so I am not sure where this drive can be located without any pavement widening.

Thanks

Mike

From: Gina Balsamo [mailto:gbalsamo@cmtran.com]

Sent: Monday, January 14, 2019 2:45 PM **To:** Chris Huber < CHuber@cityofpowell.us>

Cc: Love, Michael <<u>mlove@co.delaware.oh.us</u>>; David Betz <<u>DBetz@cityofpowell.us</u>>; Drew Laurent

<<u>dlaurent@cmtran.com</u>>

Subject: Home & Steitz TIS MOU

CAUTION EXTERNAL EMAIL

Chris/Mike,

Please see the below email MOU for the traffic impact study for a proposed mixed-use development in the northwest corner of Home Road/Steitz Road. Please respond to this email signifying your concurrence with this MOU or reply with comments if necessary.

	1
Data Collection	Collect 2-hours AM and 2-hours PM Peak hour turning movement count data at the intersection of Home Road & Steitz Road.
Analysis	Generate traffic for the proposed site. Assign traffic to the site drives according to a traffic distribution which will be determined from counts, area knowledge/travel patterns, and engineering judgment.
Analysis	Develop Opening Day (2019) and Horizon Year (2039) traffic plates for Build, AM and PM peaks based on growth data (that will be obtained from MORPC or calculated from previous traffic studies or count data) and the traffic distribution developed above.
Analysis	Perform turn lane warrants at the three proposed access points using ODOT methodologies. Design lengths for any turn lanes which meet these warrants.
Analysis	Perform a Synchro unsignalized capacity analysis at all site drive intersections with Home Road & Steitz Road. Perform signalized capacity analysis at the intersection of Home Road & Steitz Road. If acceptable LOS is not obtained under Build or No Build conditions, determine what improvements are necessary to obtain acceptable LOS.
Report	Develop a report that documents what is necessary to satisfy the City of Powell & Delaware County which typically includes analysis, results, conclusions, and recommendations. An evaluation for the need of two site drives on Home Rd, the permitted movements at each site drive, and their proximity to each other will be included.

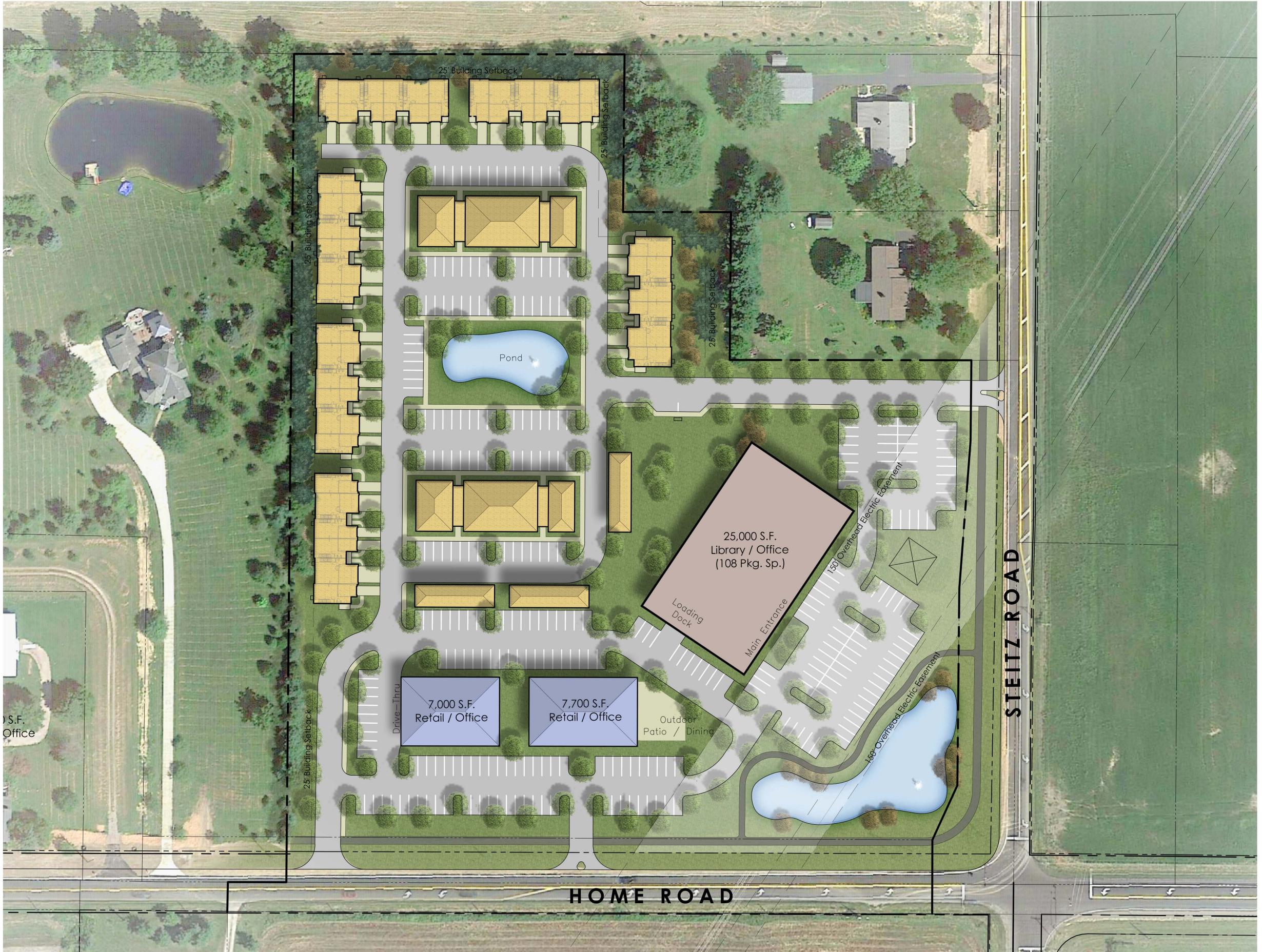
Thank you!

Gina Balsamo, PE
Traffic Engineer
CARPENTER
MARTY transportation

614.656.2429 | www.cmtran.com

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SITE DATA

Gross Acreage:

Commercial / Institutiona Uses:

Proposed Library

± 25,000 s.f.

+/- 11.75 Ac

Commercial / Office

± 14,700 s.f.

104 Parking Spaces

108 Parking spaces

Residential Uses:

(36) 2-story Town-homes Units

(32) 2-Story Garden Units

Home and Steitz Road - Concept Plan





Appendix B Count Data & MORPC Correspondence



Tue Jan 29, 2019

Full Length (7AM-9AM, 4PM-6PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Movements

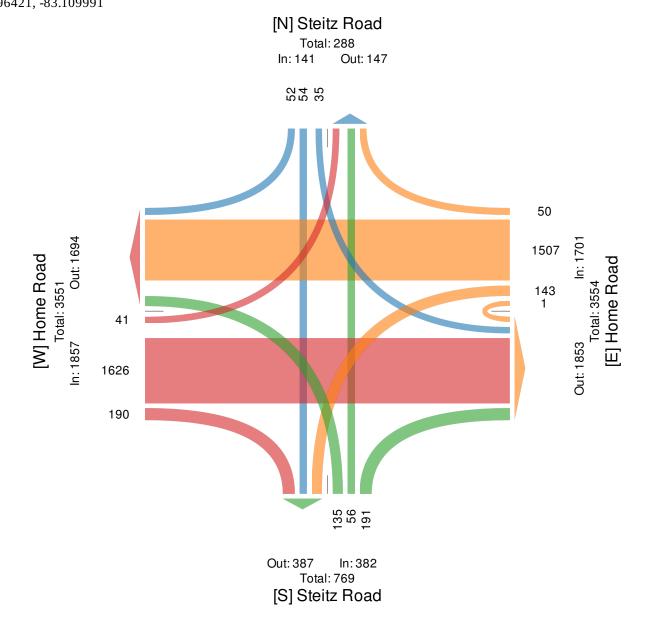
ID: 614890, Location: 40.196421, -83.109991

Leg	Home R	oad				Home R	oad				Ste itz Ro	ad			St	te itz Ro	ad				
Dire ction	Eastbou	nd				Westbou	ınd				Northbou	ınd			Sc	outhbou	ınd				
Time	L	Т	R	U	App	L	Т	R	U	App	L	Т	R	U A	рр	L	T	R	U	App	Int
2019-01-29 7:00AM	5	136	11	0	152	25	73	2	0	100	6	3	45	0	54	2	5	4	0	11	317
7:15 AM	1	103	16	0	120	19	87	3	0	109	2	4	12	0	18	0	4	4	0	8	255
7:30AM	1	107	7	0	115	6	55	1	0	62	3	6	8	0	17	3	6	5	0	14	208
7:45AM	3	143	12	0	158	5	58	3	0	66	3	6	16	0	25	1	3	4	0	8	257
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8:00AM	0	103	9	0	112	4	64	0	0	68	4	4	9	0	17	2	2	5	0	9	206
8:15AM	6	77	10	0	93	3	58	2	0	63	5	2	6	0	13	3	6	6	0	15	184
8:30AM	3	127	9	0	139	8	51	3	0	62	3	0	22	0	25	5	4	3	0	12	238
8:45 AM	3	98	18	0	119	8	60	4	1	73	5	1	19	0	25	4	6	2	0	12	229
Hourly Total	. 12	405	46	0	463	23	233	9	1	266	17	7	56	0	80	14	18	16	0	48	857
9:00AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	. 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00PM	2	85	11	0	98	14	99	2	0	115	18	6	4	0	28	1	4	1	0	6	247
4:15PM	0	86	17	0	103	7	106	4	0	117	12	0	8	0	20	1	3	1	0	5	245
4:30PM	1	89	11	0	101	5	114	3	0	122	17	3	7	0	27	0	2	1	0	3	253
4:45PM	0	88	11	0	99	6	128	2	0	136	8	5	12	0	25	2	4	1	0	7	267
Hourly Total	. 3	348	50	0	401	32	447	11	0	490	55	14	31	0 1	100	4	13	4	0	21	1012
5:00PM	5	109	11	0	125	15	120	4	0	139	15	5	6	0	26	3	0	4	0	7	297
5:15PM	3	89	8	0	100	7	158	8	0	173	9	4	4	0	17	5	1	4	0	10	300
5:30PM	5	101	14	0	120	6	147	6	0	159	15	3	4	0	22	0	1	5	0	6	307
5:45PM	3	85	15	0	103	5	127	3	0	135	10	4	9	0	23	3	3	2	0	8	269
Hourly Total	16	384	48	0	448	33	552	21	0	606	49	16	23	0	88	11	5	15	0	31	1173
6:00PM	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	2
Hourly Total	. 0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	2
Total	41	1626	190	0	1857	143	1507	50	1	1701	135	56	191	0 3	82	35	54	52	0	14 1	4081
% Approach	2.2%	87.6%	10.2%	0%	-	8.4%	88.6%	2.9%	0.1%	-	35.3%	14.7%	50.0% 0	%	- 2	24.8%	38.3%	36.9%	0%	_	-
% Total	1.0%	39.8%	4.7%	0%	45.5%	3.5%	36.9%	1.2%	0%	41.7%	3.3%	1.4%	4.7% 0	% 9.4	1 %	0.9%	1.3%	1.3%	0%	3.5%	-
Lights	40	1590	189	0	1819	137	1466	45	1	1649	133	54	188	0 3	75	34	53	50	0	137	3980
% Lights	97.6%	97.8%	99.5%	0%	98.0%	95.8%	97.3%	90.0%	100%	96.9%	98.5%	96.4%	98.4% 0	98. 2	2% 9	97.1%	98.1%	96.2%	0%	97.2%	97.5%
Articulated Trucks	0	1	0	0	1	0	2	2	0	4	0	1	0	0	1	0	0	0	0	0	6
% Articulated Trucks	0%	0.1%	0%	0%	0.1%	0%	0.1%	4.0%	0%	0.2%	0%	1.8%	0% 0	% 0.3	3%	0%	0%	0%	0%	0 %	0.1%
Buses and Single-Unit Trucks	1	35	1	0	37	6	39	3	0	48	2	1	3	0	6	1	1	2	0	4	95
% Buses and Single-Unit Trucks	2.4%	2.2%	0.5%	0%	2.0%	4.2%	2.6%	6.0%	0%	2.8%	1.5%	1.8%	1.6% 0	% 1.6	6%	2.9%	1.9%	3.8%	0%	2.8%	2.3%

^{*}L: Left, R: Right, T: Thru, U: U-Turn

Page B2 of 9 1 of 6

Tue Jan 29, 2019
Full Length (7AM-9AM, 4PM-6PM)
All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)
All Movements
ID: 614890, Location: 40.196421, -83.109991



Tue Jan 29, 2019

AM Peak (7AM - 8AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

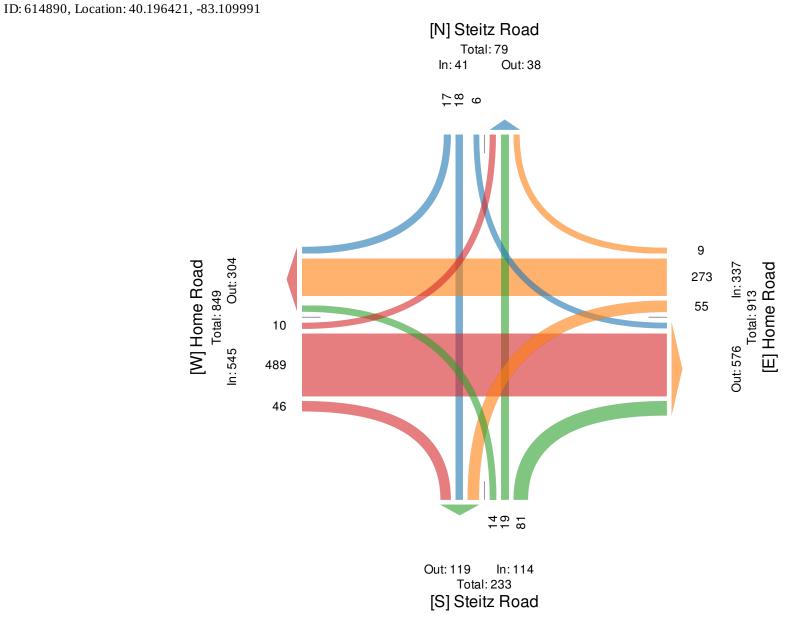
All Movements

ID: 614890, Location: 40.196421, -83.109991

Leg	Home F	Road				Home Ro	oad				Steitz Roa	ad				Ste itz Ro	ad				
Direction	Eastbou	ınd				Westbou	nd				Northbou	nd				Southbou	ınd				
Time	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	Int
2019-01-29 7:00AM	5	136	11	0	152	25	73	2	0	100	6	3	45	0	54	2	5	4	0	11	317
7:15 AM	1	103	16	0	120	19	87	3	0	109	2	4	12	0	18	0	4	4	0	8	255
7:30AM	1	107	7	0	115	6	55	1	0	62	3	6	8	0	17	3	6	5	0	14	208
7:45AM	3	143	12	0	158	5	58	3	0	66	3	6	16	0	25	1	3	4	0	8	257
Total	10	489	46	0	545	55	273	9	0	337	14	19	81	0	114	6	18	17	0	41	1037
% Approach	1.8%	89.7%	8.4%	0%	-	16.3%	81.0%	2.7% 0	1%	-	12.3%	16.7%	71.1%	0%	-	14.6%	43.9%	41.5%	0%	-	-
% Total	1.0%	47.2%	4.4%	0%	52.6%	5.3%	26.3%	0.9% 0	1%	32.5%	1.4%	1.8%	7.8%	0%	11.0%	0.6%	1.7%	1.6%	0%	4.0%	-
PHF	0.500	0.855	0.719	-	0.862	0.550	0.784	0.750	-	0.773	0.583	0.792	0.450	-	0.528	0.500	0.750	0.850	-	0.732	0.818
Lights	10	479	46	0	535	49	259	8	0	316	13	18	81	0	112	6	18	17	0	41	1004
% Lights	100%	98.0%	100%	0%	98.2%	89.1%	94.9%	88.9% 0	1%	93.8%	92.9%	94.7%	100%	0%	98.2%	100%	100%	100%	0%	100%	96.8%
Artic ulate d Truc ks	0	0	0	0	0	0	2	1	0	3	0	1	0	0	1	0	0	0	0	0	4
% Articulated Trucks	0%	0%	0%	0%	0 %	0%	0.7%	11.1% 0	1%	0.9%	0%	5.3%	0%	0%	0.9%	0%	0%	0%	0%	0%	0.4%
Buses and Single-Unit Trucks	0	10	0	0	10	6	12	0	0	18	1	0	0	0	1	0	0	0	0	0	29
% Buses and Single-Unit Trucks	0%	2.0%	0%	0%	1.8 %	10.9%	4.4%	0% 0	%	5.3%	7.1%	0%	0%	0%	0.9%	0%	0%	0%	0%	0%	2.8%

^{*}L: Left, R: Right, T: Thru, U: U-Turn

Tue Jan 29, 2019 AM Peak (7AM - 8AM) All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks) All Movements



Provided by: Carpenter Marty (CM) Transportation Inc. 6612 Singletree Drive, Columbus, OH, 43229, US

Tue Jan 29, 2019

PM Peak (5PM - 6PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

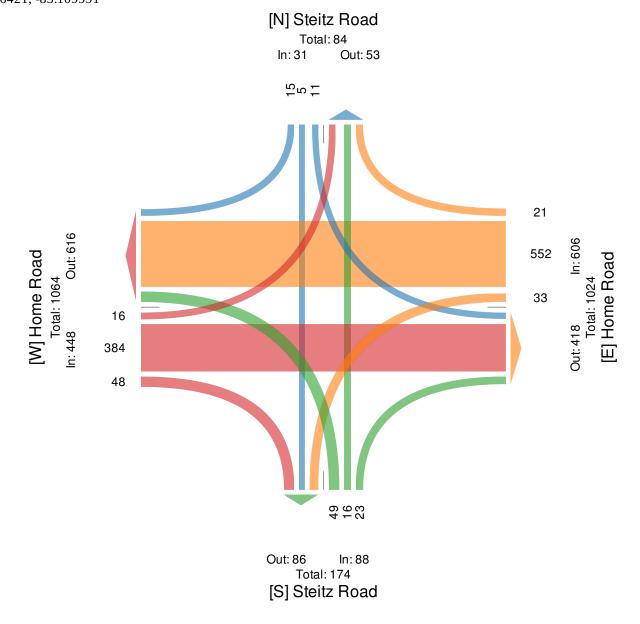
All Movements

ID: 614890, Location: 40.196421, -83.109991

Leg	Home l	Road				Home F	Road				Steitz Roa	ıd				Steitz Ro	ad				
Dire ction	Eastbou	ınd				Westbo	und				Northbou	nd				Southbou	ınd				
Time	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	Int
2019-01-29 5:00PM	5	109	11	0	125	15	120	4	0	139	15	5	6	0	26	3	0	4	0	7	297
5:15PM	3	89	8	0	100	7	158	8	0	173	9	4	4	0	17	5	1	4	0	10	300
5:30PM	5	101	14	0	120	6	147	6	0	159	15	3	4	0	22	0	1	5	0	6	307
5:45PM	. 3	85	15	0	103	5	127	3	0	135	10	4	9	0	23	3	3	2	0	8	269
Total	16	384	48	0	448	33	552	21	0	606	49	16	23	0	88	11	5	15	0	31	1173
% Approach	3.6%	85.7%	10.7%	0%	-	5.4%	91.1%	3.5%	0%	-	55.7%	18.2%	26.1%	0%	-	35.5%	16.1%	48.4%	0%	-	-
% Total	1.4%	32.7%	4.1%	0%	38.2%	2.8%	47.1%	1.8%	0%	51.7%	4.2%	1.4%	2.0%	0%	7.5%	0.9%	0.4%	1.3%	0%	2.6%	-
PHI	0.800	0.881	0.800	-	0.896	0.550	0.873	0.656	-	0.876	0.817	0.800	0.639	-	0.846	0.550	0.417	0.750	-	0.775	0.955
Lights	16	384	48	0	448	33	551	20	0	604	49	16	23	0	88	11	5	15	0	31	1171
% Lights	100%	100%	100%	0%	100%	100%	99.8%	95.2%	0%	99.7%	100%	100%	100%	0%	100%	100%	100%	100%	0%	100%	99.8%
Articulated Trucks	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	1
% Articulated Trucks	0%	0%	0%	0%	0 %	0%	0%	4.8%	0%	0.2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0.1%
Buses and Single-Unit Trucks	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1
% Buses and Single-Unit Trucks	0%	0%	0%	0%	0 %	0%	0.2%	0%	0%	0.2%	0%	0%	0%	0%	0 %	0%	0%	0%	0%	0%	0.1%

^{*}L: Left, R: Right, T: Thru, U: U-Turn

Tue Jan 29, 2019 PM Peak (5PM - 6PM) - Overall Peak Hour All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks) All Movements ID: 614890, Location: 40.196421, -83.109991



Chelsea Cousins

From:Zhuojun Jiang <zjiang@morpc.org>Sent:Tuesday, February 19, 2019 4:43 PMTo:Gina Balsamo; Hwashik Jang; Nick GillCc:Chelsea Cousins; Drew Laurent; Ying Su

Subject: RE: Growth Rate Request - Home & Steitz Mixed Use Development TIS

Hello Gina,

Please use the following linear annual growth rates for the intersection in your study.

Home Rd e/o Steitz Rd	2.50%
Steitz Rd n/o Home Rd	2.00%
Home Rd w/o Steitz Rd	2.50%
Steitz Rd s/o Home Rd	2.00%

Note: The above rates were derived based on planning level analysis by using MORPC's regional travel demand model.

If you have any questions regarding the rates, please let us know. Thanks!

Zhuojun

Zhuojun Jiang, Ph.D., P.E.

Manager, Travel Forecasting | Mid-Ohio Regional Planning Commission T: 614.233.4147

111 Liberty Street, Suite 100 | Columbus, OH 43215



From: Gina Balsamo [mailto:gbalsamo@cmtran.com]

Sent: Friday, February 1, 2019 1:18 PM

To: Hwashik Jang <hjang@morpc.org>; Nick Gill <ngill@morpc.org>; Zhuojun Jiang <zjiang@morpc.org>

Cc: Chelsea Cousins < ccousins@cmtran.com>; Drew Laurent < dlaurent@cmtran.com>

Subject: Growth Rate Request - Home & Steitz Mixed Use Development TIS

All,

We would like to request growth rates for the Home Road and Steitz Road intersection. We will be conducting a TIS for a site proposed to be a mixed-use development including apartments, office, and retail space. Please see the attached site plan and count data.

The Opening Year is 2019 and the Horizon Year is 2029 and 2039 (analysis will be completed for both Horizon Years).

The reviewing agencies are the City of Powell and Delaware County.

Please let me know if you need additional information.

Thanks,

Gina Balsamo, PE
Traffic Engineer
CARPENTER
MARTY
transportation

614.656.2429 | www.cmtran.com

Appendix C Trip Generation



Scenario - 2		
Scenario Name: AM Peak	User Group:	
Dev. phase: 1	Horizon Year: 2019	
Analyst Note:		
Warning		

VEHICLE TRIPS BEFORE REDUCTION

Land Use & Data Source	Location IV	IV	Sizo	Size Time Period	Method	Entry	Exit	Total
	LUCATION	IV	3126	Tillle Period	Rate/Equation	Split%	Split%	Iotai
590 - Library	General	1000 Ca Et CEA	25	Weekday, Peak Hour of	Best Fit (LIN)	21	8	29
Data Source: ITE-TGM 10th Edition	Urban/Suburban	1000 Sq. Ft. GFA	25	Adjacent Street Traffic,	T = 1.75(X) + -14.59	71%	29%	29
710 - General Office Building	General	1000 Sg. Ft. GFA	7	Weekday, Peak Hour of	Best Fit (LIN)	28	5	33
Data Source: ITE-TGM 10th Edition	Urban/Suburban	1000 Sq. Ft. GFA	,	Adjacent Street Traffic,	T = 0.94(X) + 26.49	86%	14%	33
710(1) - General Office Building	General	1000 Sg. Ft. GFA	7.7	Weekday, Peak Hour of	Best Fit (LIN)	29	5	24
Data Source: ITE-TGM 10th Edition	Urban/Suburban	1000 Sq. Ft. GFA	7.7	Adjacent Street Traffic,	T = 0.94(X) + 26.49	86%	14%	54
220 - Multifamily Housing (Low-Rise)	General	Dwelling Units	62	Weekday, Peak Hour of	Best Fit (LOG)	7	23	30
Data Source: ITE-TGM 10th Edition	Urban/Suburban	Dweiling Units	62	Adjacent Street Traffic,	Ln(T) =0.95Ln(X) + -0.51	23%	77%	30

VEHICLE TO PERSON TRIP CONVERSION

BASELINE SITE VEHICLE CHARACTERISTICS:

Land Use	Baseline Site V	Baseline Site Vehicle Mode Share		Baseline Site Vehicle Occupancy		le Directional Split
	Entry (%)	Exit (%)	Entry	Exit	Entry (%)	Exit (%)
590 - Library	100	100	1	1	71	29
710 - General Office Building	100	100	1	1	86	14
710(1) - General Office Building	100	100	1	1	86	14
220 - Multifamily Housing (Low-Rise)	100	100	1	1	23	77

ESTIMATED BASELINE SITE PERSON TRIPS:

Land Use	Person Trips by Vehicle		Person Trips by Other Modes		Total Baseline Site Person Trips	
	Entry	Exit	Entry	Exit	Entry	Exit
590 - Library	21	8	0	0	21	8
	29		0		29	
710 - General Office Building	28	5	0	0	28	5
710 - General Office Building	33		0		33	
710(1) - General Office Building	29	5	0	0	29	5
710(1) - General Office building	34		0		34	
220 - Multifamily Housing (Low-Rise)	7	23	0	0	7	23
	30		0		30	

NEW VEHICLE TRIPS

Land Use	New Vehicle Trips					
Lanu use	Entry	Exit	Total			
590 - Library	21	8	29			
710 - General Office Building	28	5	33			
710(1) - General Office Building	29	5	34			
220 - Multifamily Housing (Low-Rise)	7	23	30			

RESULTS

Site Totals	Entry	Exit	Total

Vehicle Trips Before Reduction	85	41	126
External Vehicle Trips	85	41	126
New Vehicle Trips	85	41	126

Scenario - 1		
Scenario Name: PM Peak	User Group:	
Dev. phase: 1	Horizon Year: 2019	
Analyst Note:		
Warning		<u> </u>

VEHICLE TRIPS BEFORE REDUCTION

Land Use & Data Source	Location	IV	Size	Size Time Period	Method	Entry	Exit	Total
	Location	10	3126	Tillie Fellou	Rate/Equation	Split%	Split%	Iotai
590 - Library	General	1000 Sq. Ft. GFA	25	Weekday, Peak Hour of	Best Fit (LIN)	104	112	216
Data Source: ITE-TGM 10th Edition	Urban/Suburban	1000 Sq. Ft. GFA	J Sq. Ft. GFA 25	Adjacent Street Traffic,	T = 9.33(X) + -17.13	48%	52%	210
710 - General Office Building	General	1000 Sq. Ft. GFA	7	Weekday, Peak Hour of	Best Fit (LOG)	1	8	0
Data Source: ITE-TGM 10th Edition	Urban/Suburban	1000 Sq. Ft. GFA	,	Adjacent Street Traffic,	Ln(T) =0.95Ln(X) + 0.36	16%	84%	9
710(1) - General Office Building	General	1000 Sq. Ft. GFA	7.7	Weekday, Peak Hour of	Best Fit (LOG)	2	8	10
Data Source: ITE-TGM 10th Edition	Urban/Suburban	1000 Sq. Ft. GFA	7.7	Adjacent Street Traffic,	Ln(T) =0.95Ln(X) + 0.36	16%	84%	10
220 - Multifamily Housing (Low-Rise)	General	Dwelling Units	(2)	Weekday, Peak Hour of	Best Fit (LOG)	24	14	38
Data Source: ITE-TGM 10th Edition	Urban/Suburban	Dwelling Units	62	Adjacent Street Traffic,	Ln(T) =0.89Ln(X) + -0.02	63%	37%	38

VEHICLE TO PERSON TRIP CONVERSION

BASELINE SITE VEHICLE CHARACTERISTICS:

Land Use	Baseline Site V	Baseline Site Vehicle Mode Share		Baseline Site Vehicle Occupancy		cle Directional Split
	Entry (%)	Exit (%)	Entry	Exit	Entry (%)	Exit (%)
590 - Library	100	100	1	1	48	52
710 - General Office Building	100	100	1	1	16	84
710(1) - General Office Building	100	100	1	1	16	84
220 - Multifamily Housing (Low-Rise)	100	100	1	1	63	37

ESTIMATED BASELINE SITE PERSON TRIPS:

Land Use	Person Trips by Vehicle		Person Trips by Other Modes		Total Baseline Site Person Trips	
Land OSC	Entry	Exit	Entry	Exit	Entry	Exit
590 - Library	104	112	0	0	104	112
	216		0		216	
710 - General Office Building	1	8	0	0	1	8
710 - General Office Building	9		0		9	
710(1) - General Office Building	2	8	0	0	2	8
7 10(1) - General Office Building	10		0		10	
220 - Multifamily Housing (Low-Rise)	24	14	0	0	24	14
	38		0		38	

NEW VEHICLE TRIPS

Land Use	New Vehicle Trips				
Lanu use	Entry	Exit	Total		
590 - Library	104	112	216		
710 - General Office Building	1	8	9		
710(1) - General Office Building	2	8	10		
220 - Multifamily Housing (Low-Rise)	24	14	38		

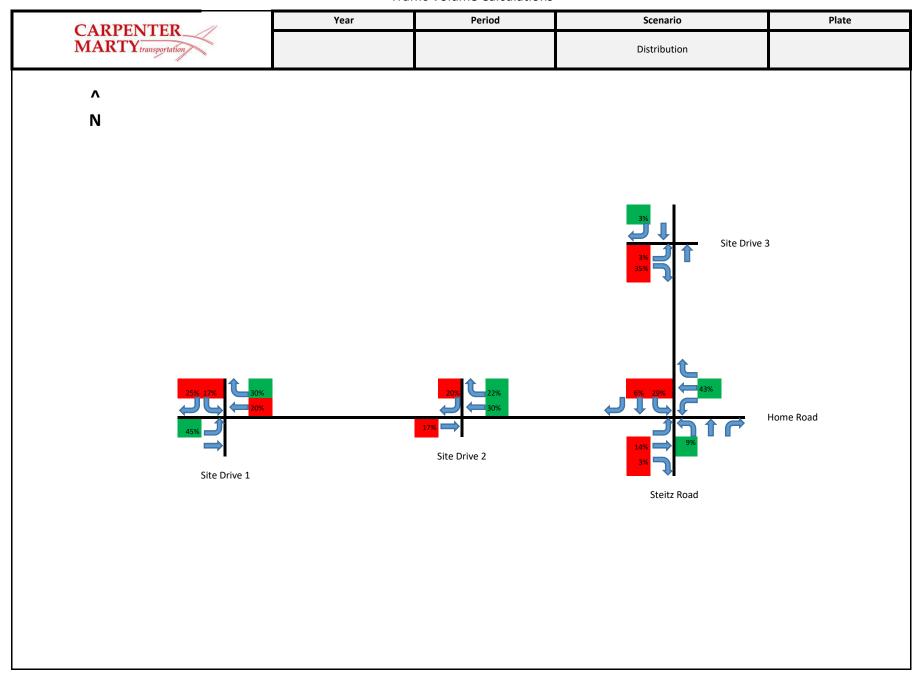
RESULTS

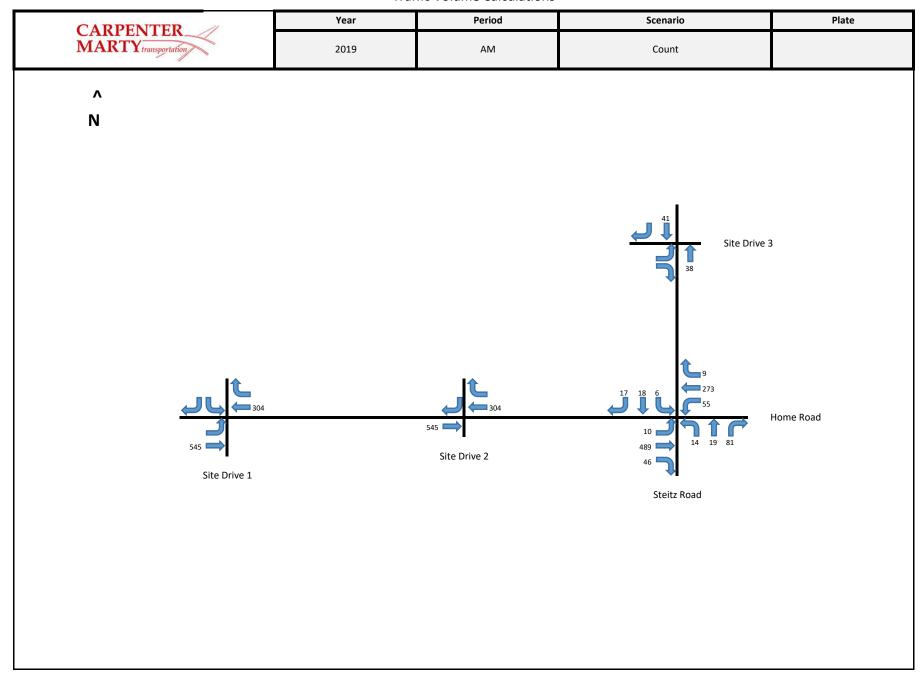
Site Totals	Entry	Exit	Total

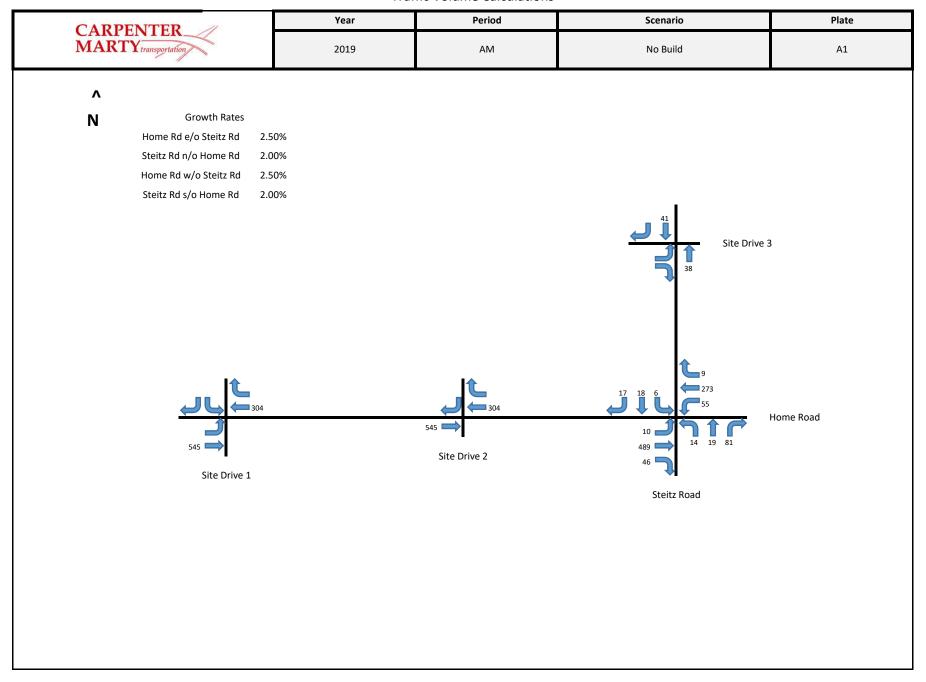
Vehicle Trips Before Reduction	131	142	273
External Vehicle Trips	131	142	273
New Vehicle Trips	131	142	273

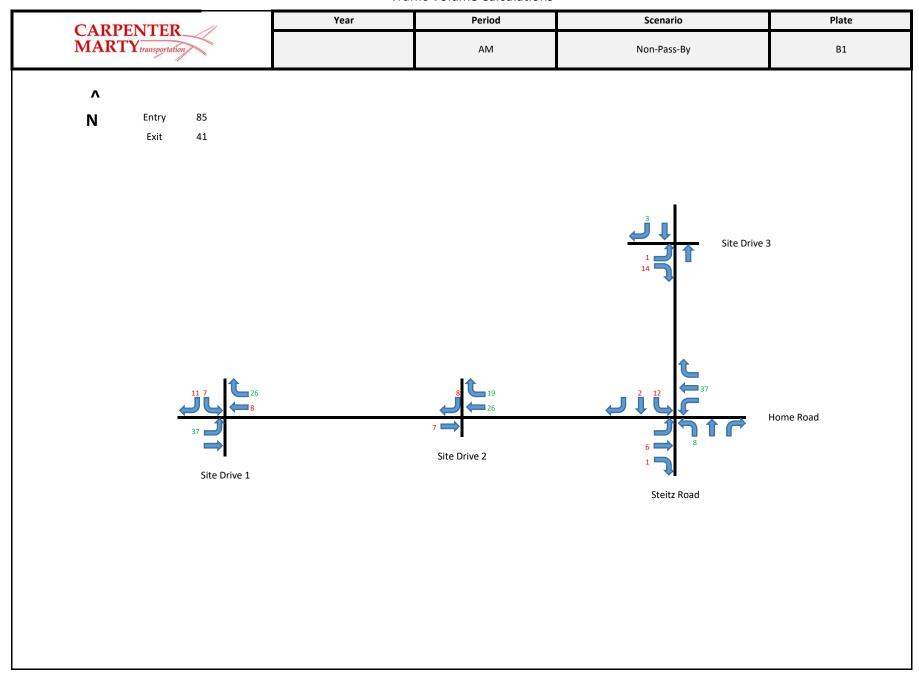
Appendix D Volume Development

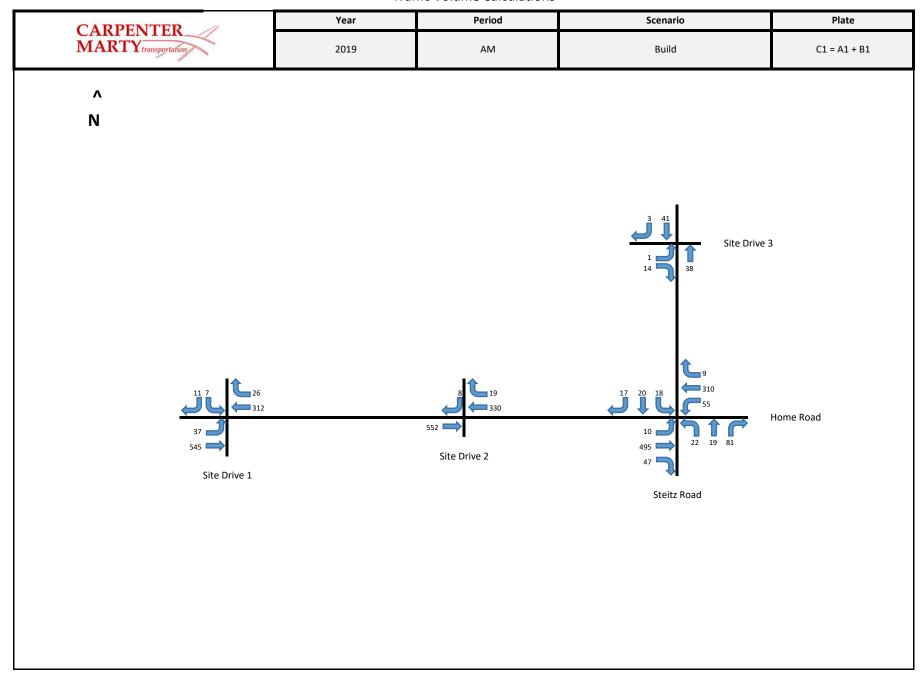


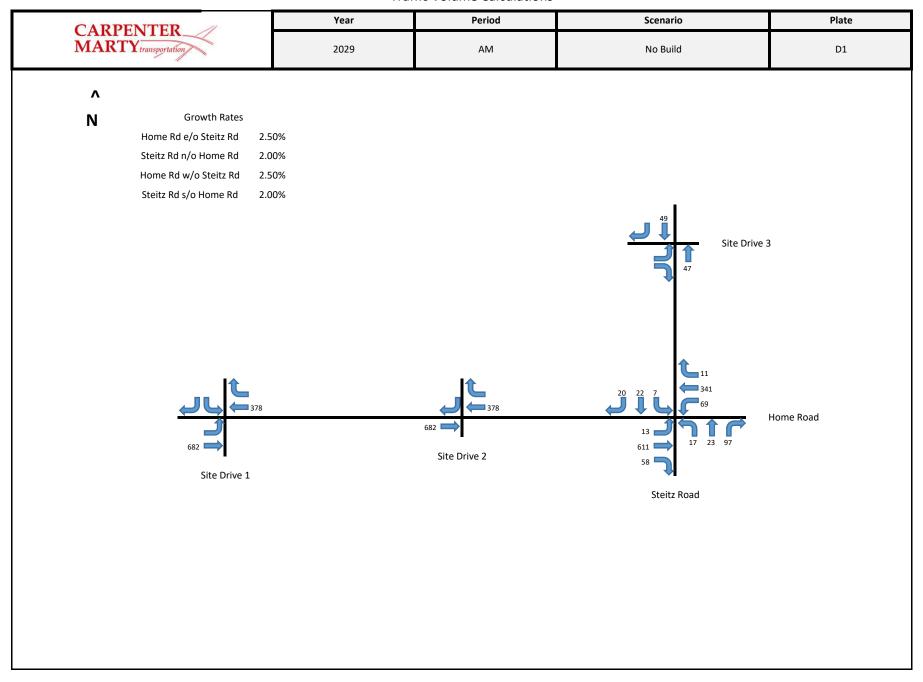


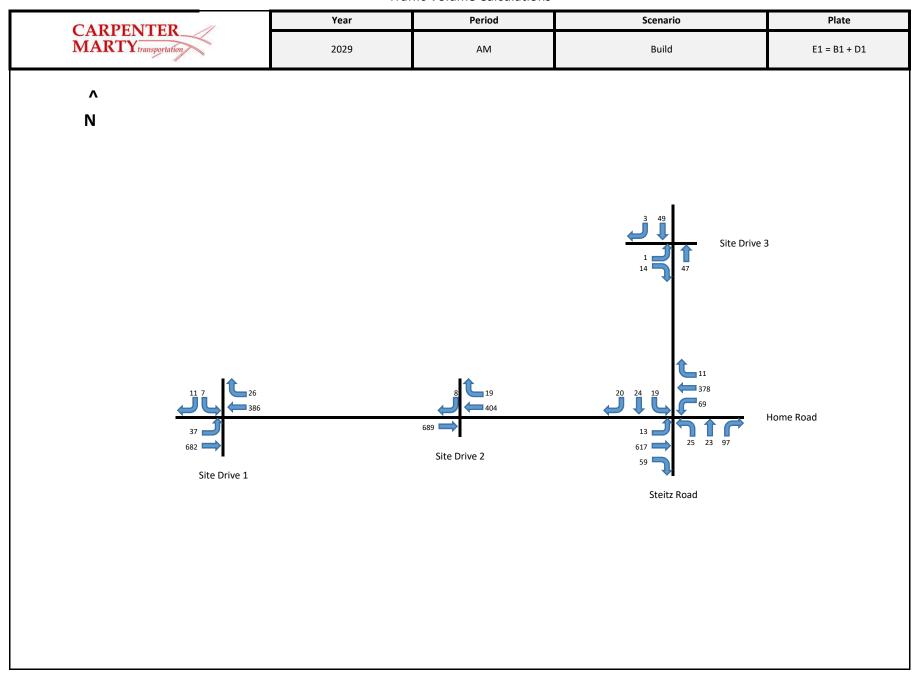


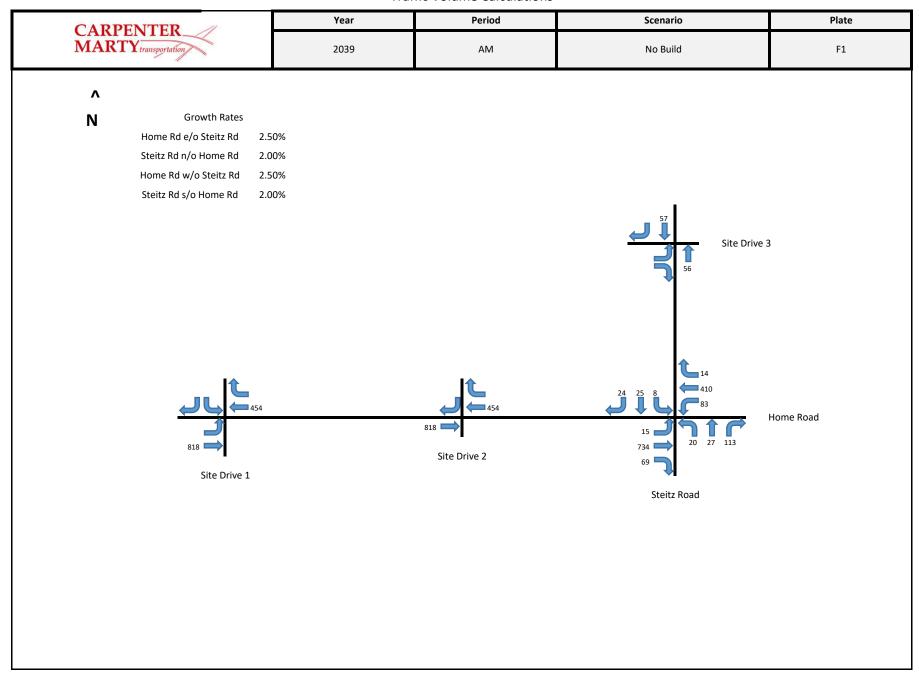


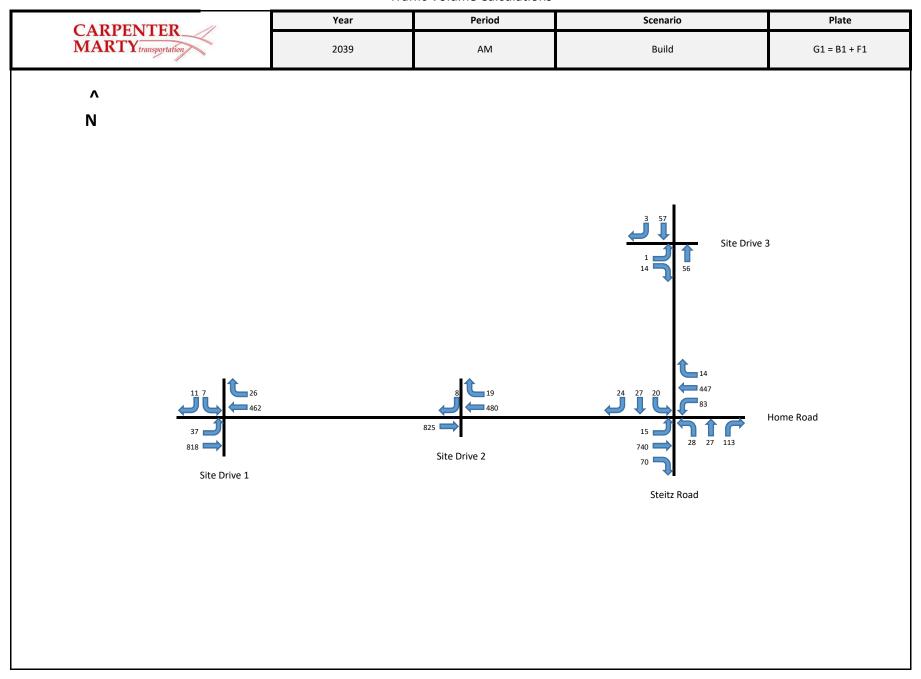


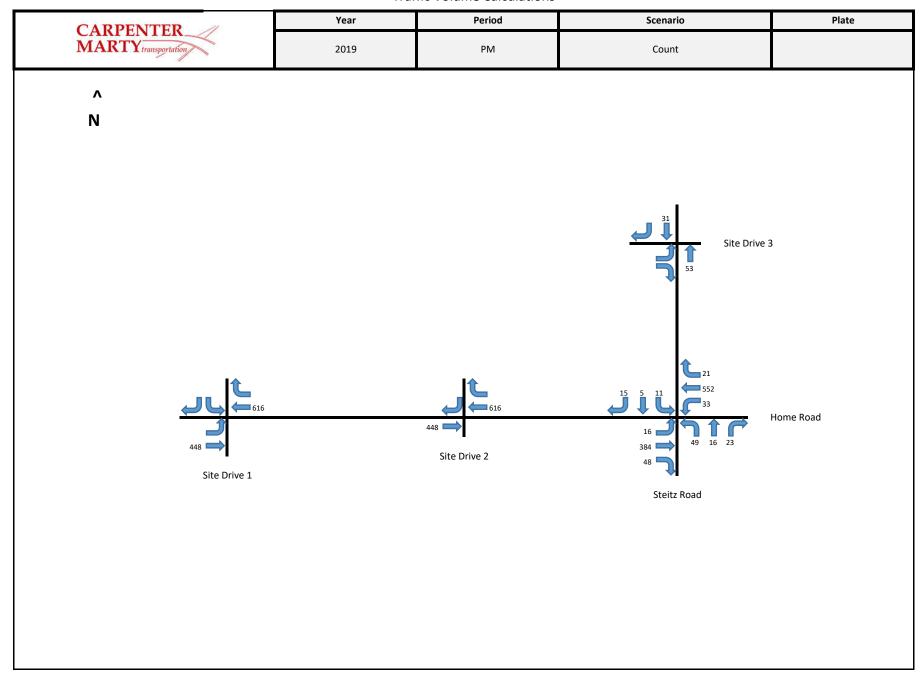


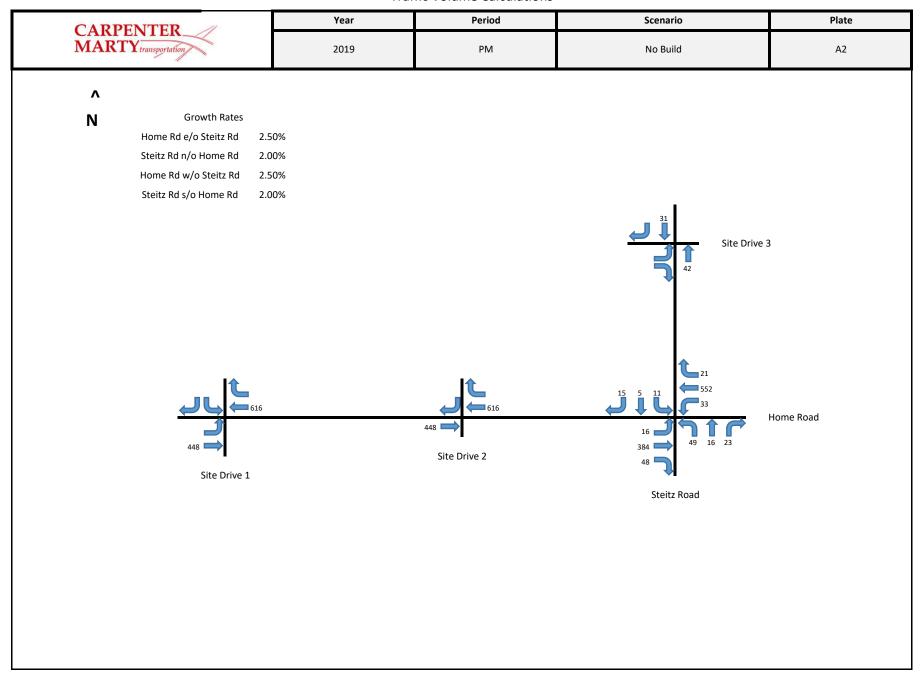


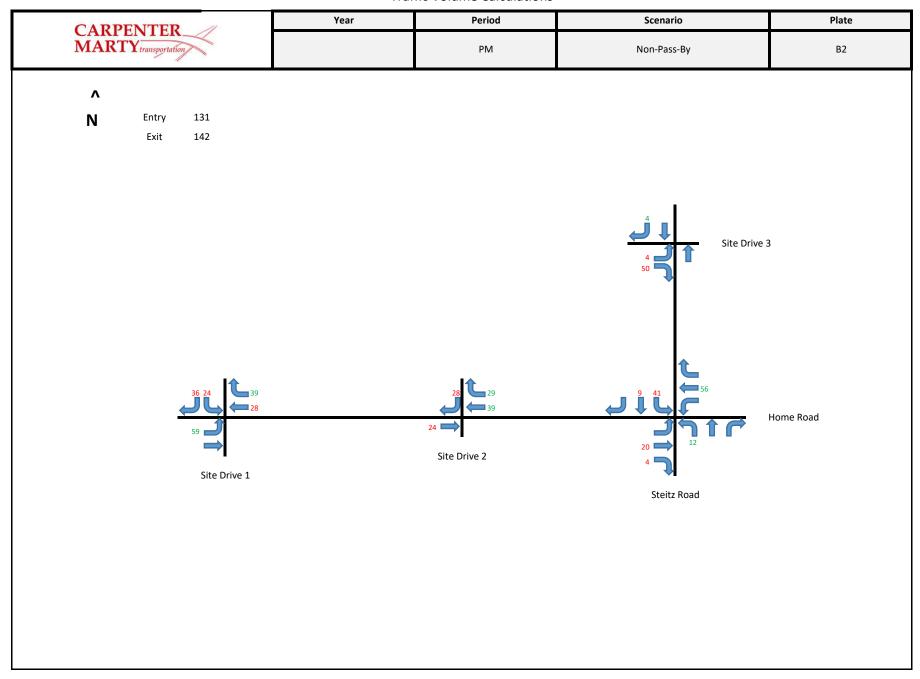


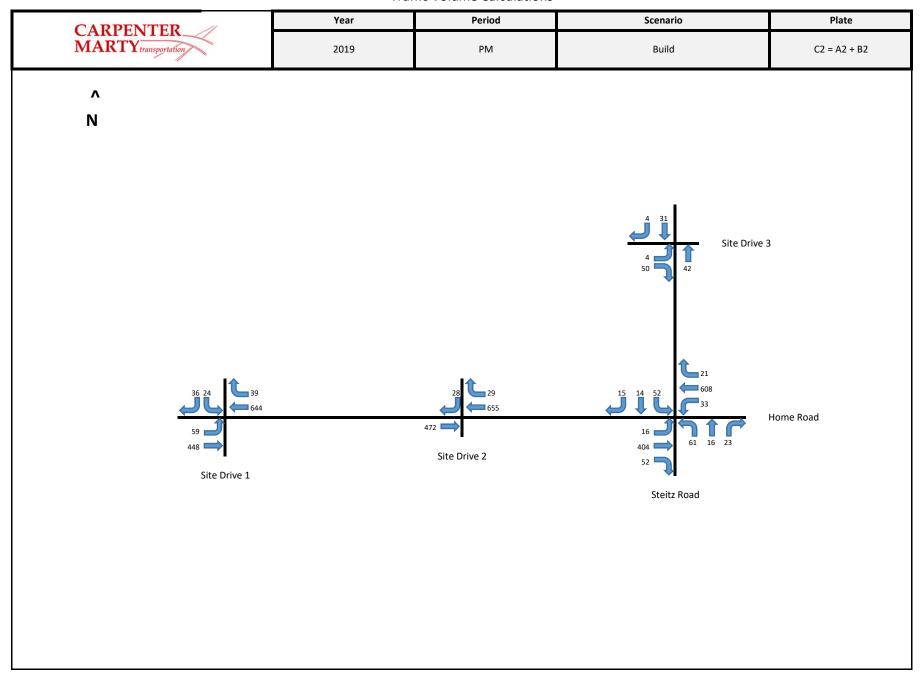


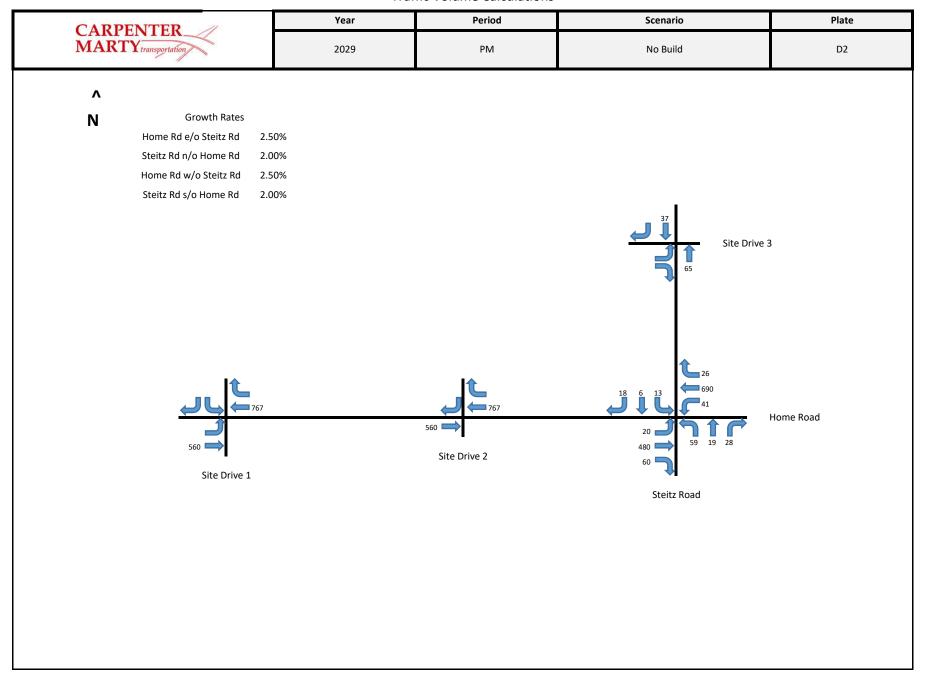


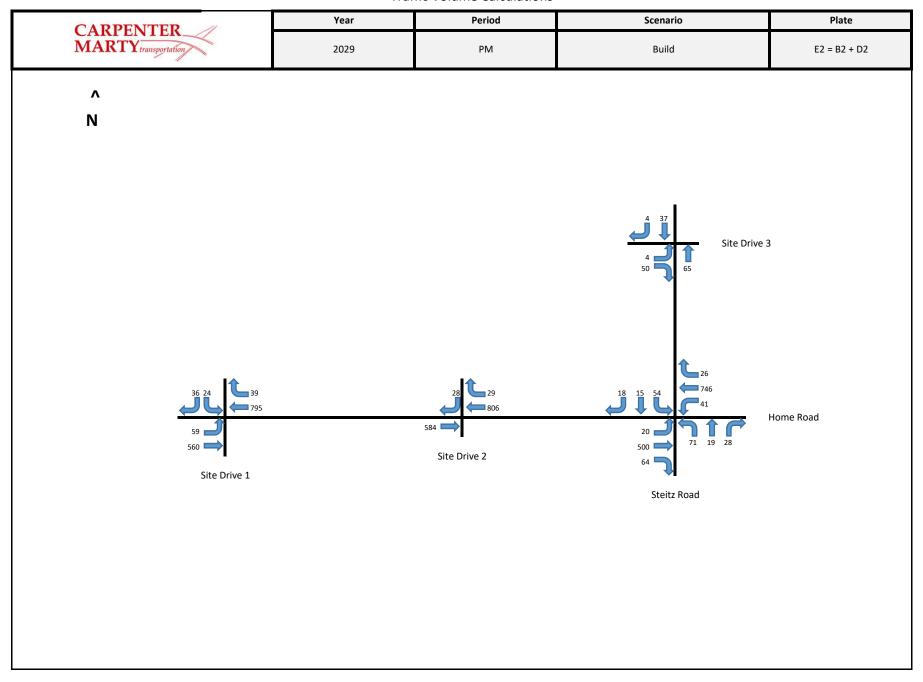


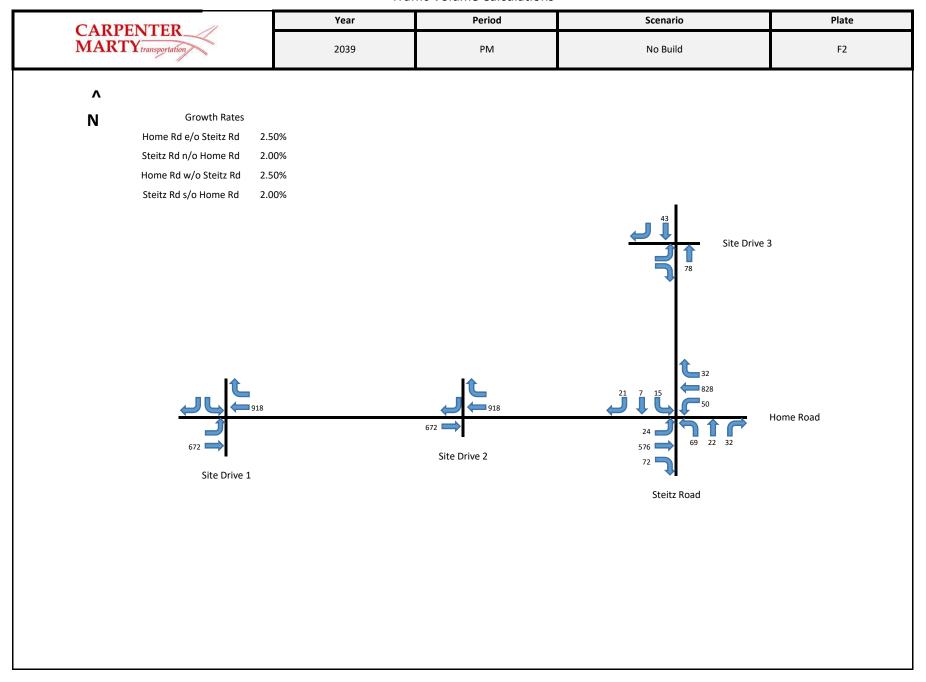


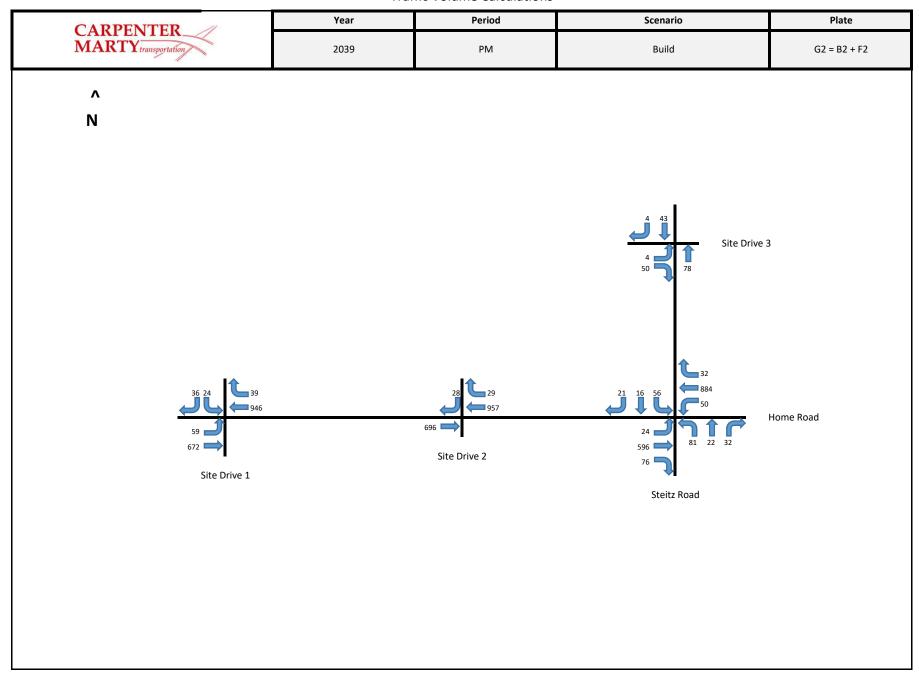










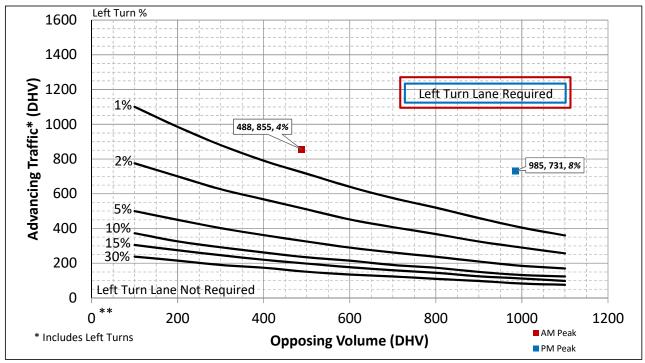


Appendix E Turn Lane Warrant Analysis





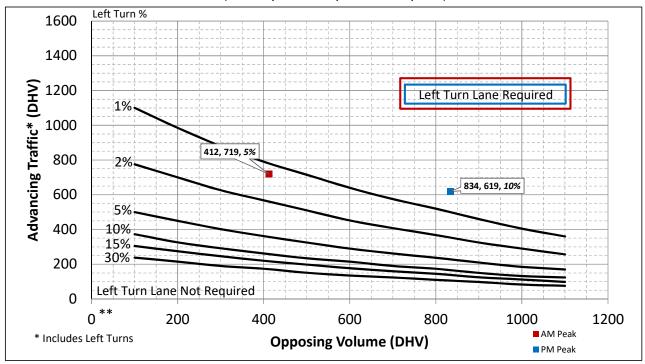
(> 40 mph or 70 kph Posted Speed)



	Design Speed	60	mph	
	Traffic Control	Unsignalized		
AM Peak	Cycle Length	Unsignalized		
	Cycles Per Hour	60	Assume 60	
	Turn Lane Volume	37	VPH	
	Advancing Traffic	855	VPH	
ď	Opposing Volume	488	VPH	
_	Left Turn Percentage	4%		
\geq	Location Type	Through Road		
7	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
	Offset Width	12		includes 50 ft diverging
	Approach Taper	720		taper
	Design Speed	60	mph	
	Traffic Control	Unsignalized		
	Cycle Length	Unsignalized		
	Cycles Per Hour	60	Assume 60	
Ť	Turn Lane Volume	59	VPH	
(1)	Advancing Traffic	731	VPH	
0	Opposing Volume	985	VPH	
	Left Turn Percentage	8%		
PM Peak	Location Type	Through Road		
	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345	· ·	* Turn Lane Length
	Offset Width	12		includes 50 ft diverging
	Approach Taper	720		taper
Is Left	Turn Warrant Met	Yes	See Above	



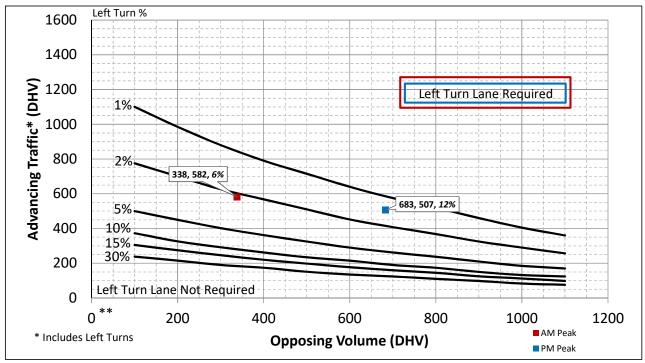
(> 40 mph or 70 kph Posted Speed)



	Design Speed	60	mph	
AM Peak	Traffic Control	Unsignalized		
	Cycle Length	Unsignalized		
	Cycles Per Hour	60	Assume 60	
	Turn Lane Volume	37	VPH	
	Advancing Traffic	719	VPH	
	Opposing Volume	412	VPH	
_	Left Turn Percentage	5%		
>	Location Type	Through Road		
\overline{A}	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
	Offset Width	12		includes 50 ft diverging
	Approach Taper	720		taper
	Design Speed	60	mph	
	Traffic Control	Unsignalized		
	Cycle Length	Unsignalized		
	Cycles Per Hour	60	Assume 60	
*	Turn Lane Volume	59	VPH	
(1)	Advancing Traffic	619	VPH	
~	Opposing Volume	834	VPH	
PM Peak	Left Turn Percentage	10%		
	Location Type	Through Road		
	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
	Offset Width	12		includes 50 ft diverging
	Approach Taper	720		taper
Is Left	Turn Warrant Met	Yes	See Above	



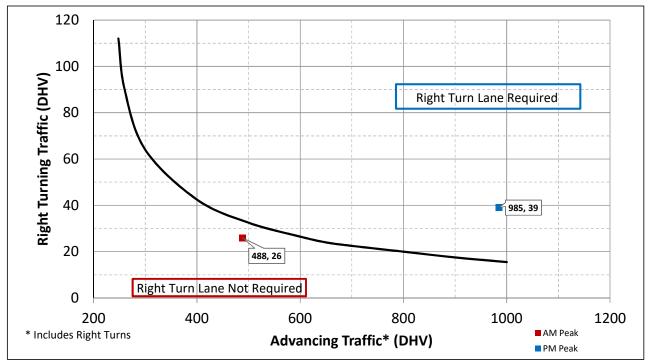
(> 40 mph or 70 kph Posted Speed)



	Design Speed	60	mph	7
	Traffic Control	Unsignalized		
AM Peak	Cycle Length	Unsignalized		
	Cycles Per Hour	60	Assume 60	7
	Turn Lane Volume	37	VPH	1
	Advancing Traffic	582	VPH	1
ď	Opposing Volume	338	VPH	1
_	Left Turn Percentage	6%		
\geq	Location Type	Through Road		
7	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
	Offset Width	12		includes 50 ft diverging
	Approach Taper	720		taper
	Design Speed	60	mph	
	Traffic Control	Unsignalized		
	Cycle Length	Unsignalized		
	Cycles Per Hour	60	Assume 60	
Ť	Turn Lane Volume	59	VPH	
(0)	Advancing Traffic	507	VPH]
	Opposing Volume	683	VPH	
	Left Turn Percentage	12%		
PM Peak	Location Type	Through Road		
	Condition	B or C		
	Vehicles/Cycle	1		
	Turn Lane Length	See Column to Right	345	* Turn Lane Length
	Offset Width	12		includes 50 ft diverging
	Approach Taper	720		taper
Is Left	Turn Warrant Met	Yes	See Above	



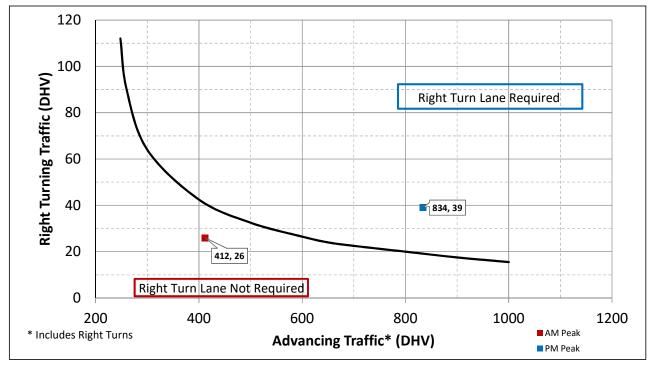
(> 40 mph or 70 kph Posted Speed)



	Design Speed	60	mph	7
	Traffic Control	Unsignalized		
~	Cycle Length	Unsignalized		
AM Peak	Cycles Per Hour	60	Assume 60	
a	Turn Lane Volume	26	VPH	
<u> </u>	Advancing Traffic	488	VPH	
_	Right Turn Percentage	5%		
	Location Type	Through Road		
\triangleleft	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
	Design Speed	60	mph	includes 50 ft diverging
	Traffic Control	Unsignalized		taper
Peak	Cycle Length	Unsignalized		
O	Cycles Per Hour	60	Assume 60	
\mathbf{o}	Turn Lane Volume	39	VPH	
	Advancing Traffic	985	VPH	
	Right Turn Percentage	4%		
PM	Location Type	Through Road		
	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
Is Right	t Turn Warrant Met	Yes	See Above	includes 50 ft diverging taper



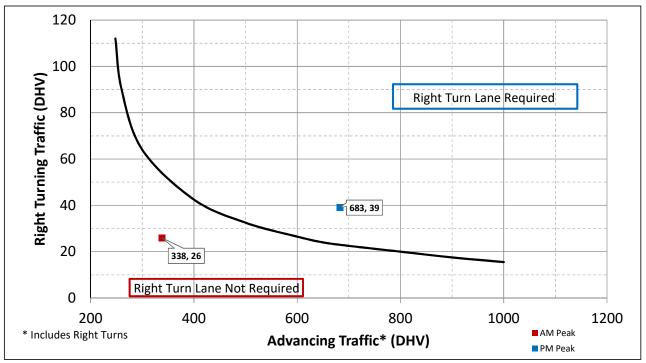
(> 40 mph or 70 kph Posted Speed)



	Design Speed	60	mph	7
	Traffic Control	Unsignalized		
~	Cycle Length	Unsignalized		
AM Peak	Cycles Per Hour	60	Assume 60	
a	Turn Lane Volume	26	VPH	
<u> </u>	Advancing Traffic	412	VPH	
	Right Turn Percentage	6%		
	Location Type	Through Road		
\triangleleft	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
	Design Speed	60	mph	includes 50 ft diverging
	Traffic Control	Unsignalized		taper
	Cycle Length	Unsignalized		
Peak	Cycles Per Hour	60	Assume 60	
\mathbf{o}	Turn Lane Volume	39	VPH	
	Advancing Traffic	834	VPH	
	Right Turn Percentage	5%		
PM	Location Type	Through Road		
	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
Is Right	t Turn Warrant Met	Yes	See Above	includes 50 ft diverging taper



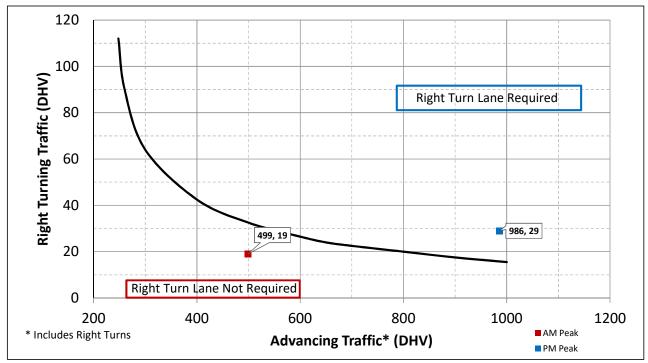
(> 40 mph or 70 kph Posted Speed)



	Design Speed	60	mph	
	Traffic Control	Unsignalized		
~	Cycle Length	Unsignalized		
AM Peak	Cycles Per Hour	60	Assume 60	
e	Turn Lane Volume	26	VPH	
<u> </u>	Advancing Traffic	338	VPH	
-	Right Turn Percentage	8%		
	Location Type	Through Road		
⋖	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345	-	* Turn Lane Length
	Design Speed	60	mph	includes 50 ft diverging
	Traffic Control	Unsignalized		taper
	Cycle Length	Unsignalized		
O	Cycles Per Hour	60	Assume 60	
\mathbf{o}	Turn Lane Volume	39	VPH	
	Advancing Traffic	683	VPH	
_	Right Turn Percentage	6%		
PM Pea	Location Type	Through Road		
	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
Is Right	t Turn Warrant Met	Yes	See Above	includes 50 ft diverging taper



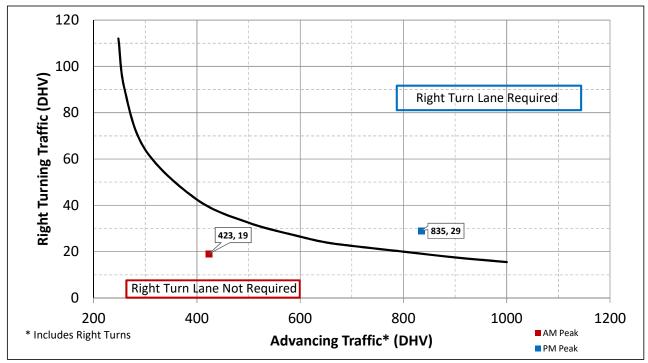
(> 40 mph or 70 kph Posted Speed)



	Design Speed	60	mph	7
	Traffic Control	Unsignalized		
~	Cycle Length	Unsignalized		
AM Peak	Cycles Per Hour	60	Assume 60	
e	Turn Lane Volume	19	VPH	
	Advancing Traffic	499	VPH	
5	Right Turn Percentage	4%		
	Location Type	Through Road		
⋖	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
	Design Speed	60	mph	includes 50 ft diverging
	Traffic Control	Unsignalized		taper
	Cycle Length	Unsignalized		
a	Cycles Per Hour	60	Assume 60	
\Box	Turn Lane Volume	29	VPH	
	Advancing Traffic	986	VPH	
	Right Turn Percentage	3%		
PM Pea	Location Type	Through Road		
	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
Is Right	t Turn Warrant Met	Yes	See Above	includes 50 ft diverging taper



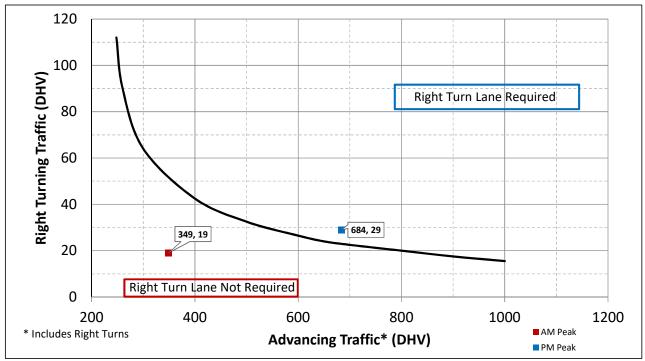
(> 40 mph or 70 kph Posted Speed)



	Design Speed	60	mph	
	Traffic Control	Unsignalized		
~	Cycle Length	Unsignalized		
AM Peak	Cycles Per Hour	60	Assume 60	
a	Turn Lane Volume	19	VPH	
<u> </u>	Advancing Traffic	423	VPH	
-	Right Turn Percentage	4%		
	Location Type	Through Road		
⋖	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345	-	* Turn Lane Length
	Design Speed	60	mph	includes 50 ft diverging
	Traffic Control	Unsignalized		taper
	Cycle Length	Unsignalized		
O	Cycles Per Hour	60	Assume 60	
\mathbf{o}	Turn Lane Volume	29	VPH	
	Advancing Traffic	835	VPH	
_	Right Turn Percentage	3%		
PM Peak	Location Type	Through Road		
	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
Is Right	t Turn Warrant Met	Yes	See Above	includes 50 ft diverging taper



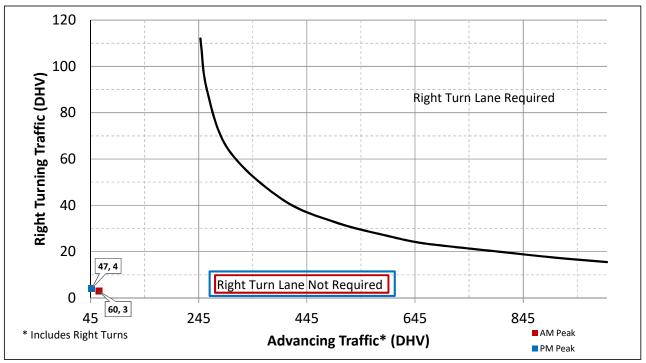
(> 40 mph or 70 kph Posted Speed)



	Design Speed	60	mph	
	Traffic Control	Unsignalized		
~	Cycle Length	Unsignalized		
AM Peak	Cycles Per Hour	60	Assume 60	
Φ	Turn Lane Volume	19	VPH	
<u> </u>	Advancing Traffic	349	VPH	
	Right Turn Percentage	5%		
	Location Type	Through Road		
\triangleleft	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
	Design Speed	60	mph	includes 50 ft diverging
	Traffic Control	Unsignalized		taper
	Cycle Length	Unsignalized		
O	Cycles Per Hour	60	Assume 60	
O	Turn Lane Volume	29	VPH	
<u> </u>	Advancing Traffic	684	VPH	
	Right Turn Percentage	4%		
PM Pea	Location Type	Through Road		
	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
Is Right	t Turn Warrant Met	Yes	See Above	includes 50 ft diverging taper



(> 40 mph or 70 kph Posted Speed)



	Design Speed	60	mph	7
	Traffic Control	Unsignalized		
AM Peak	Cycle Length	Unsignalized		
	Cycles Per Hour	60	Assume 60	
a)	Turn Lane Volume	3	VPH	
<u> </u>	Advancing Traffic	60	VPH	
_	Right Turn Percentage	5%		
	Location Type	Through Road		
\triangleleft	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
	Design Speed	60	mph	includes 50 ft diverging
	Traffic Control	Unsignalized		taper
	Cycle Length	Unsignalized		
O	Cycles Per Hour	60	Assume 60	
\mathbf{o}	Turn Lane Volume	4	VPH	
<u> </u>	Advancing Traffic	47	VPH	
	Right Turn Percentage	9%		
PM Peak	Location Type	Through Road		
	Condition	В		
	Vehicles/Cycle	1		
	Turn Lane Length	345		* Turn Lane Length
le Dieh	t Turn Warrant Mot	No	No Right Turn Lane	includes 50 ft diverging
is Kign	t Turn Warrant Met	IVO	Required	taper

Appendix F Capacity Analysis



Ped-Bike Adj(A_pbT) 1.00		•	-	7	1		•	4	Ť	~	1	ļ	4
Traffic Volume (veh/h) 10 489 46 55 273 9 14 19 81 6 18 17 Puture Volume (veh/h) 10 489 46 55 273 9 14 19 81 6 18 17 Number 7 4 14 3 3 8 18 5 2 12 1 6 16 Initial Q (Qb), veh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Future Volume (veh/h) 10 489 46 55 273 9 14 19 81 6 18 17 Number	Lane Configurations	7	↑	7	7	↑	7	7	1		7	1	
Number 7 4 14 3 8 18 5 2 12 12 1 6 16 16 Initial O (Ob), veh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			489	46	55	273	9		19		6	18	
Initial Q (Qb), weh	Future Volume (veh/h)	10	489	46	55	273	9	14		81	6	18	
Ped-Bike Adj(A_pbT)	Number	7	4	14	3	8	18	5	2	12	1	6	16
Parking Bus, Acj	Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	
Adj Sat Flow, vehi/hin 1863 1863 1863 1863 1863 1863 1863 1863	Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Adj Flow Rate, vehi/h Adj No of Lanes 1	Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Adj No. of Lanes	Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1900
Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	Adj Flow Rate, veh/h	11	532	50	60	297	10	15	21	88	7	20	18
Percent Heavy Veh, % 2 2 2 2 2 2 2 2 2	Adj No. of Lanes	1	1	1	1	1	1	1	1	0	1	1	0
Cap, veh/h	Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Arrive On Green	Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Arrive On Green	•	444	731	621	272	731	621	707	133	556	636	382	
Grp Voltume(v), vehi/h 11 532 50 60 297 10 15 0 109 7 0 38 Grp Sat Flow(s), vehi/h/ln 1068 1863 1563 829 1863 1583 1364 0 1630 1279 0 1719 Q Serve(g_s), s 0.4 11.8 1.0 3.2 5.6 0.2 0.3 0.0 2.0 0.2 0.0 0.6 Cycle Q Clear(g_c), s 6.0 11.8 1.0 15.0 5.6 0.2 1.0 0.0 2.0 2.2 0.0 0.6 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 0.81 1.00 0.47 Lane Grp Cap(c), vehi/h 444 731 621 272 731 621 707 0 688 636 0 726 V/C Ratio(X) 0.02 0.73 0.08 0.22 0.41 0.02 0.02 0.00 0.16 0.01 0.00 0.05 Avail Cap(c_a), vehi/h 1356 2321 1973 980 2321 1973 707 0 688 636 0 726 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0		0.39	0.39	0.39	0.39	0.39	0.39	0.42	0.42	0.42	0.42	0.42	0.42
Grp Volume(v), veh/h 11 532 50 60 297 10 15 0 109 7 0 38 Grp Sat Flow(s), veh/h/ln 1068 1863 1583 829 1863 1583 1364 0 1630 1279 0 1719 Q Serve(g_s), s 0.4 11.8 1.0 3.2 5.6 0.2 0.3 0.0 2.0 0.2 0.0 0.6 Cycle Q Clear(g_c), s 6.0 11.8 1.0 15.0 5.6 0.2 1.0 0.0 2.0 2.2 0.0 0.6 Prop In Lane 1.00 1.00 1.00 1.00 1.00 0.02 0.02 0.0 0.4 Lane Grp Cap(c), veh/h 444 731 621 272 731 621 707 0 688 636 0 726 W/C Ratio(X) 0.02 0.73 0.08 0.22 1.01 0.02 0.02 0.01 0.01 0.00	Sat Flow, veh/h	1068	1863	1583	829	1863	1583	1364	314	1316	1279	905	814
Grp Sat Flow(s), veh/h/ln		11		50	60	297	10	15		109	7		
Q Serve(g_s), s	1 7												
Cycle Q Člear(g_c), s 6.0 11.8 1.0 15.0 5.6 0.2 1.0 0.0 2.0 2.2 0.0 0.6 Prop In Lane 1.00 1.00 1.00 1.00 1.00 0.0 0.81 1.00 0.47 Lane Grp Cap(c), veh/h 444 731 621 272 731 621 707 0 688 636 0 726 VCR Ratio(X) 0.02 0.73 0.08 0.22 0.41 0.02 0.00 0.16 0.01 0.00 0.05 Avail Cap(c_a), veh/h 1356 2321 1973 980 2321 1973 707 0 688 636 0 726 HCM Platon Ratio 1.00													
Prop In Lane													
Lane Grp Cap(c), veh/h						0.0			0.0			0.0	
V/C Ratio(X) 0.02 0.73 0.08 0.22 0.41 0.02 0.02 0.01 0.01 0.00 0.05 Avail Cap(c_a), veh/h 1356 2321 1973 980 2321 1973 707 0 688 636 0 726 HCM Platoon Ratio 1.00			731			731			0			0	
Avail Cap(c_a), veh/h													
HCM Platoon Ratio													
Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 0.00 1.00 1.00 0.00 1.00 1.00 0.00 1.00 1.00 0.00 1.00 1.00 0.00 1.00 1.00 0.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 8.3 Incr Delay (d2), s/veh 0.0													
Uniform Delay (d), s/veh 12.8 12.5 9.3 18.9 10.7 9.0 8.6 0.0 8.7 9.4 0.0 8.3 Incr Delay (d2), s/veh 0.0 1.4 0.1 0.4 0.4 0.0 0.1 0.0 0.5 0.0 0.0 0.1 Initial Q Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.													
Incr Delay (d2), s/veh													
Initial Q Delay(d3),s/veh 0.0 <td></td>													
%ile BackOfQ(50%), veh/ln 0.1 6.2 0.4 0.8 2.9 0.1 0.1 0.0 1.0 0.1 0.0 0.3 LnGrp Delay(d), s/veh 12.8 14.0 9.3 19.3 11.0 9.0 8.6 0.0 9.2 9.4 0.0 8.4 LnGrp LOS B B A B B A </td <td></td>													
LnGrp Delay(d),s/veh 12.8 14.0 9.3 19.3 11.0 9.0 8.6 0.0 9.2 9.4 0.0 8.4 LnGrp LOS B B B A B B A													
LnGrp LOS B B A B B A A A A A Approach Vol, veh/h 593 367 124 45 Approach Delay, s/veh 13.5 12.3 9.1 8.6 Approach LOS B B A A A Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 4 6 8 8 Phs Duration (G+Y+Rc), s 25.0 23.5 25.0 23.5 Change Period (Y+Rc), s 4.5 4.5 4.5 4.5 Max Green Setting (Gmax), s 20.5 60.5 20.5 60.5 Max Q Clear Time (g_c+I1), s 4.0 13.8 4.2 17.0 Green Ext Time (p_c), s 0.5 3.6 0.1 2.0 Intersection Summary HCM 2010 Ctrl Delay 12.5													
Approach Vol, veh/h 593 367 124 45 Approach Delay, s/veh 13.5 12.3 9.1 8.6 Approach LOS B B A A A Seproach LOS B B B A A A Seproach LOS B B B A A A A Seproach LOS B B A <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>0.0</td> <td></td> <td></td> <td>0.0</td> <td></td>									0.0			0.0	
Approach Delay, s/veh 13.5 12.3 9.1 8.6 Approach LOS B B A A Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 4 6 8 8 Phs Duration (G+Y+Rc), s 25.0 23.5 25.0 23.5 Change Period (Y+Rc), s 4.5 4.5 4.5 4.5 Max Green Setting (Gmax), s 20.5 60.5 20.5 60.5 Max Q Clear Time (g_c+I1), s 4.0 13.8 4.2 17.0 Green Ext Time (p_c), s 0.5 3.6 0.1 2.0 Intersection Summary HCM 2010 Ctrl Delay 12.5									12/			15	
Approach LOS B B A A A Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 4 6 8 Phs Duration (G+Y+Rc), s 25.0 23.5 Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 20.5 60.5 Max Q Clear Time (g_c+I1), s 4.0 13.8 4.2 17.0 Green Ext Time (p_c), s 0.5 3.6 0.1 2.0 Intersection Summary HCM 2010 Ctrl Delay 12.5	• •												
Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 4 6 8 Phs Duration (G+Y+Rc), s 25.0 23.5 25.0 23.5 Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 20.5 60.5 20.5 60.5 Max Q Clear Time (g_c+I1), s 4.0 13.8 4.2 17.0 Green Ext Time (p_c), s 0.5 3.6 0.1 2.0 Intersection Summary HCM 2010 Ctrl Delay 12.5													
Assigned Phs 2 4 6 8 Phs Duration (G+Y+Rc), s 25.0 23.5 25.0 23.5 Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 20.5 60.5 20.5 60.5 Max Q Clear Time (g_c+I1), s 4.0 13.8 4.2 17.0 Green Ext Time (p_c), s 0.5 3.6 0.1 2.0 Intersection Summary HCM 2010 Ctrl Delay 12.5												А	
Phs Duration (G+Y+Rc), s 25.0 23.5 25.0 23.5 Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 20.5 60.5 20.5 60.5 Max Q Clear Time (g_c+I1), s 4.0 13.8 4.2 17.0 Green Ext Time (p_c), s 0.5 3.6 0.1 2.0 Intersection Summary HCM 2010 Ctrl Delay 12.5		1		3		5		7					
Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 20.5 60.5 20.5 60.5 Max Q Clear Time (g_c+I1), s 4.0 13.8 4.2 17.0 Green Ext Time (p_c), s 0.5 3.6 0.1 2.0 Intersection Summary HCM 2010 Ctrl Delay 12.5													
Max Green Setting (Gmax), s 20.5 60.5 20.5 60.5 Max Q Clear Time (g_c+l1), s 4.0 13.8 4.2 17.0 Green Ext Time (p_c), s 0.5 3.6 0.1 2.0 Intersection Summary HCM 2010 Ctrl Delay 12.5													
Max Q Clear Time (g_c+l1), s 4.0 13.8 4.2 17.0 Green Ext Time (p_c), s 0.5 3.6 0.1 2.0 Intersection Summary HCM 2010 Ctrl Delay 12.5	,												
Green Ext Time (p_c), s 0.5 3.6 0.1 2.0 Intersection Summary HCM 2010 Ctrl Delay 12.5													
Intersection Summary HCM 2010 Ctrl Delay 12.5													
HCM 2010 Ctrl Delay 12.5	Green Ext Time (p_c), s		0.5		3.6		0.1		2.0				
/	Intersection Summary												
HCM 2010 LOS B	HCM 2010 Ctrl Delay			12.5									
	HCM 2010 LOS			В									

OY AM No Build Synchro 10 Report

<\↑	4	\downarrow \triangleright	*
2	4	6	8
NBTL	EBTL	SBTL	WBTL
Max	None	Max	None
25	65	25	65
27.8%	72.2%	27.8%	72.2%
22.5	22.5	22.5	22.5
3.5	3.5	3.5	3.5
1	1	1	1
5	5	5	5
3	3	3	3
3	3	3	3
0	0	0	0
0	0	0	0
7	7	7	7
11	11	11	11
Yes	Yes	Yes	Yes
Yes	Yes	Yes	Yes
0	25	0	25
25	0	25	0
20.5	85.5	20.5	85.5
9.5	74.5	9.5	74.5
0	25	0	25
20.5	85.5	20.5	85.5
9.5	74.5	9.5	74.5
		90	
Actuate	ed-Uncoo	rdinated	
		50	
tz Road &	Home Ro	oad	
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	200		
	05 S		
	▼ Ø8		
	Max 25 27.8% 22.5 3.5 1 5 3 3 0 0 7 11 Yes Yes 0 25 20.5 9.5 0 20.5 9.5	Max None 25 65 27.8% 72.2% 22.5 22.5 3.5 3.5 1 1 5 5 3 3 0 0 0 0 0 7 7 11 11 Yes Yes Yes Yes Yes Yes 0 25 25 0 20.5 85.5 9.5 74.5 0 25 20.5 85.5 9.5 74.5 Actuated-Uncoor tz Road & Home Ro	Max None Max 25 65 25 27.8% 72.2% 27.8% 22.5 22.5 22.5 3.5 3.5 3.5 1 1 1 5 5 5 3 3 3 0 0 0 0 0 0 7 7 7 11 11 11 Yes Yes Yes Yes Yes Yes 0 25 0 25 0 25 20.5 85.5 20.5 9.5 74.5 9.5 4 90 Actuated-Uncoordinated 50 4 50

OY AM No Build Synchro 10 Report

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑	7	7	^	7	7	1		7	1	
Traffic Volume (veh/h)	10	495	47	55	310	9	22	19	81	18	20	17
Future Volume (veh/h)	10	495	47	55	310	9	22	19	81	18	20	17
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	11	538	51	60	337	10	24	21	88	20	22	18
Adj No. of Lanes	1	1	1	1	1	1	1	1	0	1	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	396	730	620	250	730	620	719	140	586	649	422	345
Arrive On Green	0.39	0.39	0.39	0.39	0.39	0.39	0.44	0.44	0.44	0.44	0.44	0.44
Sat Flow, veh/h	1030	1863	1583	824	1863	1583	1362	314	1316	1279	949	777
Grp Volume(v), veh/h	11	538	51	60	337	10	24	0	109	20	0	40
Grp Sat Flow(s),veh/h/ln	1030	1863	1583	824	1863	1583	1362	0	1630	1279	0	1726
Q Serve(g_s), s	0.4	13.6	1.1	3.7	7.4	0.2	0.6	0.0	2.2	0.5	0.0	0.7
Cycle Q Clear(g_c), s	7.8	13.6	1.1	17.3	7.4	0.2	1.3	0.0	2.2	2.7	0.0	0.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.81	1.00		0.45
Lane Grp Cap(c), veh/h	396	730	620	250	730	620	719	0	725	649	0	768
V/C Ratio(X)	0.03	0.74	0.08	0.24	0.46	0.02	0.03	0.00	0.15	0.03	0.00	0.05
Avail Cap(c_a), veh/h	1049	1911	1624	772	1911	1624	719	0	725	649	0	768
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	15.4	14.3	10.5	21.7	12.4	10.3	9.1	0.0	9.1	9.9	0.0	8.7
Incr Delay (d2), s/veh	0.0	1.5	0.1	0.5	0.5	0.0	0.1	0.0	0.4	0.1	0.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	7.2	0.5	0.9	3.8	0.1	0.2	0.0	1.1	0.2	0.0	0.4
LnGrp Delay(d),s/veh	15.4	15.8	10.6	22.2	12.9	10.3	9.1	0.0	9.5	10.0	0.0	8.8
LnGrp LOS	В	В	В	С	В	В	Α		Α	Α		Α
Approach Vol, veh/h		600			407			133			60	
Approach Delay, s/veh		15.4			14.2			9.5			9.2	
Approach LOS		В			В			Α			Α	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		29.0		26.1		29.0		26.1				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		24.5		56.5		24.5		56.5				
Max Q Clear Time (g_c+l1), s		4.2		15.6		4.7		19.3				
Green Ext Time (p_c), s		0.6		3.7		0.2		2.3				
Intersection Summary												
HCM 2010 Ctrl Delay			14.0									
HCM 2010 LOS			В									

	<₫	4	\$⊳	W.
Phase Number	2	4	6	8
Movement	NBTL	EBTL	SBTL	WBTL
Lead/Lag				
Lead-Lag Optimize				
Recall Mode	Max	None	Max	None
Maximum Split (s)	29	61	29	61
Maximum Split (%)	32.2%	67.8%	32.2%	67.8%
Minimum Split (s)	22.5	22.5	22.5	22.5
Yellow Time (s)	3.5	3.5	3.5	3.5
All-Red Time (s)	1	1	1	1
Minimum Initial (s)	5	5	5	5
Vehicle Extension (s)	3	3	3	3
Minimum Gap (s)	3	3	3	3
Time Before Reduce (s)	0	0	0	0
Time To Reduce (s)	0	0	0	0
Walk Time (s)	7	7	7	7
Flash Dont Walk (s)	11	11	11	11
Dual Entry	Yes	Yes	Yes	Yes
Inhibit Max	Yes	Yes	Yes	Yes
Start Time (s)	0	29	0	29
End Time (s)	29	0	29	0
Yield/Force Off (s)	24.5	85.5	24.5	85.5
Yield/Force Off 170(s)	13.5	74.5	13.5	74.5
Local Start Time (s)	0	29	0	29
Local Yield (s)	24.5	85.5	24.5	85.5
Local Yield 170(s)	13.5	74.5	13.5	74.5
Intersection Summary				
Cycle Length			90	
Control Type	Actuate	ed-Uncoo	rdinated	
Natural Cycle			50	
•				
Splits and Phases: 3: Steit	z Road &	Home Ro	oad	
d ø₂			Ø4	
79 s	0	61	S S	
	- 11		4	
▼ Ø6	100	3	▼ Ø8	

Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7	↑	↑	7	Y	
Traffic Vol, veh/h	37	545	312	26	7	11
Future Vol, veh/h	37	545	312	26	7	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	345	-	_	345	0	-
Veh in Median Storage		0	0	-	0	_
Grade, %		0	0	_	0	_
Peak Hour Factor	92	92	92	92	92	92
	2	2	2	2		2
Heavy Vehicles, %				28	2	
Mvmt Flow	40	592	339	28	8	12
Major/Minor	Major1	N	//ajor2	ı	Minor2	
Conflicting Flow All	367	0		0	1011	339
Stage 1	-	_	-	-	339	_
Stage 2	_	_	_	_	672	_
Critical Hdwy	4.12	_	_	-	6.42	6.22
Critical Hdwy Stg 1	-	_	_	_	5.42	-
Critical Hdwy Stg 2	_		_	_	5.42	_
Follow-up Hdwy	2.218	_	_		3.518	
Pot Cap-1 Maneuver	1192	-	-	_	265	703
•		-	-		722	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	508	-
Platoon blocked, %	1.100	-	-	-		
Mov Cap-1 Maneuver	1192	-	-	-	256	703
Mov Cap-2 Maneuver	-	-	-	-	256	-
Stage 1	-	-	-	-	697	-
Stage 2	-	-	-	-	508	-
Approach	EB		WB		SB	
	0.5		0 0		14	
HCM Control Delay, s	0.5		U			
HCM LOS					В	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1192	_	-	-	419
HCM Lane V/C Ratio		0.034	_	_		0.047
HCM Control Delay (s)		8.1	_	_	_	14
HCM Lane LOS		Α	_	_	_	В
HCM 95th %tile Q(veh))	0.1	_	_	_	0.1
HOW JOHN JOHN Q VOIL)	0.1				0.1

Intersection						
Int Delay, s/veh	0.1					
	EDI	EDT	WDT	MDD	ODI	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			₽			7
Traffic Vol, veh/h	0	552	330	19	0	8
Future Vol, veh/h	0	552	330	19	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	_	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	600	359	21	0	9
IVIVIIIL I IOW	U	000	000	Z 1	U	3
Major/Minor M	lajor1	N	Major2	N	/linor2	
Conflicting Flow All	_	0	_	0	_	370
Stage 1	-	_	-	_	-	_
Stage 2	_	_	_	_	_	_
Critical Hdwy	_	_	_	_	_	6.22
Critical Hdwy Stg 1	_	_	_	_	_	0.22
Critical Hdwy Stg 2	-	-	-	-	-	2 240
Follow-up Hdwy	-	-	-	-	-	3.318
Pot Cap-1 Maneuver	0	-	-	-	0	676
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	676
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	_	-	-	-	_	_
Stage 2	_	_	_	_	_	_
Glago L						
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		10.4	
HCM LOS					В	
Minor Lane/Major Mvmt		EBT	WBT	WBR S		
Capacity (veh/h)		-	-	-	0.0	
HCM Lane V/C Ratio		-	-	-	0.013	
HCM Control Delay (s)		-	-	-	10.4	
HCM Lane LOS		-	-	-	В	
HCM 95th %tile Q(veh)		_	_	-	0	
					•	

1.3					
	EDD	NDI	NDT	CDT	CDD
EBL	EBR	NBL	NBT	SBT	SBR
•					3
-					3
					0
Stop		Free		Free	Free
-	None	-	None	-	None
0	-	-	-	-	-
je,# 0	-	-	0	0	-
0	-	-	0	0	-
92	92	92	92	92	92
2	2	2	2	2	2
1	15	0	41	45	3
	_				
		Major1		/lajor2	
	47	-	0	-	0
	-	-	-	-	-
	-	-	-	-	-
6.42	6.22	-	-	-	-
5.42	-	-	-	-	-
5.42	-	-	-	-	-
3.518	3.318	-	-	-	-
913	1022	0	-	-	-
	-	0	-	-	-
	-		-	-	_
301			_	_	_
. 012	4000				
	11177				
913	1022	-	-	-	-
913	-	-	-	-	-
913 975				-	-
913	-	-	-	-	-
913 975	-	-	-	-	-
913 975	-	-	-	-	-
913 975 981 EB	-	- - - NB	-	- - - SB	-
913 975 981 EB	-	- - -	-	- - -	-
913 975 981 EB	-	- - - NB	-	- - - SB	-
913 975 981 EB 8.6 A		- - - NB 0	-	- - - SB 0	-
913 975 981 EB	- - - NBT [- - - NB 0	-	- - - SB	-
913 975 981 EB 8.6 A	- - - NBT [- - - NB 0	-	- - - SB 0	-
913 975 981 EB 8 8.6 A	- - - NBT [NB 0 =BLn1 1014 0.016	-	- - - SB 0	-
913 975 981 EB 8.6 A	- - - NBT [- - - NB 0	SBT	- - - SB 0	-
913 975 981 EB 8 8.6 A	- - - NBT [NB 0 =BLn1 1014 0.016	SBT	- - - SB 0	-
	Stop - 0 Stop - 0 ge, # 0 0 92 2 1 Minor2 88 47 41 6.42 5.42 5.42 3.518	1 14 1 14 1 14 0 0 0 Stop Stop - None 0 - 9e, # 0 - 92 92 2 2 2 1 15 Minor2 N 88 47 47 - 41 - 6.42 6.22 5.42 - 5.42 - 5.42 - 3.518 3.318 913 1022 975 -	1 14 0 1 14 0 1 14 0 5 0 0 0 Stop Stop Free - None - 0 9e, # 0 92 92 92 2 2 2 2 1 15 0 Minor2 Major1 88 47 - 41 6.42 6.22 - 5.42 5.42 3.518 3.318 - 913 1022 0 975 - 0	1 14 0 38 1 14 0 38 1 14 0 38 1 14 0 38 1 0 0 0 0 0 Stop Stop Free Free - None - None 0 0,e, # 0 0 92 92 92 92 2 2 2 2 2 1 15 0 41 Minor2 Major1 N 88 47 - 0 47 41 6.42 6.22 5.42 5.42 3.518 3.318 913 1022 0 - 975 - 0 - 981 - 0 -	1 14 0 38 41 1 14 0 38 41 1 14 0 38 41 1 0 0 0 0 0 0 Stop Stop Free Free Free - None - None 0 0 0 92 92 92 92 92 2 2 2 2 2 2 1 15 0 41 45 Minor2 Major1 Major2 88 47 - 0 - 41 41 6.42 6.22 5.42 5.42 3.518 3.318 913 1022 0 975 - 0 981 - 0

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	^	7	7	1		7	1	
Traffic Volume (veh/h)	16	384	48	33	552	21	49	16	23	11	5	15
Future Volume (veh/h)	16	384	48	33	552	21	49	16	23	11	5	15
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	17	417	52	36	600	23	53	17	25	12	5	16
Adj No. of Lanes	1	1	1	1	1	1	1	1	0	1	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	244	757	643	361	757	643	706	281	414	685	161	516
Arrive On Green	0.41	0.41	0.41	0.41	0.41	0.41	0.41	0.41	0.41	0.41	0.41	0.41
Sat Flow, veh/h	798	1863	1583	920	1863	1583	1385	682	1003	1359	391	1251
Grp Volume(v), veh/h	17	417	52	36	600	23	53	0	42	12	0	21
Grp Sat Flow(s),veh/h/ln	798	1863	1583	920	1863	1583	1385	0	1686	1359	0	1642
Q Serve(g_s), s	0.9	8.5	1.0	1.5	14.0	0.4	1.2	0.0	0.7	0.3	0.0	0.4
Cycle Q Clear(g_c), s	15.0	8.5	1.0	10.1	14.0	0.4	1.6	0.0	0.7	1.0	0.0	0.4
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.60	1.00		0.76
Lane Grp Cap(c), veh/h	244	757	643	361	757	643	706	0	695	685	0	677
V/C Ratio(X)	0.07	0.55	0.08	0.10	0.79	0.04	0.08	0.00	0.06	0.02	0.00	0.03
Avail Cap(c_a), veh/h	1373	3392	2884	1664	3392	2884	706	0	695	685	0	677
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	19.5	11.3	9.1	15.1	12.9	8.9	9.1	0.0	8.8	9.1	0.0	8.7
Incr Delay (d2), s/veh	0.1	0.6	0.1	0.1	1.9	0.0	0.2	0.0	0.2	0.0	0.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	4.4	0.4	0.4	7.6	0.2	0.5	0.0	0.4	0.1	0.0	0.2
LnGrp Delay(d),s/veh	19.6	11.9	9.1	15.2	14.8	8.9	9.4	0.0	9.0	9.1	0.0	8.8
LnGrp LOS	В	В	Α	В	В	Α	Α		Α	Α		Α
Approach Vol, veh/h		486			659			95			33	
Approach Delay, s/veh		11.9			14.7			9.2			8.9	
Approach LOS		В			В			Α			Α	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		25.0		24.7		25.0		24.7				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		20.5		90.5		20.5		90.5				
Max Q Clear Time (g_c+l1), s		3.6		17.0		3.0		16.0				
Green Ext Time (p_c), s		0.2		2.8		0.1		4.2				
Intersection Summary												
HCM 2010 Ctrl Delay			13.0									
HCM 2010 LOS			В									
Notes												

1	4	>	V
2	4	6	8
NBTL	EBTL	SBTL	WBTL
Max	None	Max	None
25	95	25	95
20.8%	79.2%	20.8%	79.2%
22.5	22.5	22.5	22.5
3.5	3.5	3.5	3.5
1	1	1	1
5	5	5	5
3	3	3	3
3	3	3	3
0	0	0	0
	0	0	0
7	7	7	7
11	11	11	11
Yes	Yes	Yes	Yes
			Yes
0		0	25
			0
			115.5
			104.5
			25
			115.5
			104.5
0.0		0.0	
Actuate	d-Uncoo		
		50	
itz Road &	Home Ro	oad	
1			
05 c			
728			
100			
	Max 25 20.8% 22.5 3.5 1 5 3 3 0 0 7 11 Yes Yes 0 25 20.5 9.5 0 20.5 9.5	Max None 25 95 20.8% 79.2% 22.5 22.5 3.5 3.5 1 1 5 5 3 3 3 3 0 0 0 0 7 7 11 11 Yes Yes Yes Yes Yes Yes 0 25 25 0 20.5 115.5 9.5 104.5 0 25 20.5 115.5 9.5 104.5 Actuated-Uncoo	Max None Max 25 95 25 20.8% 79.2% 20.8% 22.5 22.5 3.5 3.5 3.5 1 1 1 1 5 5 5 5 3 3 3 3 3 3 3 3 3 3 3

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑	7	ħ	^	7	7	1		ħ	1	
Traffic Volume (veh/h)	16	404	52	33	608	21	61	16	23	52	14	15
Future Volume (veh/h)	16	404	52	33	608	21	61	16	23	52	14	15
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	17	439	57	36	661	23	66	17	25	57	15	16
Adj No. of Lanes	1	1	1	1	1	1	1	1	0	1	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	204	796	677	345	796	677	686	289	425	675	350	373
Arrive On Green	0.43	0.43	0.43	0.43	0.43	0.43	0.42	0.42	0.42	0.42	0.42	0.42
Sat Flow, veh/h	754	1863	1583	898	1863	1583	1373	682	1003	1359	826	881
Grp Volume(v), veh/h	17	439	57	36	661	23	66	0	42	57	0	31
Grp Sat Flow(s),veh/h/ln	754	1863	1583	898	1863	1583	1373	0	1686	1359	0	1707
Q Serve(g_s), s	1.2	10.6	1.3	1.9	19.0	0.5	1.8	0.0	0.9	1.6	0.0	0.6
Cycle Q Clear(g_c), s	20.2	10.6	1.3	12.5	19.0	0.5	2.4	0.0	0.9	2.4	0.0	0.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.60	1.00		0.52
Lane Grp Cap(c), veh/h	204	796	677	345	796	677	686	0	713	675	0	723
V/C Ratio(X)	0.08	0.55	0.08	0.10	0.83	0.03	0.10	0.00	0.06	0.08	0.00	0.04
Avail Cap(c_a), veh/h	952	2643	2247	1235	2643	2247	686	0	713	675	0	723
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.3	12.9	10.2	17.6	15.3	10.0	10.9	0.0	10.3	11.0	0.0	10.2
Incr Delay (d2), s/veh	0.2	0.6	0.1	0.1	2.3	0.0	0.3	0.0	0.2	0.2	0.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	5.5	0.6	0.5	10.1	0.2	0.7	0.0	0.4	0.6	0.0	0.3
LnGrp Delay(d),s/veh	24.5	13.5	10.3	17.7	17.6	10.0	11.2	0.0	10.4	11.2	0.0	10.3
LnGrp LOS	С	В	В	В	В	В	В		В	В		В
Approach Vol, veh/h		513			720			108			88	
Approach Delay, s/veh		13.5			17.4			10.9			10.9	
Approach LOS		В			В			В			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		30.0		30.3		30.0		30.3				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		25.5		85.5		25.5		85.5				
Max Q Clear Time (g_c+l1), s		4.4		22.2		4.4		21.0				
Green Ext Time (p_c), s		0.3		3.0		0.2		4.8				
Intersection Summary												
HCM 2010 Ctrl Delay			15.1									
HCM 2010 LOS			В									
Notes												

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Phase Number	2	4	6	8
Movement	NBTL	EBTL	SBTL	WBTL
Lead/Lag				
Lead-Lag Optimize				
Recall Mode	Max	None	Max	None
Maximum Split (s)	30	90	30	90
Maximum Split (%)	25.0%	75.0%	25.0%	75.0%
Minimum Split (s)	22.5	22.5	22.5	22.5
Yellow Time (s)	3.5	3.5	3.5	3.5
All-Red Time (s)	1	1	1	1
Minimum Initial (s)	5	5	5	5
Vehicle Extension (s)	3	3	3	3
Minimum Gap (s)	3	3	3	3
Time Before Reduce (s)	0	0	0	0
Time To Reduce (s)	0	0	0	0
Walk Time (s)	7	7	7	7
Flash Dont Walk (s)	11	11	11	11
Dual Entry	Yes	Yes	Yes	Yes
Inhibit Max	Yes	Yes	Yes	Yes
Start Time (s)	0	30	0	30
End Time (s)	30	0	30	0
Yield/Force Off (s)	25.5	115.5	25.5	115.5
Yield/Force Off 170(s)	14.5	104.5	14.5	104.5
Local Start Time (s)	0	30	0	30
Local Yield (s)	25.5	115.5	25.5	115.5
Local Yield 170(s)	14.5	104.5	14.5	104.5
Intersection Summary				
Cycle Length			120	
Control Type	Actuate	d-Uncoo	rdinated	
Natural Cycle			55	
,				
Splits and Phases: 3: Steitz	z Road &	Home Ro	oad	
↑ ø2	- 12	04		
30 s	90 4	ידשי		
		4		
▼ Ø6	. 1	Ø8		

Intersection						
Int Delay, s/veh	1.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	↑	↑	7	Y	
Traffic Vol, veh/h	59	448	644	39	24	36
Future Vol, veh/h	59	448	644	39	24	36
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	345	-	_	345	0	-
Veh in Median Storage		0	0	-	0	_
Grade, %		0	0	_	0	<u>-</u>
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	64	487	700	42	26	39
INIVITIL FIOW	04	401	700	42	20	39
Major/Minor	Major1	N	//ajor2		Minor2	
Conflicting Flow All	742	0	-	0	1315	700
Stage 1	-	-	-	-	700	-
Stage 2	_	-	_	_	615	-
Critical Hdwy	4.12	-	_	_	6.42	6.22
Critical Hdwy Stg 1		_	_	_	5.42	-
Critical Hdwy Stg 2	_			_	5.42	_
Follow-up Hdwy	2.218	_	_		3.518	
Pot Cap-1 Maneuver	865	_	-	_	174	439
Stage 1	- 005	_	_	_	493	439
		-	-		539	
Stage 2	-	-	-	-	539	-
Platoon blocked, %	005	-	-	-	101	400
Mov Cap-1 Maneuver		-	-	-	161	439
Mov Cap-2 Maneuver	-	-	-	-	161	-
Stage 1	-	-	-	-	457	-
Stage 2	-	-	-	-	539	-
Approach	EB		WB		SB	
HCM Control Delay, s	1.1		0		23.4	
HCM LOS	1.1		U		23.4 C	
I IOWI LOO					U	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		865	-	-	-	260
HCM Lane V/C Ratio		0.074	-	-	_	0.251
HCM Control Delay (s))	9.5	-	-	-	23.4
HCM Lane LOS		Α	-	-	_	С
HCM 95th %tile Q(veh)	0.2	_	_	_	1
	,					

Intersection						
Int Delay, s/veh	0.3					
		EDT	WDT	WDD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	^	470	1	00		7
Traffic Vol, veh/h	0	472	655	29	0	28
Future Vol, veh/h	0	472	655	29	0	28
Conflicting Peds, #/hr	0	_ 0	0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage,		0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	513	712	32	0	30
Major/Minor N	lajor1	N	Major2	N	Minor2	
Conflicting Flow All	- -	0	- viajoiz	0	-	728
Stage 1	-	-	-	-	-	-
Stage 2	-		-	-	-	-
Critical Hdwy	-	-	-	-	-	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.318
Pot Cap-1 Maneuver	0	-	-	-	0	423
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	423
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Annroach	EB		WB		SB	
Approach						
HCM Control Delay, s	0		0		14.2	
HCM LOS					В	
Minor Lane/Major Mvmt		EBT	WBT	WBR S	SBLn1	
Capacity (veh/h)		-	_	_		
HCM Lane V/C Ratio		_	_		0.072	
HCM Control Delay (s)		_	_	_		
				_	В	
HCM Lane LOS		-	-	-	1.3	
HCM Lane LOS HCM 95th %tile Q(veh)		-	-	_	0.2	

Intersection						
Int Delay, s/veh	3.6					
Movement	[DI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y	=6		↑	4	
Traffic Vol, veh/h	4	50	0	42	31	4
Future Vol, veh/h	4	50	0	42	31	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		_	_	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	54	0	46	34	4
Major/Minor	Minor2	N	Major1	N	/lajor2	
Conflicting Flow All	82	36	- -	0	-	0
	36	-		-		
Stage 1			-		-	-
Stage 2	46	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	-	-
Pot Cap-1 Maneuver	920	1037	0	-	-	-
Stage 1	986	-	0	-	_	_
Stage 2	976	_	0	_	_	_
Platoon blocked, %	310		U	_	_	_
	020	1027				
Mov Cap-1 Maneuver	920	1037	-	-	-	-
Mov Cap-2 Maneuver	920	-	-	-	-	-
Stage 1	986	-	-	-	-	-
Stage 2	976	-	-	-	-	-
Annroach	ΓD		ND		CD	
Approach	EB		NB		SB	
HCM Control Delay, s	8.7		0		0	
HCM LOS	Α					
Minor Lane/Major Mvn	nt	NRT	EBLn1	SBT	SBR	
	κ				אומט	
Capacity (veh/h)			1027	-	-	
HCM Lane V/C Ratio		-	0.057	-	-	
HCM Control Delay (s)		-	8.7	-	-	
HCM Lane LOS		-	Α	-	-	
HCM 95th %tile Q(veh)	-	0.2	-	-	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑	7	7	↑	7	7	1		7	1	
Traffic Volume (veh/h)	13	611	58	69	341	11	17	23	97	7	22	20
Future Volume (veh/h)	13	611	58	69	341	11	17	23	97	7	22	20
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	14	664	63	75	371	12	18	25	105	8	24	22
Adj No. of Lanes	1	1	1	1	1	1	1	1	0	1	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	457	873	742	248	873	742	612	119	500	530	340	312
Arrive On Green	0.47	0.47	0.47	0.47	0.47	0.47	0.38	0.38	0.38	0.38	0.38	0.38
Sat Flow, veh/h	996	1863	1583	725	1863	1583	1354	314	1317	1255	896	822
Grp Volume(v), veh/h	14	664	63	75	371	12	18	0	130	8	0	46
Grp Sat Flow(s),veh/h/ln	996	1863	1583	725	1863	1583	1354	0	1630	1255	0	1718
Q Serve(g_s), s	0.6	17.4	1.3	5.7	7.8	0.2	0.5	0.0	3.2	0.3	0.0	1.0
Cycle Q Clear(g_c), s	8.4	17.4	1.3	23.1	7.8	0.2	1.5	0.0	3.2	3.4	0.0	1.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.81	1.00		0.48
Lane Grp Cap(c), veh/h	457	873	742	248	873	742	612	0	619	530	0	652
V/C Ratio(X)	0.03	0.76	0.08	0.30	0.42	0.02	0.03	0.00	0.21	0.02	0.00	0.07
Avail Cap(c_a), veh/h	973	1838	1562	623	1838	1562	612	0	619	530	0	652
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	13.2	13.0	8.7	22.5	10.4	8.4	12.2	0.0	12.4	13.6	0.0	11.7
Incr Delay (d2), s/veh	0.0	1.4	0.0	0.7	0.3	0.0	0.1	0.0	0.8	0.1	0.0	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	9.2	0.6	1.2	4.1	0.1	0.2	0.0	1.5	0.1	0.0	0.5
LnGrp Delay(d),s/veh	13.3	14.4	8.8	23.2	10.8	8.4	12.3	0.0	13.2	13.6	0.0	11.9
LnGrp LOS	В	В	Α	С	В	Α	В		В	В		B
Approach Vol, veh/h		741			458			148			54	
Approach Delay, s/veh		13.9			12.8			13.1			12.2	
Approach LOS		В			В			В			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		27.0		32.3		27.0		32.3				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		22.5		58.5		22.5		58.5				
Max Q Clear Time (g_c+l1), s		5.2		19.4		5.4		25.1				
Green Ext Time (p_c), s		0.6		4.9		0.1		2.7				
Intersection Summary												
HCM 2010 Ctrl Delay			13.4									
HCM 2010 LOS			В									_
Notes												

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Phase Number	2	4	6	8
Movement	NBTL	EBTL	SBTL	WBTL
Lead/Lag				
Lead-Lag Optimize				
Recall Mode	Max	None	Max	None
Maximum Split (s)	27	63	27	63
Maximum Split (%)	30.0%	70.0%	30.0%	70.0%
Minimum Split (s)	22.5	22.5	22.5	22.5
Yellow Time (s)	3.5	3.5	3.5	3.5
All-Red Time (s)	1	1	1	1
Minimum Initial (s)	5	5	5	5
Vehicle Extension (s)	3	3	3	3
Minimum Gap (s)	3	3	3	3
Time Before Reduce (s)	0	0	0	0
Time To Reduce (s)	0	0	0	0
Walk Time (s)	7	7	7	7
Flash Dont Walk (s)	11	11	11	11
Dual Entry	Yes	Yes	Yes	Yes
Inhibit Max	Yes	Yes	Yes	Yes
Start Time (s)	0	27	0	27
End Time (s)	27	0	27	0
Yield/Force Off (s)	22.5	85.5	22.5	85.5
Yield/Force Off 170(s)	11.5	74.5	11.5	74.5
Local Start Time (s)	0	27	0	27
Local Yield (s)	22.5	85.5	22.5	85.5
Local Yield 170(s)	11.5	74.5	11.5	74.5
Intersection Summary				
Cycle Length			90	
Control Type	Actuate	ed-Uncoo	rdinated	
Natural Cycle			55	
•				
Splits and Phases: 3: Stei	tz Road &	Home Ro	oad	
<.		人		
Ø2	-	-	Ø4	
2/s		63 s		
Ø6		4	Ø8	
27.0	-	63 e	20	

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
				↑	•						
											20
											20
											16
	0			0			0			0	0
											1.00
											1.00
											1900
											22
											0
											0.92
											2
											287
											0.36
											790
											48
											1723
											1.0
	16.6			8.4			0.0			0.0	1.0
	200			000							0.46
											626
											0.08
											626
											1.00
											1.00
											11.8
											0.2
											0.0
											0.5
							0.0			0.0	12.0
В		A	U		A	В	457	В	Б	CO	В
	В			В			В			В	
1	2	3	4	5	6	7	8				
	2		4		6		8				
	25.0		31.5		25.0		31.5				
	4.5		4.5		4.5		4.5				
	20.5		60.5		20.5		60.5				
	5.1		18.6		5.8		24.0				
	0.6		5.0		0.2		3.0				
		12.6									
		В									
	13 13 7 0 1.00 1.00 1863 14 1 0.92 2 444 0.48 960 0.6 8.9 1.00 444 0.03 1014 1.00 1.00 12.9 0.0 0.2 12.9 B	13 617 13 617 7 4 0 0 1.00 1.00 1.00 1.00 1863 1863 14 671 1 1 0.92 0.92 2 2 444 890 0.48 0.48 960 1863 14 671 960 1863 0.6 16.6 8.9 16.6 1.00 444 890 0.03 0.75 1014 1995 1.00 1.00 1.00 1.00 12.9 12.0 0.0 1.3 0.0 0.0 0.2 8.7 12.9 13.4 B B 749 12.9 B 1 2 25.0 4.5 20.5 5.1	13 617 59 13 617 59 7 4 14 0 0 0 0 1.00 1.00 1.00 1.00 1863 1863 1863 14 671 64 1 1 1 0.92 0.92 0.92 2 2 2 444 890 756 0.48 0.48 0.48 960 1863 1583 14 671 64 960 1863 1583 0.6 16.6 1.2 1.00 1.00 444 890 756 0.03 0.75 0.08 1014 1995 1696 1.00	13 617 59 69 13 617 59 69 7 4 14 3 0 0 0 0 0 1.00 1.00 1.00 1.00 1.00 1.00	13 617 59 69 378 13 617 59 69 378 7 4 14 3 8 0 0 0 0 0 0 0 1.00 1.00 1.00 1.00 1.00	13 617 59 69 378 11 13 617 59 69 378 11 7 4 14 3 8 18 0 0 0 0 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 14 671 64 75 411 12 1	13 617 59 69 378 11 25 13 617 59 69 378 11 25 7 4 14 3 8 18 5 0 0 0 0 0 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00<	13 617 59 69 378 11 25 23 13 617 59 69 378 11 25 23 7 4 14 3 8 18 5 2 0 0 0 0 0 0 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.01 1	13 617 59 69 378 11 25 23 97 13 617 59 69 378 11 25 23 97 7 4 14 3 8 18 5 2 12 0 0 0 0 0 0 0 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.4 671 64 75 411 12 27 25 20 2 2 2 2 2 <td> 13</td> <td> 13</td>	13	13

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Phase Number	2	4	6	8
Movement	NBTL	EBTL	SBTL	WBTL
Lead/Lag			55.5	
Lead-Lag Optimize				
Recall Mode	Max	None	Max	None
Maximum Split (s)	25	65	25	65
Maximum Split (%)	27.8%	72.2%	27.8%	72.2%
Minimum Split (s)	22.5	22.5	22.5	22.5
Yellow Time (s)	3.5	3.5	3.5	3.5
All-Red Time (s)	1	1	1	1
Minimum Initial (s)	5	5	5	5
Vehicle Extension (s)	3	3	3	3
Minimum Gap (s)	3	3	3	3
Time Before Reduce (s)	0	0	0	0
Time To Reduce (s)	0	0	0	0
Walk Time (s)	7	7	7	7
Flash Dont Walk (s)	11	11	11	11
Dual Entry	Yes	Yes	Yes	Yes
Inhibit Max	Yes	Yes	Yes	Yes
Start Time (s)	0	25	0	25
End Time (s)	25	0	25	0
Yield/Force Off (s)	20.5	85.5	20.5	85.5
Yield/Force Off 170(s)	9.5	74.5	9.5	74.5
Local Start Time (s)	0	25	0	25
Local Yield (s)	20.5	85.5	20.5	85.5
Local Yield 170(s)	9.5	74.5	9.5	74.5
Intersection Summary				
Cycle Length			90	
Control Type	Actuate	ed-Uncoo		
Natural Cycle	, 1010.011	J. 011000	55	
Splits and Phases: 3: Steit	z Road &	Home Ro	oad	
√Îø2		₽ 04		
25 s		65 s		
		44		
▼ Ø6		▼ Ø8		

Intersection						
Int Delay, s/veh	0.5					
-	EDI	EDT	WDT	WDD	ODL	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7	^	•	7	Y	
Traffic Vol, veh/h	37	682	386	26	7	11
Future Vol, veh/h	37	682	386	26	7	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	345	-	-	345	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	40	741	420	28	8	12
WIVIII(I IOW	40	7-7-1	720	20	U	12
Major/Minor	Major1	N	Major2	ľ	Minor2	
Conflicting Flow All	448	0	-	0	1241	420
Stage 1	-	-	-	-	420	-
Stage 2	-	-	-	-	821	-
Critical Hdwy	4.12	-	_	-	6.42	6.22
Critical Hdwy Stg 1	- 1.12	_	_	_	5.42	-
Critical Hdwy Stg 2	_	_		_	5.42	_
Follow-up Hdwy	2.218	_	_		3.518	
Pot Cap-1 Maneuver	1112	_	_	_	193	633
•		-	-		663	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	432	-
Platoon blocked, %	44.5	-	-	-	100	000
Mov Cap-1 Maneuver		-	-	-	186	633
Mov Cap-2 Maneuver	-	-	-	-	186	-
Stage 1	-	-	-	-	639	-
Stage 2	-	-	-	-	432	-
Annroach	EB		WB		SB	
Approach						
HCM Control Delay, s	0.4		0		16.7	
HCM LOS					С	
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1112			-	327
HCM Lane V/C Ratio		0.036		_	<u> </u>	0.06
	\	8.4	-	-		16.7
HCM Control Delay (s)			-	-	
HCM Lane LOS		Α	-	-	-	С
HCM 95th %tile Q(veh	1)	0.1	-	-	-	0.2

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		†	†	7		7
Traffic Vol, veh/h	0	689	404	19	0	8
Future Vol, veh/h	0	689	404	19	0	8
Conflicting Peds, #/hr	0	003	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	- -	None
		-		345	_	0
Storage Length	<u>-</u> ц		-			
Veh in Median Storage,		0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	749	439	21	0	9
Major/Minor Ma	ajor1	N	Major2		/linor2	
						400
Conflicting Flow All	-	0	-	0	-	439
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.318
Pot Cap-1 Maneuver	0	_	-	-	0	618
Stage 1	0	_	_	-	0	-
Stage 2	0	_	_	_	0	_
Platoon blocked, %	U	_	_	_	U	
		_	-			618
Mov Cap-1 Maneuver	-	-		-	-	010
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		10.9	
HCM LOS	U		U		10.9 B	
HOW LOS					D	
Minor Lane/Major Mvmt		EBT	WBT	WBR S	SBLn1	
Capacity (veh/h)					618	
HCM Lane V/C Ratio		<u>-</u>	_		0.014	
HCM Control Delay (s)		_	_	_	10.9	
HCM Lane LOS				_	В	
		-	-			
HCM 95th %tile Q(veh)		-	-	-	0	

Intersection						
Intersection Int Delay, s/veh	1.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			•	1	
Traffic Vol, veh/h	1	14	0	17	49	3
Future Vol, veh/h	1	14	0	17	49	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	15	0	18	53	3
	Minor2		//ajor1		//ajor2	
Conflicting Flow All	73	55	-	0	-	0
Stage 1	55	-	-	-	-	-
Stage 2	18	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	-	-
Pot Cap-1 Maneuver	931	1012	0	-	-	-
Stage 1	968	-	0	_	-	-
Stage 2	1005	_	0	_	-	_
Platoon blocked, %	. 300			_	_	_
Mov Cap-1 Maneuver	931	1012	_	_	_	_
Mov Cap-1 Maneuver	931	-	_	_	_	_
Stage 1	968	_				_
_	1005	-	_	-	_	-
Stage 2	1003	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.6		0		0	
HCM LOS	Α					
Mineral and Maria Ad	-1	NET	-DL 4	OPT	ODD	
Minor Lane/Major Mvr	nt		EBLn1	SBT	SBR	
Capacity (veh/h)			1006	-		
Capacity (veh/h) HCM Lane V/C Ratio			0.016	-	-	
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		0.016		- -	
Capacity (veh/h) HCM Lane V/C Ratio		-	0.016	-	- -	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑	7	7	↑	7	7	1		7	1	
Traffic Volume (veh/h)	20	480	60	41	690	26	59	19	28	13	6	18
Future Volume (veh/h)	20	480	60	41	690	26	59	19	28	13	6	18
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	22	522	65	45	750	28	64	21	30	14	7	20
Adj No. of Lanes	1	1	1	1	1	1	1	1	0	1	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	203	891	757	343	891	757	621	262	374	598	161	460
Arrive On Green	0.48	0.48	0.48	0.48	0.48	0.48	0.38	0.38	0.38	0.38	0.38	0.38
Sat Flow, veh/h	691	1863	1583	825	1863	1583	1378	695	993	1348	427	1220
Grp Volume(v), veh/h	22	522	65	45	750	28	64	0	51	14	0	27
Grp Sat Flow(s),veh/h/ln	691	1863	1583	825	1863	1583	1378	0	1688	1348	0	1647
Q Serve(g_s), s	1.8	12.7	1.4	2.6	21.9	0.6	1.9	0.0	1.2	0.4	0.0	0.6
Cycle Q Clear(g_c), s	23.7	12.7	1.4	15.3	21.9	0.6	2.6	0.0	1.2	1.6	0.0	0.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.59	1.00		0.74
Lane Grp Cap(c), veh/h	203	891	757	343	891	757	621	0	636	598	0	621
V/C Ratio(X)	0.11	0.59	0.09	0.13	0.84	0.04	0.10	0.00	0.08	0.02	0.00	0.04
Avail Cap(c_a), veh/h	843	2616	2223	1107	2616	2223	621	0	636	598	0	621
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.5	11.8	8.8	17.3	14.2	8.6	13.1	0.0	12.5	13.0	0.0	12.3
Incr Delay (d2), s/veh	0.2	0.6	0.0	0.2	2.2	0.0	0.3	0.0	0.2	0.1	0.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	6.5	0.6	0.6	11.6	0.3	0.8	0.0	0.6	0.2	0.0	0.3
LnGrp Delay(d),s/veh	24.8	12.4	8.9	17.5	16.4	8.6	13.4	0.0	12.7	13.1	0.0	12.4
LnGrp LOS	С	В	Α	В	В	Α	В		В	В		B
Approach Vol, veh/h		609			823			115			41	
Approach Delay, s/veh		12.5			16.2			13.1			12.6	
Approach LOS		В			В			В			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		28.0		34.3		28.0		34.3				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		23.5		87.5		23.5		87.5				
Max Q Clear Time (g_c+l1), s		4.6		25.7		3.6		23.9				
Green Ext Time (p_c), s		0.3		3.8		0.1		5.9				
Intersection Summary												
HCM 2010 Ctrl Delay			14.5									
HCM 2010 LOS			В									
Notes												

	<↑	4	↓ ⊳	*
Phase Number	2	4	6	8
Movement	NBTL	EBTL	SBTL	WBTL
Lead/Lag				
Lead-Lag Optimize				
Recall Mode	Max	None	Max	None
Maximum Split (s)	28	92	28	92
Maximum Split (%)	23.3%	76.7%	23.3%	76.7%
Minimum Split (s)	22.5	22.5	22.5	22.5
Yellow Time (s)	3.5	3.5	3.5	3.5
All-Red Time (s)	1	1	1	1
Minimum Initial (s)	5	5	5	5
Vehicle Extension (s)	3	3	3	3
Minimum Gap (s)	3	3	3	3
Time Before Reduce (s)	0	0	0	0
Time To Reduce (s)	0	0	0	0
Walk Time (s)	7	7	7	7
Flash Dont Walk (s)	11	11	11	11
Dual Entry	Yes	Yes	Yes	Yes
Inhibit Max	Yes	Yes	Yes	Yes
Start Time (s)	0	28	0	28
End Time (s)	28	0	28	0
Yield/Force Off (s)	23.5	115.5	23.5	115.5
Yield/Force Off 170(s)	12.5	104.5	12.5	104.5
Local Start Time (s)	0	28	0	28
Local Yield (s)	23.5	115.5	23.5	115.5
Local Yield 170(s)	12.5	104.5	12.5	104.5
Intersection Summary				
Cycle Length			120	
Control Type	Actuate	ed-Uncoo		
Natural Cycle	riotaate	,	60	
rtatarar Oyolo			00	
Splits and Phases: 3: Steit	z Road &	Home Ro	nad	
4	_ Noud &	- 101110 TKC	,uu	
Tø2	4	Ø 4		
28 s	92 s			
I.	2			
♥ Ø6	92.0	Ø8		

Lane Configurations Traffic Volume (velhh) 20 500 64 41 746 26 71 19 28 54 15 18 Number 7 414 38 818 52 212 16 616 Initial Q (Qb), veh 7 414 38 818 52 212 16 616 Initial Q (Qb), veh 7 414 38 818 52 212 16 616 Initial Q (Qb), veh 7 414 38 818 52 212 16 616 Initial Q (Qb), veh 7 414 38 818 52 212 16 616 Initial Q (Qb), veh 7 414 38 818 52 212 16 616 Initial Q (Qb), veh 7 414 43 818 818 82 818 82 818 83 818 83 818 83 818 83 818 83 818 83 818 83 818 83 818 83 818 83 818 83 818 83 818 83 818 83 818 83 818 83 818 83 83		۶		•	•	*	•	4	1	~	1	ļ	4
Traffire Volume (vehlh)	Movement		EBT		WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Future Volume (velvh)	Lane Configurations	7	↑	7	7	^	7	7	1		7	1	
Number 7 4 14 3 8 8 18 5 2 12 1 1 6 16 16 initial Q (Qb), veh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Traffic Volume (veh/h)		500		41	746	26		19	28	54	15	
Initial Q(0b), weh O	Future Volume (veh/h)		500			746					54		
Peac-Bite Adji(A_pbT)	Number				3	8	18					6	16
Parking Bus, Ādj	Initial Q (Qb), veh		0		•	0			0			0	
Adj Sat Flow, vehirh/in 1863 1863 1863 1863 1863 1863 1863 1863	,												
Adj Flow Rate, veh/h													
Adj No. of Lanes	Adj Sat Flow, veh/h/ln												
Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	Adj Flow Rate, veh/h												
Percent Heavy Veh, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2													
Cap, veh/h 194 949 807 357 949 807 574 246 352 560 267 334 Arrive On Green 0.51 0.51 0.51 0.51 0.51 0.51 0.51 0.51													
Arrive On Green	Percent Heavy Veh, %												
Sat Flow, veh/h 653 1863 1583 806 1863 1583 1367 695 993 1348 754 942 Grp Volume(v), veh/h 22 543 70 45 811 28 77 0 51 59 0 36 Grp Sat Flow(s), veh/h/ln 653 1863 1583 806 1863 1583 1367 0 1688 1348 0 1696 Q Serve(g.s), s 2.0 13.4 1.5 2.7 25.1 0.6 2.6 0.0 1.3 2.0 0.0 0.9 Cycle Q Clear(g.c), s 27.1 13.4 1.5 16.1 25.1 0.6 3.5 0.0 1.3 3.4 0.0 0.9 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.59 1.00 0.59 1.00 0.59 1.00 0.59 1.00 0.60 Avail Cap(c., veh/h 724 2460 2091 1010 2460 2091 574 0 599 560 0 602 V/C Ratio(X) 0.11 0.57 0.09 0.13 0.85 0.03 0.13 0.00 0.09 0.11 0.00 0.06 Avail Cap(c., a), veh/h 724 2460 2091 1010 1.00 1.00 0.100 0.100 0.100 0.100 0.100 0.100 0.100 0.00 0.00 EVIC Ratio(X) 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.0	Cap, veh/h												
Grp Volume(v), veh/h Grp Saft Flow(s), veh/h/ln Grp Saft Flow(s), veh/h Grp Saft Flow(s), veh/h/ln Grp Saft Flow(s), veh/h Grp Saft	Arrive On Green												
Grp Sat Flow(s), veh/h/ln 653 1863 1583 806 1863 1583 1367 0 1688 1348 0 1696 Q Serve(g_s), s 2.0 13.4 1.5 2.7 25.1 0.6 2.6 0.0 1.3 2.0 0.0 0.9 Cycle Q Clear(g_c), s 27.1 13.4 1.5 16.1 25.1 0.6 3.5 0.0 1.3 3.4 0.0 0.9 Prop In Lane 1.00 1.00 1.00 1.00 1.00 0.59 1.00 0.56 Lane Grp Cap(c), veh/h 194 949 807 357 949 807 574 0 599 560 0 602 V/C Ratio(X) 0.11 0.57 0.09 0.13 0.85 0.03 0.13 0.00 0.09 0.11 0.00 0.06 Avail Cap(c_a), veh/h 724 2460 2091 1010 2460 2091 574 0 599 560 0 602 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	Sat Flow, veh/h	653	1863	1583	806	1863	1583	1367	695	993	1348	754	
Q Serve(g_s), s	Grp Volume(v), veh/h								0			0	
Cycle Q Člear(g_c), s	Grp Sat Flow(s),veh/h/ln	653	1863	1583	806	1863	1583	1367	0	1688	1348	0	1696
Prop In Lane	Q Serve(g_s), s				2.7		0.6		0.0			0.0	
Lane Grp Cap(c), veh/h 194 949 807 357 949 807 574 0 599 560 0 602 V/C Ratio(X) 0.11 0.57 0.09 0.13 0.85 0.03 0.13 0.00 0.09 0.11 0.00 0.06 Avail Cap(c_a), veh/h 724 2460 2091 1010 2460 2091 574 0 599 560 0 602 Avail Cap(c_a), veh/h 724 2460 2091 1010 2460 2091 574 0 599 560 0 602 CMC Ratio(X) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	Cycle Q Clear(g_c), s	27.1	13.4		16.1	25.1			0.0	1.3		0.0	
V/C Ratio(X)	Prop In Lane				1.00								
Avail Cap(c_a), veh/h 724 2460 2091 1010 2460 2091 574 0 599 560 0 602 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	Lane Grp Cap(c), veh/h												
HCM Platoon Ratio	V/C Ratio(X)											0.00	
Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 0.00 1.00 0.00 1.00 0.00 1.00 1.00 0.00 1.00 0.00 1.00 1.00 1.00 0.00 1.00 0.0 <td></td>													
Uniform Delay (d), s/veh	HCM Platoon Ratio												
Incr Delay (d2), s/veh													
Initial Q Delay(d3),s/veh													
%ile BackOfQ(50%),veh/ln													
LnGrp Delay(d),s/veh 26.1 11.8 8.4 17.0 16.4 8.1 15.7 0.0 14.5 15.7 0.0 14.5 15.7 0.0 14.5 15.7 0.0 14.3 LnGrp LOS C B A B													
LnGrp LOS C B A B B A B A S S S	` ,												
Approach Vol, veh/h Approach Delay, s/veh Approach Delay, s/veh Approach LOS B B B B B B B B B B B B B B B B B B B	LnGrp Delay(d),s/veh								0.0			0.0	
Approach Delay, s/veh 11.9 16.2 15.3 15.2 Approach LOS B B B B B Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 4 6 8 Phs Duration (G+Y+Rc), s 28.0 38.3 28.0 38.3 Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 23.5 87.5 Max Q Clear Time (g_c+I1), s 5.5 29.1 5.4 27.1 Green Ext Time (p_c), s 0.4 4.0 0.3 6.7 Intersection Summary HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B	LnGrp LOS	С		Α	В		Α	В		В	В		B
Approach LOS B B B B B Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 4 6 8 Phs Duration (G+Y+Rc), s 28.0 38.3 28.0 38.3 Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 23.5 87.5 23.5 87.5 Max Q Clear Time (g_c+l1), s 5.5 29.1 5.4 27.1 Green Ext Time (p_c), s 0.4 4.0 0.3 6.7 Intersection Summary HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B	Approach Vol, veh/h												
Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 4 6 8 Phs Duration (G+Y+Rc), s 28.0 38.3 28.0 38.3 Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 23.5 87.5 23.5 87.5 Max Q Clear Time (g_c+I1), s 5.5 29.1 5.4 27.1 Green Ext Time (p_c), s 0.4 4.0 0.3 6.7 Intersection Summary HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B	Approach Delay, s/veh		11.9			16.2			15.3			15.2	
Assigned Phs 2 4 6 8 Phs Duration (G+Y+Rc), s 28.0 38.3 28.0 38.3 Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 23.5 87.5 23.5 87.5 Max Q Clear Time (g_c+l1), s 5.5 29.1 5.4 27.1 Green Ext Time (p_c), s 0.4 4.0 0.3 6.7 Intersection Summary HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B	Approach LOS		В			В			В			В	
Assigned Phs 2 4 6 8 Phs Duration (G+Y+Rc), s 28.0 38.3 28.0 38.3 Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 23.5 87.5 23.5 87.5 Max Q Clear Time (g_c+l1), s 5.5 29.1 5.4 27.1 Green Ext Time (p_c), s 0.4 4.0 0.3 6.7 Intersection Summary HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B	Timer	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s 28.0 38.3 28.0 38.3 Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 23.5 87.5 23.5 87.5 Max Q Clear Time (g_c+I1), s 5.5 29.1 5.4 27.1 Green Ext Time (p_c), s 0.4 4.0 0.3 6.7 Intersection Summary HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B	Assigned Phs				4								
Change Period (Y+Rc), s 4.5 4.5 4.5 Max Green Setting (Gmax), s 23.5 87.5 23.5 87.5 Max Q Clear Time (g_c+l1), s 5.5 29.1 5.4 27.1 Green Ext Time (p_c), s 0.4 4.0 0.3 6.7 Intersection Summary HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B			28.0		38.3		28.0		38.3				
Max Green Setting (Gmax), s 23.5 87.5 23.5 87.5 Max Q Clear Time (g_c+l1), s 5.5 29.1 5.4 27.1 Green Ext Time (p_c), s 0.4 4.0 0.3 6.7 Intersection Summary HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B	Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Q Clear Time (g_c+l1), s 5.5 29.1 5.4 27.1 Green Ext Time (p_c), s 0.4 4.0 0.3 6.7 Intersection Summary HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B	` ,						23.5		87.5				
Green Ext Time (p_c), s 0.4 4.0 0.3 6.7 Intersection Summary HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B							5.4						
HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B	Green Ext Time (p_c), s												
HCM 2010 Ctrl Delay 14.5 HCM 2010 LOS B	Intersection Summary												
HCM 2010 LOS B				14.5									
Notes	HCM 2010 LOS												
	Notes												

	<₫	4	\$⊳	w.
Phase Number	2	4	6	8
Movement	NBTL	EBTL	SBTL	WBTL
Lead/Lag				
Lead-Lag Optimize				
Recall Mode	Max	None	Max	None
Maximum Split (s)	28	92	28	92
Maximum Split (%)	23.3%	76.7%	23.3%	76.7%
Minimum Split (s)	22.5	22.5	22.5	22.5
Yellow Time (s)	3.5	3.5	3.5	3.5
All-Red Time (s)	1	1	1	1
Minimum Initial (s)	5	5	5	5
Vehicle Extension (s)	3	3	3	3
Minimum Gap (s)	3	3	3	3
Time Before Reduce (s)	0	0	0	0
Time To Reduce (s)	0	0	0	0
Walk Time (s)	7	7	7	7
Flash Dont Walk (s)	11	11	11	11
Dual Entry	Yes	Yes	Yes	Yes
Inhibit Max	Yes	Yes	Yes	Yes
Start Time (s)	0	28	0	28
End Time (s)	28	0	28	0
Yield/Force Off (s)	23.5	115.5	23.5	115.5
Yield/Force Off 170(s)	12.5	104.5	12.5	104.5
Local Start Time (s)	0	28	0	28
Local Yield (s)	23.5	115.5	23.5	115.5
Local Yield 170(s)	12.5	104.5	12.5	104.5
Intersection Summary				
Cycle Length			120	
Control Type	Actuate	d-Uncoo		
Natural Cycle			60	
Splits and Phases: 3: Steit	z Road &	Home Ro	oad	
↑ Ø2	14	Ø4		
28 s	92 s			
	- 40	S.		
▼ Ø6	4	Ø8		

Intersection						
Int Delay, s/veh	1.8					
		EDT	MPT	WDD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7	†	705	7	Y	00
Traffic Vol, veh/h	59	560	795	39	24	36
Future Vol, veh/h	59	560	795	39	24	36
Conflicting Peds, #/hr	_ 0	_ 0	0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	345	-	-	345	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	64	609	864	42	26	39
Maiay/Minay	Maia = 1		Anin nO		\	
	Major1		Major2		Minor2	
Conflicting Flow All	906	0	-	0	1601	864
Stage 1	-	-	-	-	864	-
Stage 2	-	-	-	-	737	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	_	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	751	-	-	-	117	354
Stage 1	-	-	-	-	413	-
Stage 2	_	-	-	-	473	-
Platoon blocked, %		-	_	_		
Mov Cap-1 Maneuver	751	-	-	_	107	354
Mov Cap-2 Maneuver		_	_	_	107	_
Stage 1	_	_	_	_	378	_
Stage 2	_	_	_	_	473	_
Olaye Z					713	
Approach	EB		WB		SB	
HCM Control Delay, s	1		0		34.9	
HCM LOS					D	
NA: 1 (0.4.1		ED!	EDT	14/57	14/55	ODL 4
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	
Capacity (veh/h)		751	-	-	-	184
HCM Lane V/C Ratio		0.085	-	-	-	0.354
HCM Control Delay (s)	10.2	-	-	-	34.9
HCM Lane LOS		В	-	-	-	D
HCM 95th %tile Q(veh	1)	0.3	-	-	-	1.5

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		•	•	7		7
Traffic Vol, veh/h	0	584	806	29	0	28
Future Vol, veh/h	0	584	806	29	0	28
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	345	_	0
Veh in Median Storage,	# -	0	0	-	0	_
Grade, %	_	0	0	_	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	635	876	32	0	30
MAIN LIOM	U	033	0/0	32	U	30
Major/Minor M	lajor1	N	Major2	N	/linor2	
Conflicting Flow All		0		0	_	876
Stage 1	_	_	_		_	-
Stage 2	_	_	_	_	_	_
Critical Hdwy	_	_	_	_	_	6.22
Critical Hdwy Stg 1	_		_	_	_	0.22
, ,	-	_	_			-
Critical Hdwy Stg 2	-	-	-	-	-	
Follow-up Hdwy	-	-	-	-		3.318
Pot Cap-1 Maneuver	0	-	-	-	0	348
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	348
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	_	-	-	-	-	-
oluge _						
Annesah	ED		\A/D		CD	
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		16.3	
HCM LOS					С	
Minor Lane/Major Mvmt		EBT	WBT	WBR S	SRI n1	
		LDI	VVDI			
Capacity (veh/h)		-	-	-	348	
HCM Caretral Dalay (a)		-	-		0.087	
HCM Control Delay (s)		-	-	-		
HCM Lane LOS		-	-	-	С	
HCM 95th %tile Q(veh)		-	-	-	0.3	

Interception						
Intersection	3					
Int Delay, s/veh						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	N.			•	1	
Traffic Vol, veh/h	4	50	0	65	37	4
Future Vol, veh/h	4	50	0	65	37	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	54	0	71	40	4
	•	•				•
				_		
	Minor2		Major1		/lajor2	
Conflicting Flow All	113	42	-	0	-	0
Stage 1	42	-	-	-	-	-
Stage 2	71	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	-	-
Pot Cap-1 Maneuver	884	1029	0	-	-	-
Stage 1	980	-	0	-	-	-
Stage 2	952	-	0	_	_	-
Platoon blocked, %	302		- 3	_	_	_
Mov Cap-1 Maneuver	884	1029	_	_	_	_
Mov Cap-1 Maneuver	884	1023	_	_	_	_
Stage 1	980	_		_	_	_
Stage 2	952	-		_		_
Staye 2	302	-	-	-	_	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.8		0		0	
HCM LOS	Α					
Minor Long/Major Maria	ot.	NDT	EDI 51	CDT	CDD	
Minor Lane/Major Mvn	IL		EBLn1	SBT	SBR	
Capacity (veh/h)			1017	-	-	
HCM Lane V/C Ratio			0.058	-	-	
HCM Control Delay (s)		-	8.8	-	-	
HCM Lane LOS		-	Α	-	-	
HCM 95th %tile Q(veh)	-	0.2	-	-	

	۶	→	•	•	←	•	1	†	<i>></i>	/		4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ť	↑	7	ሻ	†	7	ሻ	₽		ሻ	₽	
Traffic Volume (veh/h)	15	734	69	83	410	14	20	27	113	8	25	24
Future Volume (veh/h)	15	734	69	83	410	14	20	27	113	8	25	24
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	16	798	75	90	446	15	22	29	123	9	27	26
Adj No. of Lanes	1	1	1	1	1	1	1	1	0	1	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	489	1025	871	245	1025	871	501	97	412	407	273	263
Arrive On Green	0.55	0.55	0.55	0.55	0.55	0.55	0.31	0.31	0.31	0.31	0.31	0.31
Sat Flow, veh/h	927	1863	1583	632	1863	1583	1346	311	1319	1230	873	841
Grp Volume(v), veh/h	16	798	75	90	446	15	22	0	152	9	0	53
Grp Sat Flow(s),veh/h/ln	927	1863	1583	632	1863	1583	1346	0	1630	1230	0	1714
Q Serve(g_s), s	0.7	22.1	1.5	8.6	9.3	0.3	0.8	0.0	4.6	0.4	0.0	1.4
Cycle Q Clear(g_c), s	10.0	22.1	1.5	30.7	9.3	0.3	2.2	0.0	4.6	5.0	0.0	1.4
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.81	1.00		0.49
Lane Grp Cap(c), veh/h	489	1025	871	245	1025	871	501	0	509	407	0	536
V/C Ratio(X)	0.03	0.78	0.09	0.37	0.44	0.02	0.04	0.00	0.30	0.02	0.00	0.10
Avail Cap(c_a), veh/h	833	1718	1460	480	1718	1460	501	0	509	407	0	536
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	11.7	11.6	7.0	23.7	8.7	6.7	16.8	0.0	17.1	19.0	0.0	16.0
Incr Delay (d2), s/veh	0.0	1.3	0.0	0.9	0.3	0.0	0.2	0.0	1.5	0.1	0.0	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	11.5	0.6	1.6	4.8	0.1	0.3	0.0	2.3	0.1	0.0	0.7
LnGrp Delay(d),s/veh	11.7	12.9	7.0	24.6	9.0	6.7	17.0	0.0	18.6	19.1	0.0	16.4
LnGrp LOS	В	В	A	С	A	A	В		В	B		В
Approach Vol, veh/h		889			551			174			62	
Approach Delay, s/veh		12.4			11.5			18.4			16.8	
Approach LOS		В			В			В			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		25.0		40.6		25.0		40.6				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		20.5		60.5		20.5		60.5				
Max Q Clear Time (g_c+l1), s		6.6		24.1		7.0		32.7				
Green Ext Time (p_c), s		0.7		6.5		0.2		3.4				
Intersection Summary												
HCM 2010 Ctrl Delay			12.9									
HCM 2010 LOS			В									
Notes												

	4	*	4	*
Phase Number	2	4	6	8
Movement	NBTL	EBTL	SBTL	WBTL
Lead/Lag				
Lead-Lag Optimize				
Recall Mode	Max	None	Max	None
Maximum Split (s)	25	65	25	65
Maximum Split (%)	27.8%	72.2%	27.8%	72.2%
Minimum Split (s)	22.5	22.5	22.5	22.5
Yellow Time (s)	3.5	3.5	3.5	3.5
All-Red Time (s)	1	1	1	1
Minimum Initial (s)	5	5	5	5
Vehicle Extension (s)	3	3	3	3
Minimum Gap (s)	3	3	3	3
Time Before Reduce (s)	0	0	0	0
Time To Reduce (s)	0	0	0	0
Walk Time (s)	7	7	7	7
Flash Dont Walk (s)	11	11	11	11
Dual Entry	Yes	Yes	Yes	Yes
Inhibit Max	Yes	Yes	Yes	Yes
Start Time (s)	0	25	0	25
End Time (s)	25	0	25	0
Yield/Force Off (s)	20.5	85.5	20.5	85.5
Yield/Force Off 170(s)	9.5	74.5	9.5	74.5
Local Start Time (s)	0	25	0	25
Local Yield (s)	20.5	85.5	20.5	85.5
Local Yield 170(s)	9.5	74.5	9.5	74.5
	7.0	. 110	7.0	. 110
Intersection Summary Cycle Length			90	
Control Type	Actuato	ed-Uncoo		
Natural Cycle	Actuale	u-Uncuu	60	
ivatural Cycle			00	
Splits and Phases: 3: Steit	z Road &	Home Re	nad	
φ in a rina co. σ. σισι	1\0au \0	A IOITIC IX	Juu	
Tø2		∜ Ø4		
25 s		65 s		
1		+2-		
▼ Ø6		▼ Ø8		

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ř	†	7	7		7	Ĭ	f)		J.	ĵ»	
Traffic Volume (veh/h)	15	740	70	83	447	14	28	27	113	20	27	24
Future Volume (veh/h)	15	740	70	83	447	14	28	27	113	20	27	24
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	16	804	76	90	486	15	30	29	123	22	29	26
Adj No. of Lanes	1	1	1	1	1	1	1	1	0	1	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	464	1034	879	245	1034	879	493	96	408	401	280	251
Arrive On Green	0.56	0.56	0.56	0.56	0.56	0.56	0.31	0.31	0.31	0.31	0.31	0.31
Sat Flow, veh/h	894	1863	1583	628	1863	1583	1343	311	1319	1230	907	813
Grp Volume(v), veh/h	16	804	76	90	486	15	30	0	152	22	0	55
Grp Sat Flow(s),veh/h/ln	894	1863	1583	628	1863	1583	1343	0	1630	1230	0	1719
Q Serve(g_s), s	0.7	22.4	1.5	8.7	10.4	0.3	1.1	0.0	4.7	0.9	0.0	1.5
Cycle Q Clear(g_c), s	11.1	22.4	1.5	31.1	10.4	0.3	2.6	0.0	4.7	5.6	0.0	1.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.81	1.00		0.47
Lane Grp Cap(c), veh/h	464	1034	879	245	1034	879	493	0	504	401	0	531
V/C Ratio(X)	0.03	0.78	0.09	0.37	0.47	0.02	0.06	0.00	0.30	0.05	0.00	0.10
Avail Cap(c_a), veh/h	783	1699	1444	469	1699	1444	493	0	504	401	0	531
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	12.2	11.5	6.9	23.7	8.9	6.6	17.3	0.0	17.5	19.6	0.0	16.3
Incr Delay (d2), s/veh	0.0	1.3	0.0	0.9	0.3	0.0	0.2	0.0	1.5	0.3	0.0	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	11.8	0.6	1.6	5.4	0.1	0.4	0.0	2.3	0.3	0.0	0.8
LnGrp Delay(d),s/veh	12.3	12.8	6.9	24.6	9.2	6.6	17.5	0.0	19.0	19.9	0.0	16.7
LnGrp LOS	В	В	Α	С	Α	Α	В		В	В		В
Approach Vol, veh/h		896			591			182			77	
Approach Delay, s/veh		12.3			11.5			18.7			17.6	
Approach LOS		В			В			В			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		25.0		41.3		25.0		41.3				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		20.5		60.5		20.5		60.5				
Max Q Clear Time (g_c+I1), s		6.7		24.4		7.6		33.1				
Green Ext Time (p_c), s		0.7		6.5		0.2		3.7				
Intersection Summary												
HCM 2010 Ctrl Delay			12.9									
HCM 2010 LOS			В									
Notes												

	4	4	4	*
Phase Number	2	4	6	8
Movement	NBTL	EBTL	SBTL	WBTL
Lead/Lag				
Lead-Lag Optimize				
Recall Mode	Max	None	Max	None
Maximum Split (s)	25	65	25	65
Maximum Split (%)	27.8%	72.2%	27.8%	72.2%
Minimum Split (s)	22.5	22.5	22.5	22.5
Yellow Time (s)	3.5	3.5	3.5	3.5
All-Red Time (s)	1	1	1	1
Minimum Initial (s)	5	5	5	5
Vehicle Extension (s)	3	3	3	3
Minimum Gap (s)	3	3	3	3
Time Before Reduce (s)	0	0	0	0
Time To Reduce (s)	0	0	0	0
Walk Time (s)	7	7	7	7
Flash Dont Walk (s)	11	11	11	11
Dual Entry	Yes	Yes	Yes	Yes
Inhibit Max	Yes	Yes	Yes	Yes
Start Time (s)	0	25	0	25
End Time (s)	25	0	25	0
Yield/Force Off (s)	20.5	85.5	20.5	85.5
Yield/Force Off 170(s)	9.5	74.5	9.5	74.5
Local Start Time (s)	0	25	0	25
Local Yield (s)	20.5	85.5	20.5	85.5
Local Yield 170(s)	9.5	74.5	9.5	74.5
Intersection Summary				
Cycle Length			90	
Control Type	Actuate	ed-Uncoo		
Natural Cycle	7.0.000	200	60	
,				
Splits and Phases: 3: Steit:	z Road &	Home Ro	oad	
-4.♣		<u> </u>		
Ø2		∜ Ø4		
25 s		65 s		
1		 2		
▼ Ø6 25 s		∜ Ø8 65 s		

Intersection						
Int Delay, s/veh	0.5					
					0.57	005
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations				7	W	
Traffic Vol, veh/h	37	818	462	26	7	11
Future Vol, veh/h	37	818	462	26	7	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	345	-	-	345	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	40	889	502	28	8	12
					41 0	
	Major1		/lajor2		Minor2	
Conflicting Flow All	530	0	-	0	1471	502
Stage 1	-	-	-	-	502	-
Stage 2	-	-	-	-	969	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1037	-	-	-	140	569
Stage 1	-	-	-	-	608	-
Stage 2	-	-	_	-	368	-
Platoon blocked, %		-	_	-		
Mov Cap-1 Maneuver	1037	-	-	-	135	569
Mov Cap-2 Maneuver	-	_	_	_	135	-
Stane 1	_	_		_		-
Stage 1	-	-	-		584	-
Stage 1 Stage 2	-	-	-			
· ·		-	-		584	-
· ·		-	-		584	-
Stage 2	EB		-		584 368	-
Stage 2 Approach	-	-	- - WB		584 368 SB	-
Stage 2 Approach HCM Control Delay, s	EB	-	- - WB		584 368 SB 20.4	-
Stage 2 Approach HCM Control Delay, s HCM LOS	EB 0.4		- - WB	-	584 368 SB 20.4 C	-
Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvn	EB 0.4	EBL	- - WB		584 368 SB 20.4	- - SBLn1
Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvn Capacity (veh/h)	EB 0.4	EBL 1037	- - WB	-	584 368 SB 20.4 C	SBLn1 253
Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvn Capacity (veh/h) HCM Lane V/C Ratio	EB 0.4	EBL 1037 0.039	- - WB	-	584 368 SB 20.4 C	SBLn1 253 0.077
Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvn Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	EB 0.4	EBL 1037 0.039 8.6	WB 0	- - WBT	584 368 SB 20.4 C	SBLn1 253 0.077 20.4
Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvn Capacity (veh/h) HCM Lane V/C Ratio	EB 0.4	EBL 1037 0.039	- - WB 0	WBT	584 368 SB 20.4 C	SBLn1 253 0.077

Intersection						
Int Delay, s/veh	0.1					
						0.5
	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations				- 7		7
Traffic Vol, veh/h	0	825	480	19	0	8
Future Vol, veh/h	0	825	480	19	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	345	-	0
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	897	522	21	0	9
WWW. Tiow	U	071	022	21	U	,
Major/Minor Ma	ajor1	Λ	Najor2	N	/linor2	
Conflicting Flow All	-	0	-	0	-	522
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	_	_	-	-	3.318
Pot Cap-1 Maneuver	0	_	_	_	0	555
Stage 1	0	_	_	_	0	-
Stage 2	0				0	_
Platoon blocked, %	U	_	-	-	U	_
		-	-	-	-	555
Mov Cap 2 Manager	-			-		333
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		11.6	
HCM LOS	0				В	
TOW LOO						
Minor Lane/Major Mvmt		EBT	WBT	WBR S	SBLn1	
Capacity (veh/h)		-	_	-	555	
HCM Lane V/C Ratio		-	-	-	0.016	
HCM Control Delay (s)		-	-	-	11.6	
HCM Lane LOS		_	_	_	В	
HCM 95th %tile Q(veh)		-	-		0	
HOW FOUT FOUT QUELLY					U	

Intersection						
Intersection Int Delay, s/veh	1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥				₽	
Traffic Vol, veh/h	1	14	0	56	57	3
Future Vol, veh/h	1	14	0	56	57	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	15	0	61	62	3
N.A. 1. (N.A.)	N 41 C					
	Minor2		/lajor1		/lajor2	
Conflicting Flow All	125	64	-	0	-	0
Stage 1	64	-	-	-	-	-
Stage 2	61	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	-	-
Pot Cap-1 Maneuver	870	1000	0	-	-	-
Stage 1	959	-	0	-	-	-
Stage 2	962	-	0	-	-	-
Platoon blocked, %	702			_	_	_
Mov Cap-1 Maneuver	870	1000		_	_	_
Mov Cap 1 Maneuver	870	-	_	_	_	_
Stage 1	959	-		-	-	-
· ·	962	-	-	-	-	-
Stage 2	702	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.7		0		0	
HCM LOS	Α					
Minan Lana (Martan Ad		NET	-DL 4	CDT	CDD	
Minor Lane/Major Mvn	nt	NBT E		SBT	SBR	
Capacity (veh/h)		-	990	-	-	
HCM Lane V/C Ratio		-	0.016	-	-	
HCM Control Delay (s))	-	8.7	-	-	
HCM Lane LOS HCM 95th %tile Q(veh		-	A 0.1	-	-	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ť	^	7	7	^	7	7	f)		7	f)	
Traffic Volume (veh/h)	24	576	72	50	828	32	69	22	32	15	7	21
Future Volume (veh/h)	24	576	72	50	828	32	69	22	32	15	7	21
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	26	626	78	54	900	35	75	24	35	16	8	23
Adj No. of Lanes	1	1	1	1	1	1	1	1	0	1	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	198	1047	890	359	1047	890	507	209	305	481	129	372
Arrive On Green	0.56	0.56	0.56	0.56	0.56	0.56	0.30	0.30	0.30	0.30	0.30	0.30
Sat Flow, veh/h	596	1863	1583	740	1863	1583	1373	686	1000	1338	425	1222
Grp Volume(v), veh/h	26	626	78	54	900	35	75	0	59	16	0	31
Grp Sat Flow(s),veh/h/ln	596	1863	1583	740	1863	1583	1373	0	1686	1338	0	1647
Q Serve(g_s), s	2.6	14.9	1.5	3.5	27.6	0.7	2.8	0.0	1.7	0.6	0.0	0.9
Cycle Q Clear(g_c), s	30.2	14.9	1.5	18.4	27.6	0.7	3.7	0.0	1.7	2.3	0.0	0.9
Prop In Lane	1.00	4047	1.00	1.00	40.47	1.00	1.00		0.59	1.00		0.74
Lane Grp Cap(c), veh/h	198	1047	890	359	1047	890	507	0	513	481	0	502
V/C Ratio(X)	0.13	0.60	0.09	0.15	0.86	0.04	0.15	0.00	0.11	0.03	0.00	0.06
Avail Cap(c_a), veh/h	664	2504	2128	938	2504	2128	507	0	513	481	0	502
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	25.3	9.7	6.8	15.8	12.5	6.6	17.9	0.0	16.9	17.7	0.0	16.6
Incr Delay (d2), s/veh	0.3	0.6	0.0	0.2	2.2	0.0	0.6	0.0	0.5	0.1	0.0	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	7.6	0.7	0.7	14.6	0.3	1.1	0.0	0.9	0.2	0.0	0.4
LnGrp Delay(d),s/veh	25.6	10.3	6.8	16.0	14.7	6.6	18.5	0.0	17.3	17.8	0.0	16.8
LnGrp LOS	С	B 700	A	В	В	A	В	104	В	В	47	В
Approach Vol, veh/h		730			989			134			47	
Approach LOS		10.5			14.5			18.0			17.2	
Approach LOS		В			В			В			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		25.0		42.3		25.0		42.3				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		20.5		90.5		20.5		90.5				
Max Q Clear Time (g_c+I1), s		5.7		32.2		4.3		29.6				
Green Ext Time (p_c), s		0.4		4.9		0.1		8.2				
Intersection Summary												
HCM 2010 Ctrl Delay			13.3									
HCM 2010 LOS			В									
Notes												

	4∱	4	4	*
Phase Number	2	4	6	8
Movement	NBTL	EBTL	SBTL	WBTL
Lead/Lag				
Lead-Lag Optimize				
Recall Mode	Max	None	Max	None
Maximum Split (s)	25	95	25	95
Maximum Split (%)	20.8%	79.2%	20.8%	79.2%
Minimum Split (s)	22.5	22.5	22.5	22.5
Yellow Time (s)	3.5	3.5	3.5	3.5
All-Red Time (s)	1	1	1	1
Minimum Initial (s)	5	5	5	5
Vehicle Extension (s)	3	3	3	3
Minimum Gap (s)	3	3	3	3
Time Before Reduce (s)	0	0	0	0
Time To Reduce (s)	0	0	0	0
Walk Time (s)	7	7	7	7
Flash Dont Walk (s)	11	11	11	11
Dual Entry	Yes	Yes	Yes	Yes
Inhibit Max	Yes	Yes	Yes	Yes
Start Time (s)	0	25	0	25
End Time (s)	25	0	25	0
Yield/Force Off (s)	20.5	115.5	20.5	115.5
Yield/Force Off 170(s)	9.5	104.5	9.5	104.5
Local Start Time (s)	0	25	0	25
Local Yield (s)	20.5	115.5	20.5	115.5
Local Yield 170(s)	9.5	104.5	9.5	104.5
Intersection Summary				
Cycle Length			120	
Control Type	Actuate	ed-Uncoo		
Natural Cycle			60	
- ,				
Splits and Phases: 3: Stei	itz Road &	Home Ro	oad	
+	1			
Ø2	₩ -1 04			
25 s	50 S			
₽ 06	₩ Ø8			
25.0	05 c			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	↑	7	ሻ	•	7	ሻ	₽		ነ	₽	
Traffic Volume (veh/h)	24	596	76	50	884	32	81	22	32	56	16	21
Future Volume (veh/h)	24	596	76	50	884	32	81	22	32	56	16	21
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	26	648	83	54	961	35	88	24	35	61	17	23
Adj No. of Lanes	1	1	1	1	1	1	1	1	0	1	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	188	1102	937	370	1102	937	462	195	284	444	204	276
Arrive On Green	0.59	0.59	0.59	0.59	0.59	0.59	0.28	0.28	0.28	0.28	0.28	0.28
Sat Flow, veh/h	563	1863	1583	722	1863	1583	1362	686	1000	1338	719	972
Grp Volume(v), veh/h	26	648	83	54	961	35	88	0	59	61	0	40
Grp Sat Flow(s),veh/h/ln	563	1863	1583	722	1863	1583	1362	0	1686	1338	0	1691
Q Serve(g_s), s	2.9	15.7	1.6	3.7	31.4	0.7	3.7	0.0	1.9	2.6	0.0	1.3
Cycle Q Clear(g_c), s	34.4	15.7	1.6	19.4	31.4	0.7	4.9	0.0	1.9	4.4	0.0	1.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.59	1.00		0.57
Lane Grp Cap(c), veh/h	188	1102	937	370	1102	937	462	0	478	444	0	480
V/C Ratio(X)	0.14	0.59	0.09	0.15	0.87	0.04	0.19	0.00	0.12	0.14	0.00	0.08
Avail Cap(c_a), veh/h	560	2332	1983	846	2332	1983	462	0	478	444	0	480
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	26.9	9.2	6.4	15.3	12.4	6.2	20.8	0.0	19.2	20.9	0.0	19.0
Incr Delay (d2), s/veh	0.3	0.5	0.0	0.2	2.3	0.0	0.9	0.0	0.5	0.6	0.0	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	8.1	0.7	0.7	16.5	0.3	1.5	0.0	0.9	1.0	0.0	0.6
LnGrp Delay(d),s/veh	27.3	9.7	6.4	15.5	14.7	6.2	21.7	0.0	19.7	21.5	0.0	19.3
LnGrp LOS	С	Α	Α	В	В	Α	С		В	С		<u>B</u>
Approach Vol, veh/h		757			1050			147			101	
Approach Delay, s/veh		10.0			14.5			20.9			20.6	
Approach LOS		Α			В			С			С	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		25.0		47.3		25.0		47.3				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		20.5		90.5		20.5		90.5				
Max Q Clear Time (g_c+l1), s		6.9		36.4		6.4		33.4				
Green Ext Time (p_c), s		0.4		5.2		0.2		9.3				
Intersection Summary												
HCM 2010 Ctrl Delay			13.6									
HCM 2010 LOS			В									
Notes												

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Phase Number	2	4	6	8
Movement	NBTL	EBTL	SBTL	WBTL
Lead/Lag			33.2	
Lead-Lag Optimize				
Recall Mode	Max	None	Max	None
Maximum Split (s)	25	95	25	95
Maximum Split (%)	20.8%	79.2%	20.8%	79.2%
Minimum Split (s)	22.5	22.5	22.5	22.5
Yellow Time (s)	3.5	3.5	3.5	3.5
All-Red Time (s)	1	1	1	1
Minimum Initial (s)	5	5	5	5
Vehicle Extension (s)	3	3	3	3
Minimum Gap (s)	3	3	3	3
Time Before Reduce (s)	0	0	0	0
Time To Reduce (s)	0	0	0	0
Walk Time (s)	7	7	7	7
Flash Dont Walk (s)	11	11	11	11
Dual Entry	Yes	Yes	Yes	Yes
Inhibit Max	Yes	Yes	Yes	Yes
Start Time (s)	0	25	0	25
End Time (s)	25	0	25	0
Yield/Force Off (s)	20.5	115.5	20.5	115.5
Yield/Force Off 170(s)	9.5	104.5	9.5	104.5
Local Start Time (s)	0	25	0	25
Local Yield (s)	20.5	115.5	20.5	115.5
Local Yield 170(s)	9.5	104.5	9.5	104.5
Intersection Summary			7.0	
Cycle Length			120	
Control Type	Actuate	ed-Uncoo		
Natural Cycle	rioladic	Ja Onlood	65	
reater at Oyolo			00	
Splits and Phases: 3: Stei	itz Road &	Home Ro	oad	
A		. 101110 14		
Ø2	₩04			
25 s	95 s			
I leave	1 +2			
▼ Ø6 25 s	∜ Ø8 95 s			

Intersection						
Int Delay, s/veh	2.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7	<u> </u>	↑	7	₩	JUIN
Traffic Vol, veh/h	59	672	946	39	24	36
Future Vol, veh/h	59	672	946	39	24	36
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	- 0.45	None	-	None	-	None
Storage Length	345	-	-	345	0	-
Veh in Median Storage	2,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	64	730	1028	42	26	39
Major/Minor	Major1	N	Najor2		Minor2	
Conflicting Flow All	1070	0	-	0	1886	1028
Stage 1	-	-	-	-	1028	-
Stage 2	-	-	-	-	858	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	_	-	_	5.42	_
Critical Hdwy Stg 2	-	_	_	_	5.42	_
Follow-up Hdwy	2.218	_	_	_	3.518	3 318
Pot Cap-1 Maneuver	651		_	-	78	284
Stage 1	- 001		_	_	345	204
		-				
Stage 2	-	-	-	-	415	-
Platoon blocked, %	(54	-	-	-	70	004
Mov Cap-1 Maneuver	651	-	-	-	70	284
Mov Cap-2 Maneuver	-	-	-	-	70	-
Stage 1	-	-	-	-	311	-
Stage 2	-	-	-	-	415	-
Annraaah	ΓD		WD		CD	
Approach	EB		WB		SB	
HCM Control Delay, s	0.9		0		59.2	
HCM LOS					F	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR S	SRI n1
	It		LUI	VVDI		
Capacity (veh/h)		651	-	-	-	128
HCM Lane V/C Ratio		0.099	-	-	-	0.51
HCM Control Delay (s)		11.1	-	-	-	59.2
HCM Lane LOS		В	-	-	-	F
HCM 95th %tile Q(veh)	0.3	-	-	-	2.4

Intersection						
Intersection Int Delay, s/veh	0.3					
	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations				- 7		7
Traffic Vol, veh/h	0	696	957	29	0	28
Future Vol, veh/h	0	696	957	29	0	28
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	345	-	0
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	757	1040	32	0	30
IVIVIIIC I IOVV	U	131	1040	32	U	30
Major/Minor Ma	ajor1	N	Najor2	N	/linor2	
Conflicting Flow All	-	0	-	0	-	1040
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	_	-	-	-	-	6.22
Critical Hdwy Stg 1	-	-	-	_	-	_
Critical Hdwy Stg 2	_	-	_	_	-	-
Follow-up Hdwy	_	_	_	_	_	3.318
Pot Cap-1 Maneuver	0	_	_	_	0	280
Stage 1	0	_	_	_	0	-
Stage 2	0	_	-	_	0	-
	U	-	_		U	-
Platoon blocked, %		-	-	-		200
Mov Cap-1 Maneuver	-	-	-	-	-	280
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		19.4	
HCM LOS	U		- 0		C	
TIOWI LOO						
Minor Lane/Major Mvmt		EBT	WBT	WBR S	SBLn1	
Capacity (veh/h)		-	-	-	280	
HCM Lane V/C Ratio		-	-	-	0.109	
HCM Control Delay (s)		-	-	-	19.4	
HCM Lane LOS		-	_	_	С	
HCM 95th %tile Q(veh)		-	-	-	0.4	
1101V1 70111 701110 Q(VCII)					0.7	

Intersection						
Int Delay, s/veh	2.7					
		E55	No	NET	057	055
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W				₽	
Traffic Vol, veh/h	4	50	0	78	43	4
Future Vol, veh/h	4	50	0	78	43	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	54	0	85	47	4
Major/Minor	Minor2	N	/lajor1	, A	/lajor2	
						0
Conflicting Flow All	134	49	-	0	-	0
Stage 1	49	-	-	-	-	-
Stage 2	85	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518		-	-	-	-
Pot Cap-1 Maneuver	860	1020	0	-	-	-
Stage 1	973	-	0	-	-	-
Stage 2	938	-	0	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	860	1020	-	-	-	-
Mov Cap-2 Maneuver	860	-	-	-	-	-
Stage 1	973	-	-	-	-	-
Stage 2	938	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.8		0		0	
HCM LOS	Α					
Minor Lane/Major Mvn	nt	NBT E	EBLn1	SBT	SBR	
Capacity (veh/h)		_	1006	_	_	
HCM Lane V/C Ratio			0.058	_	-	
HCM Control Delay (s)		_	8.8	-	_	
HCM Lane LOS		_	A	_	_	
HCM 95th %tile Q(veh)	-	0.2	-	-	
1.5W 75W 76W 64 (VCI)	,		0.2			

December 1, 2018

City of Powell 47 Hall Street Powell, OH 43065 Attention: Steve Lutz

Dear Mr. Lutz:

Please accept this letter as confirmation of our appointment of John C. Wicks, member of Home Steitz LLC, as our Agent relating to all matters concerning the current and Annexation, Zoning, and Development Plan applications of our property known as Parcel Number 319-230-01-003-000. I hereby give Mr. Wicks the authority to sign relevant zoning applications, annexation applications, forms and other documents on our behalf, as required by the City of Powell or Delaware County, for the current annexation and zoning applications.

Sincerely,

Kim E. Kelsik Authorized Member TLK Development LLC

State of AZ County of PIMA

Subscribed and sworn to (or affirmed) before me

this 1th day of DECEMBER , 20 18

By JANNA SHOPHERS

Personally known OR produced identification

Type of identification produced AZDL

Notary Name Here, Notary Public

My Commission Expires APPIL 29, 2022

NOTARY PUBLIC
STATE OF ARIZONA
Pima County
JANNA SHEPHERD
My Commission Expires April 29, 2022